

J. H. Frankland

STRUCTURAL STEEL SHAPES

INFORMATION AND TABLES
FOR
ENGINEERS AND DESIGNERS

CARNEGIE STEEL COMPANY

PITTSBURGH, PA.

COMPLIMENTS

OF

AMERICAN BRIDGE COMPANY

57 BROADWAY, NEW YORK

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STRUCTURAL STEEL SHAPES

INFORMATION AND TABLES
FOR
ENGINEERS AND DESIGNERS
AND OTHER DATA
PERTAINING TO
STRUCTURAL STEEL

MANUFACTURED BY
CARNEGIE STEEL COMPANY
PITTSBURGH, PA.

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STRUCTURAL
STEEL SHAPES

INFORMATION AND TABLES

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Pittsburgh, Pa.

STRUCTURAL STEEL

CARNegie STEEL COMPANY

First Edition, January 1, 1926.

Printed in U. S. A.

THIS pamphlet contains tables and other data for structural sections considered most suitable for use in bridge and building construction.

The safe loads for structural sections used as beams are given in completely tabulated form for unit fiber stresses of 18,000 and of 16,000 pounds per square inch.

Structural steel is rolled to permissible variations given in the specifications of the American Society for Testing Materials and of the Association of American Steel Manufacturers. In designs requiring close fitting, allowance should be made for rolling variations to insure ample clearance between abutting or interfitting surfaces.

All dimensions given on profiles are theoretical. The exact dimensions of structural sections depend on condition of rolls.

Wherever the profile applies to more than one weight of section, the dimensions are for the normal profile, which is the section of minimum thickness unless otherwise indicated in bold type. Sections having but one weight specified are rolled to that weight only.

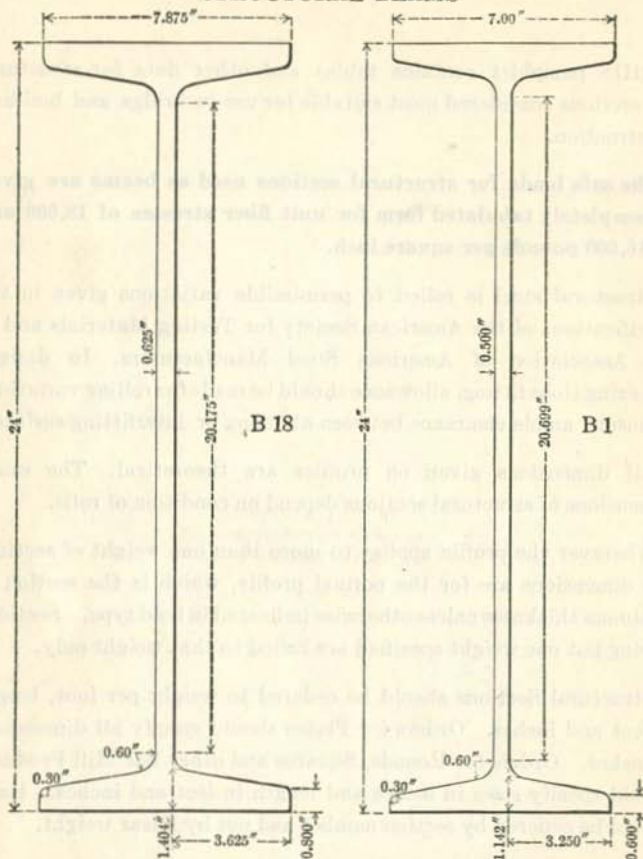
Structural Sections should be ordered to weight per foot, length in feet and inches. Orders for Plates should specify all dimensions in inches. Orders for Rounds, Squares and other Bar Mill Products should specify sizes in inches and length in feet and inches. Rails should be ordered by section number and not by linear weight.

Section number should be specified on orders for all sections.

The Association of American Steel Manufacturers has recommended certain angle sections as standard for general building construction, and quicker deliveries can be obtained by ordering standard sizes. Angles not standard are marked "special" on profile pages.

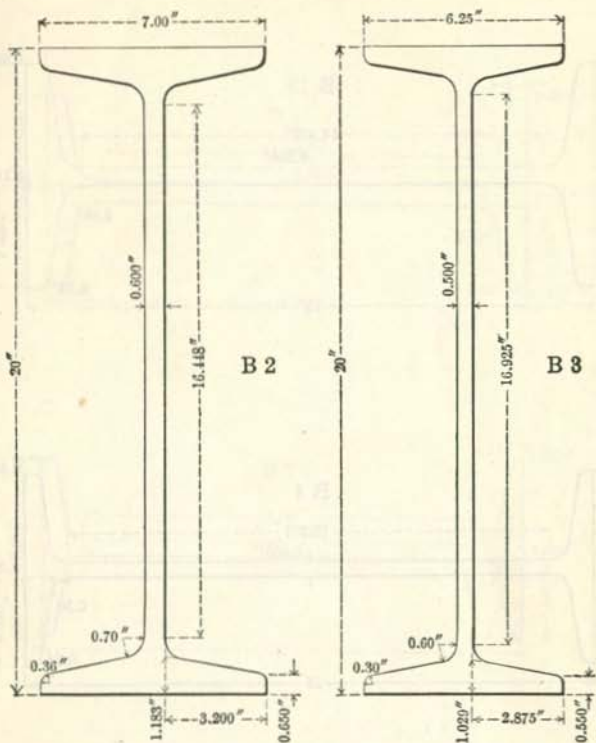
CARNEGIE STEEL COMPANY

STRUCTURAL BEAMS



Section Index	Depth of Beam, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
B 18 (Old No. B 24)	24	120.0	8.048	$8\frac{3}{64}$	0.798	$\frac{51}{64}$
		115.0	7.987	$7\frac{63}{64}$	0.737	$\frac{47}{64}$
		110.0	7.925	$7\frac{59}{64}$	0.675	$\frac{43}{64}$
		105.9	7.875	$7\frac{7}{8}$	0.625	$\frac{5}{8}$
B 1	24	100.0	7.247	$7\frac{1}{4}$	0.747	$\frac{3}{4}$
		95.0	7.186	$7\frac{1}{16}$	0.686	$1\frac{1}{16}$
		90.0	7.124	$7\frac{1}{8}$	0.624	$\frac{5}{8}$
		85.0	7.063	$7\frac{1}{16}$	0.563	$\frac{9}{16}$
		79.9	7.000	7	0.500	$\frac{1}{2}$

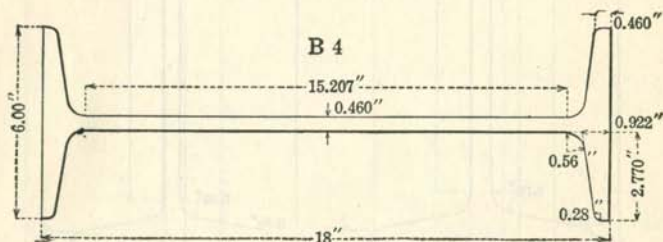
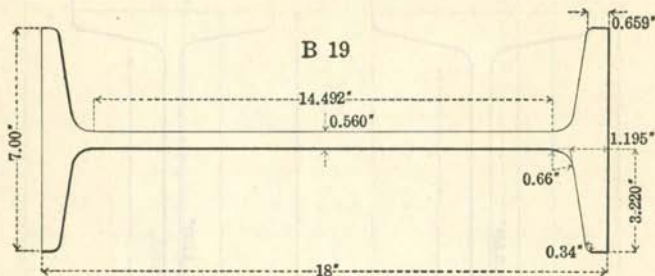
STRUCTURAL BEAMS—Continued



Section Index	Depth of Beam, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
B 2	20	100.0	7.273	$7\frac{1}{4}$	0.873	$\frac{7}{8}$
		95.0	7.200	$7\frac{1}{8}$	0.800	$5\frac{1}{8}$
		90.0	7.126	$7\frac{1}{8}$	0.726	$2\frac{3}{32}$
		85.0	7.053	$7\frac{3}{16}$	0.653	$2\frac{1}{32}$
		81.4	7.000	7	0.600	$1\frac{9}{16}$
B 3	20	75.0	6.391	$6\frac{2}{8}$	0.641	$4\frac{1}{8}$
		70.0	6.317	$6\frac{5}{16}$	0.567	$9\frac{1}{16}$
		65.4	6.250	$6\frac{1}{4}$	0.500	$\frac{1}{2}$

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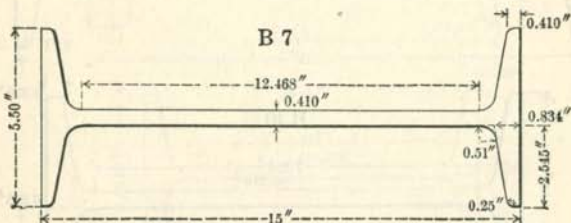
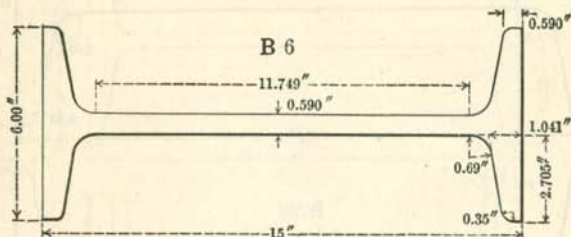
STRUCTURAL BEAMS—Continued



Section Index	Depth of Beam, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
B 19 (Old No. B 81)	18	90.0	7.236	$7\frac{15}{64}$	0.796	$\frac{51}{64}$
		85.0	7.154	$7\frac{7}{32}$	0.714	$\frac{23}{32}$
		80.0	7.072	$7\frac{7}{64}$	0.632	$\frac{5}{8}$
		75.6	7.000	7	0.560	$\frac{7}{16}$
B 4 (Old No. B 80)	18	70.0	6.251	$6\frac{1}{4}$	0.711	$\frac{23}{32}$
		65.0	6.169	$6\frac{11}{64}$	0.629	$\frac{5}{8}$
		60.0	6.087	$6\frac{3}{32}$	0.547	$\frac{35}{64}$
		54.7	6.000	6	0.460	$\frac{29}{64}$

BEAMS

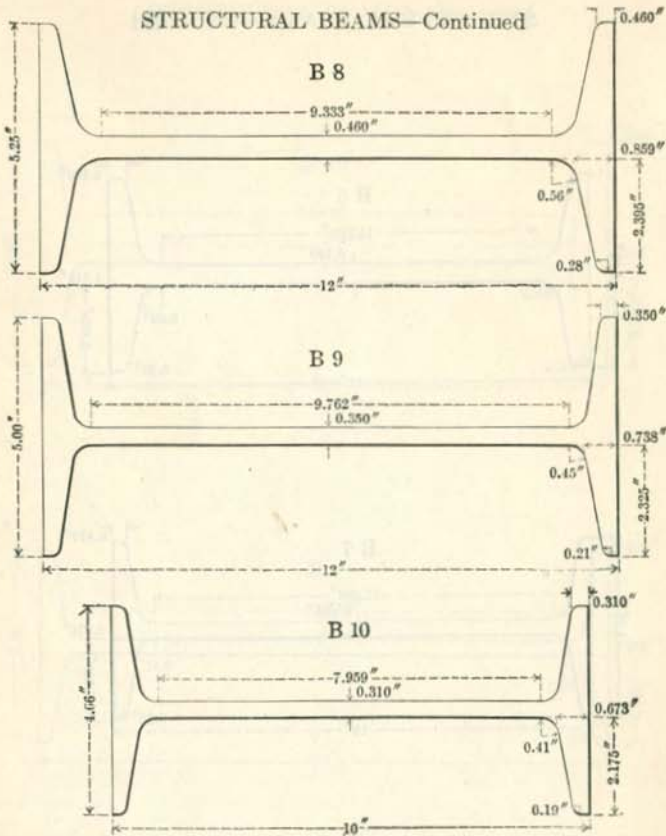
STRUCTURAL BEAMS—Continued



Section Index	Depth of Beam, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
B 6 (Old No. B 5)	15	75.0	6.278	6 $\frac{9}{32}$	0.868	$\frac{7}{8}$
		70.0	6.180	6 $\frac{7}{16}$	0.770	4 $\frac{9}{16}$
		65.0	6.082	6 $\frac{5}{16}$	0.672	4 $\frac{3}{16}$
		60.8	6.000	6	0.590	1 $\frac{9}{32}$
B 7	15	55.0	5.738	5 $\frac{7}{16}$	0.648	4 $\frac{1}{16}$
		50.0	5.640	5 $\frac{1}{16}$	0.550	3 $\frac{5}{16}$
		45.0	5.542	5 $\frac{25}{64}$	0.452	2 $\frac{9}{16}$
		42.9	5.500	5 $\frac{1}{2}$	0.410	1 $\frac{3}{8}$

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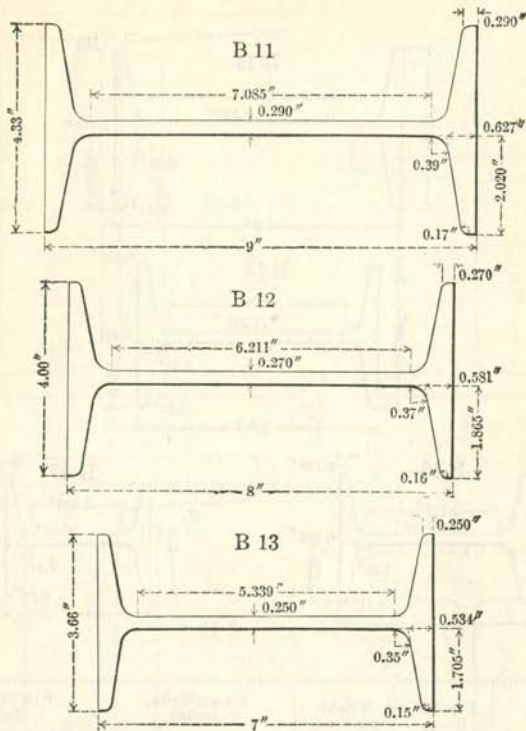
STRUCTURAL BEAMS—Continued



Section Index	Depth of Beam, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
B 8	12	55.0	5.600	5 ¹⁹ / ₃₂	0.810	13 ¹ / ₁₆
		50.0	5.477	5 ³¹ / ₆₄	0.687	1 ¹ / ₁₆
		45.0	5.355	5 ²³ / ₆₄	0.565	9 ¹ / ₁₆
		40.8	5.250	5 ¹ / ₄	0.460	2 ⁹ / ₁₆
B 9	12	35.0	5.078	5 ⁵ / ₁₆	0.428	27 ³ / ₁₆
		31.8	5.000	5	0.350	1 ¹³ / ₃₂
B 10 (Old No. B 11)	10	40.0	5.091	5 ⁹ / ₃₂	0.741	47 ³ / ₁₆
		35.0	4.944	4 ¹⁹ / ₁₆	0.594	1 ⁹ / ₃₂
		30.0	4.797	4 ³¹ / ₆₄	0.447	2 ⁹ / ₁₆
		25.4	4.660	4 ²¹ / ₃₂	0.310	5 ¹ / ₁₆

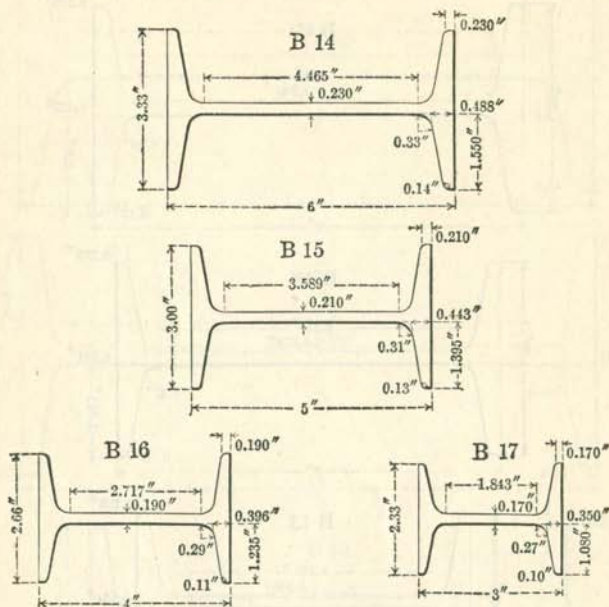
BEAMS

STRUCTURAL BEAMS—Continued



Section Index	Depth of Beam, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
B 11 (Old No B 13)	9	35.0	4.764	$4\frac{49}{64}$	0.724	$2\frac{3}{32}$
		30.0	4.601	$4\frac{19}{32}$	0.561	$\frac{9}{16}$
		25.0	4.437	$4\frac{7}{16}$	0.397	$2\frac{3}{64}$
		21.8	4.330	$4\frac{27}{64}$	0.290	$1\frac{9}{64}$
B 12 (Old No B 15)	8	25.5	4.262	$4\frac{17}{64}$	0.532	$1\frac{7}{32}$
		23.0	4.171	$4\frac{11}{64}$	0.441	$\frac{7}{16}$
		20.5	4.079	$4\frac{5}{64}$	0.349	$1\frac{1}{32}$
		18.4	4.000	4	0.270	$1\frac{7}{64}$
B 13 (Old No B 17)	7	20.0	3.860	$3\frac{55}{64}$	0.450	$2\frac{9}{32}$
		17.5	3.755	$3\frac{3}{4}$	0.345	$1\frac{1}{32}$
		15.3	3.660	$3\frac{21}{32}$	0.250	$\frac{1}{4}$

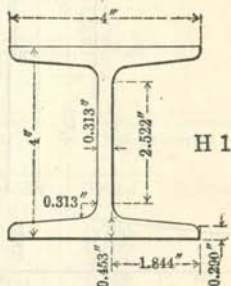
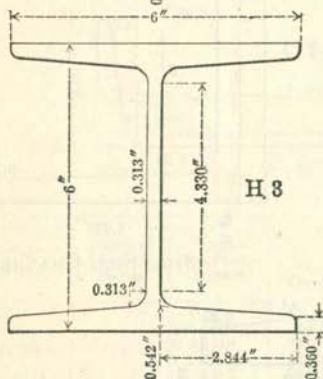
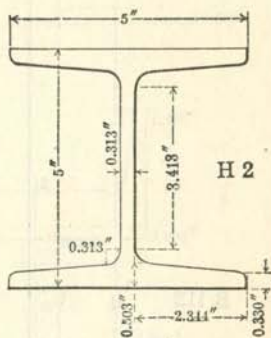
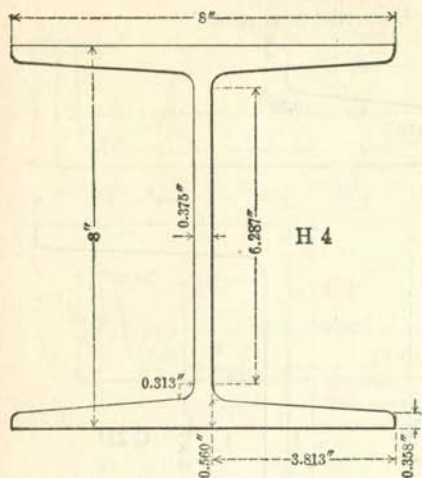
STRUCTURAL BEAMS—Concluded



Section Index	Depth of Beam, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
B 14 (Old No. B 19)	6	17.25	3.565	$3\frac{9}{16}$	0.465	$1\frac{5}{32}$
		14.75	3.443	$3\frac{7}{16}$	0.343	$1\frac{1}{32}$
		12.5	3.330	$3\frac{21}{64}$	0.230	$1\frac{5}{64}$
B 15 (Old No. B 21)	5	14.75	3.284	$3\frac{9}{32}$	0.494	$\frac{1}{2}$
		12.25	3.137	$3\frac{9}{64}$	0.347	$1\frac{1}{32}$
		10.0	3.000	3	0.210	$1\frac{3}{64}$
B 16 (Old No. B 23)	4	10.5	2.870	$2\frac{7}{8}$	0.400	$1\frac{9}{32}$
		9.5	2.796	$2\frac{51}{64}$	0.326	$2\frac{1}{64}$
		8.5	2.723	$2\frac{23}{32}$	0.253	$\frac{1}{4}$
		7.7	2.660	$2\frac{21}{32}$	0.190	$\frac{3}{16}$
B 17 (Old No. B 77)	3	7.5	2.509	$2\frac{69}{64}$	0.349	$1\frac{1}{32}$
		6.5	2.411	$2\frac{13}{32}$	0.251	$\frac{1}{4}$
		5.7	2.330	$2\frac{21}{64}$	0.170	$1\frac{1}{64}$

H-BEAMS

H-BEAMS

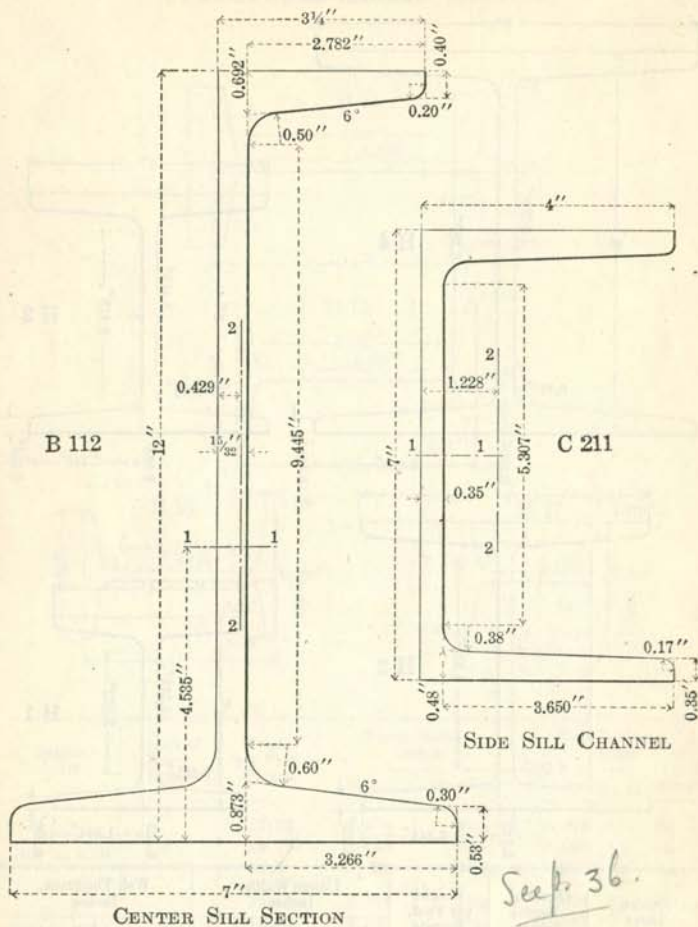


Section Index	Depth of Beam, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
H 4	8	37.7	8.125	8 ¹ / ₈	0.500	¹ / ₂
		34.3	8.000	8	0.375	³ / ₈
		32.6	7.938	7 ¹⁵ / ₁₆	0.313	⁵ / ₁₆
H 3	6	26.7	6.125	6 ¹ / ₈	0.438	⁷ / ₁₆
		24.1	6.000	6	0.313	⁵ / ₁₆
		22.8	5.938	5 ¹⁵ / ₁₆	0.250	¹ / ₄
H 2	5	18.9	5.000	5	0.313	⁵ / ₁₆
H 1	4	13.8	4.000	4	0.313	⁵ / ₁₆

Full information as to uses of H-Beams is given in pamphlet entitled "Steel Mine Timbers."

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CAR SILL SECTIONS

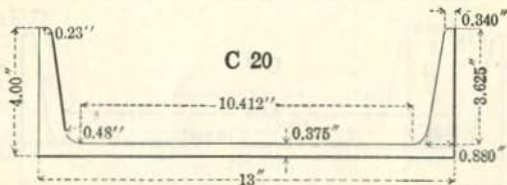
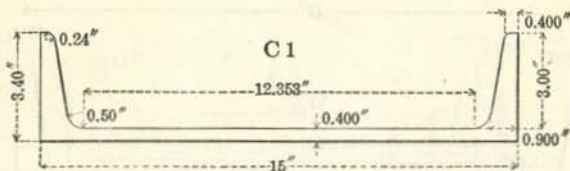
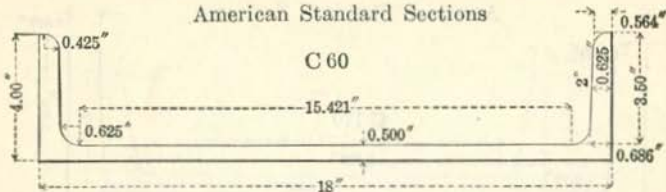


Section Index	Depth In.	Weight per Foot Lb.	Area In. ²	Width of Flange In.	Thick- ness of Web In.	Axis 1-1				Axis 2-2			
						I	r	S	x	I	r	S	y
B 112	12	40.3	11.72	3 3/4	15/32	238.1	4.51	31.9	4.54	21.8	1.36	5.9	4.54
C 211	7	18.8	5.48	4	0.35	42.9	2.80	12.2	3.50	8.3	1.23	3.0	1.23

Full information is given in pamphlet entitled "Carnegie Car Sill Sections."

STRUCTURAL CHANNELS

American Standard Sections

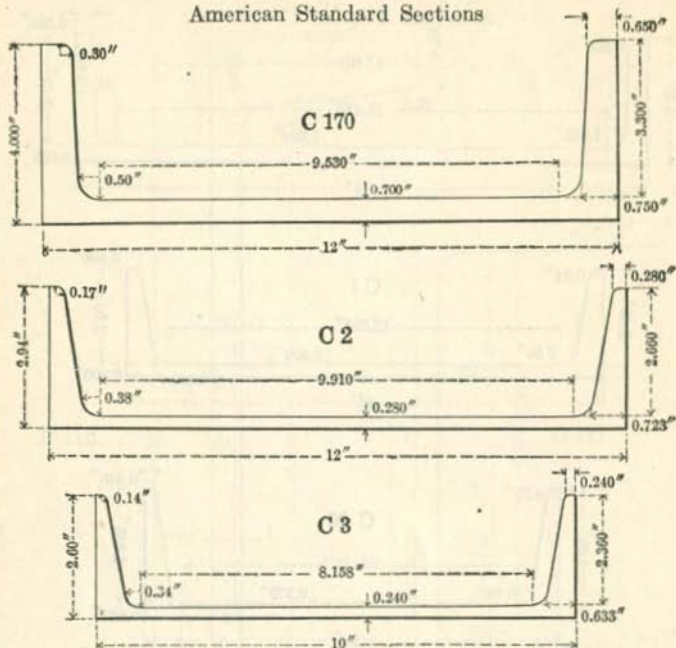


Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
†C 60	18	58.0	4.200	4 13/64	0.700	45/64
		51.9	4.100	4 7/32	0.600	19/32
		45.8	4.000	4	0.500	1/2
		42.7	3.950	3 61/64	0.450	29/64
C 1	15	55.0	3.814	3 13/64	0.814	131/64
		50.0	3.716	3 23/32	0.716	23/32
		45.0	3.618	3 5/8	0.618	5/8
		40.0	3.520	3 33/64	0.520	33/64
		35.0	3.422	3 27/64	0.422	27/64
33.9	3.400	3 13/32	0.400	13/32		
†C 20	13	50.0	4.412	4 13/32	0.787	25/32
		45.0	4.298	4 19/64	0.673	43/64
		40.0	4.185	4 3/16	0.560	9/16
		37.0	4.117	4 7/64	0.492	31/64
		35.0	4.072	4 5/64	0.447	29/64
31.8	4.000	4	0.375	3/8		

†C 60 is a Ship Building Channel (not an American Standard.) †C 20 is a Car Building Channel.

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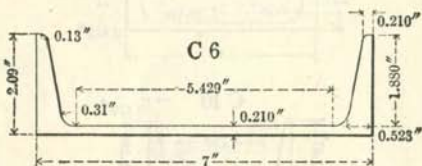
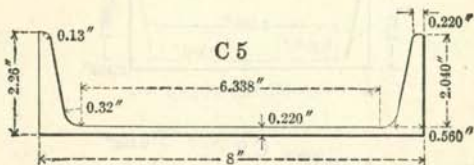
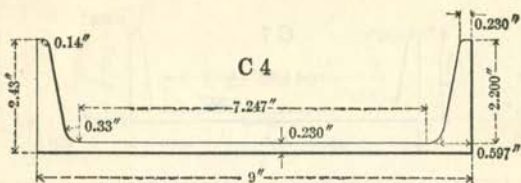
STRUCTURAL CHANNELS—Continued
American Standard Sections



Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
†C 170	12	50.0	4.135	$4\frac{9}{64}$	0.835	$\frac{53}{64}$
		48.6	4.100	$4\frac{9}{32}$	0.800	$\frac{51}{64}$
		46.6	4.050	$4\frac{3}{64}$	0.750	$\frac{5}{8}$
		44.5	4.000	4	0.700	$\frac{45}{64}$
		40.0	3.890	$3\frac{37}{64}$	0.590	$\frac{19}{32}$
	35.0	3.767	$3\frac{49}{64}$	0.467	$\frac{15}{32}$	
C 2	12	40.0	3.415	$3\frac{27}{64}$	0.755	$\frac{3}{4}$
		35.0	3.292	$3\frac{19}{64}$	0.632	$\frac{5}{8}$
		30.0	3.170	$3\frac{11}{64}$	0.510	$\frac{23}{64}$
		25.0	3.047	$3\frac{3}{64}$	0.387	$\frac{25}{64}$
		20.7	2.940	$2\frac{19}{16}$	0.280	$\frac{9}{32}$
C 3	10	35.0	3.180	$3\frac{3}{16}$	0.820	$\frac{13}{16}$
		30.0	3.033	$3\frac{1}{32}$	0.673	$\frac{43}{64}$
		25.0	2.886	$2\frac{57}{64}$	0.526	$\frac{17}{32}$
		20.0	2.739	$2\frac{47}{64}$	0.379	$\frac{3}{8}$
		15.3	2.600	$2\frac{19}{32}$	0.240	$\frac{15}{64}$

†C 170 is a Car Building Channel (not an American Standard.)

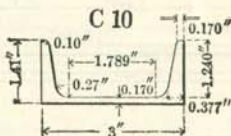
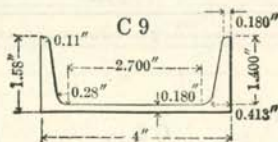
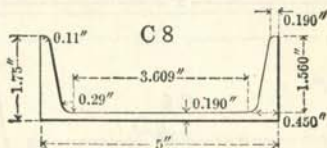
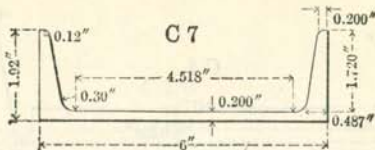
CHANNELS

STRUCTURAL CHANNELS—Continued
American Standard Sections

Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
C 4	9	25.0	2.812	$2\frac{13}{16}$	0.612	$\frac{39}{64}$
		20.0	2.648	$2\frac{41}{64}$	0.448	$\frac{29}{64}$
		15.0	2.485	$2\frac{31}{64}$	0.285	$\frac{9}{32}$
		13.4	2.430	$2\frac{7}{16}$	0.230	$\frac{15}{64}$
C 5	8	21.25	2.619	$2\frac{5}{8}$	0.579	$\frac{37}{64}$
		18.75	2.527	$2\frac{17}{32}$	0.487	$\frac{31}{64}$
		16.25	2.435	$2\frac{7}{16}$	0.395	$\frac{25}{64}$
		13.75	2.343	$2\frac{11}{32}$	0.303	$\frac{19}{64}$
		11.5	2.260	$2\frac{17}{64}$	0.220	$\frac{7}{32}$
C 6	7	19.75	2.509	$2\frac{33}{64}$	0.629	$\frac{5}{8}$
		17.25	2.404	$2\frac{13}{32}$	0.524	$\frac{17}{32}$
		14.75	2.299	$2\frac{19}{64}$	0.419	$\frac{27}{64}$
		12.25	2.194	$2\frac{3}{16}$	0.314	$\frac{5}{16}$
		9.8	2.090	$2\frac{3}{32}$	0.210	$\frac{13}{64}$

CARNEGIE STEEL COMPANY

STRUCTURAL CHANNELS—Concluded
American Standard Sections

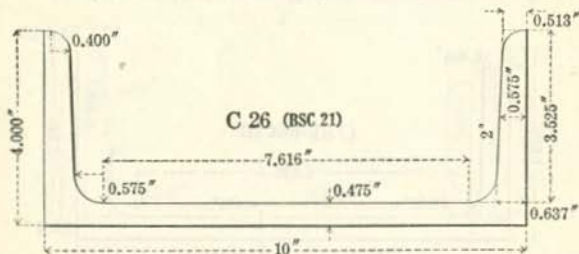
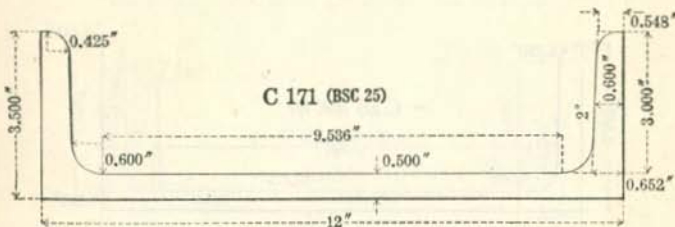
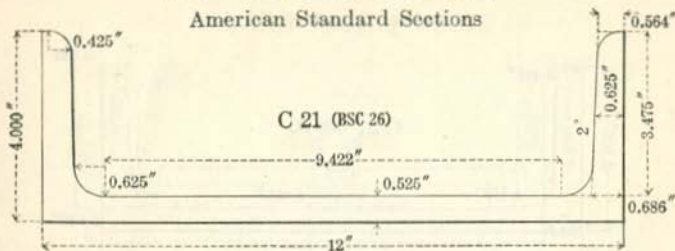


Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
C 7	6	15.5	2.279	$2\frac{9}{32}$	0.559	$\frac{9}{16}$
		13.0	2.157	$2\frac{25}{32}$	0.437	$\frac{7}{16}$
		10.5	2.034	$2\frac{1}{32}$	0.314	$\frac{5}{16}$
		8.2	1.920	$1\frac{5}{16}$	0.200	$\frac{1}{8}$
C 8	5	11.5	2.032	$2\frac{1}{32}$	0.472	$1\frac{15}{32}$
		9.0	1.885	$1\frac{57}{64}$	0.325	$2\frac{1}{64}$
		6.7	1.750	$1\frac{3}{4}$	0.190	$\frac{3}{16}$
C 9	4	7.25	1.720	$1\frac{23}{32}$	0.320	$\frac{5}{16}$
		6.25	1.647	$1\frac{41}{64}$	0.247	$\frac{1}{4}$
		5.4	1.580	$1\frac{37}{64}$	0.180	$\frac{3}{16}$
C 10 (Old No. C 72)	3	6.0	1.596	$1\frac{19}{32}$	0.356	$2\frac{3}{64}$
		5.0	1.498	$1\frac{1}{2}$	0.258	$\frac{1}{4}$
		4.1	1.410	$1\frac{13}{32}$	0.170	$1\frac{1}{64}$

CHANNELS

SHIP BUILDING CHANNELS

American Standard Sections

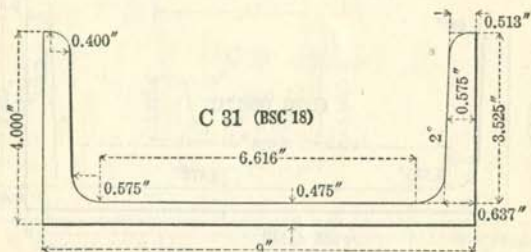
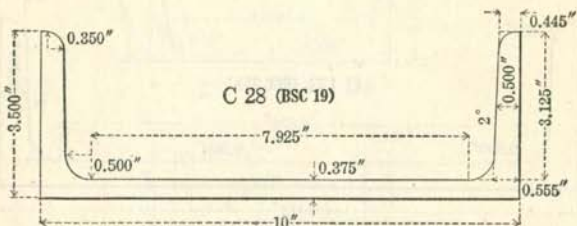
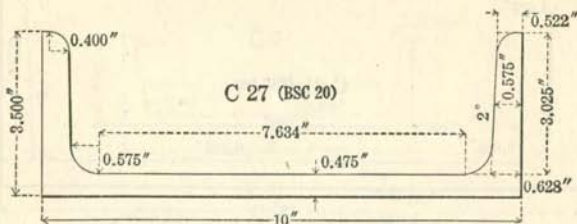


Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
C 21 (BSC 26)	12	44.7	4.200	4 ¹³ / ₆₄	0.725	23 ⁵ / ₃₂
		40.6	4.100	4 ⁹ / ₃₂	0.625	9 ⁵ / ₁₆
		36.5	4.000	4	0.525	17 ⁵ / ₃₂
		34.5	3.950	3 ³¹ / ₆₄	0.475	15 ⁵ / ₃₂
C 171 (BSC 25)	12	41.1	3.700	3 ⁴⁵ / ₆₄	0.700	45 ⁵ / ₆₄
		37.0	3.600	3 ¹⁹ / ₃₂	0.600	19 ⁵ / ₃₂
		32.9	3.500	3¹/₂	0.500	1 ¹ / ₂
		30.9	3.450	3 ²⁹ / ₆₄	0.450	29 ⁵ / ₆₄
C 26 (BSC 21)	10	37.0	4.200	4 ¹³ / ₆₄	0.675	43 ⁵ / ₆₄
		33.6	4.100	4 ⁹ / ₃₂	0.575	37 ⁵ / ₆₄
		30.2	4.000	4	0.475	15 ⁵ / ₃₂
		28.5	3.950	3 ³¹ / ₆₄	0.425	27 ⁵ / ₆₄

Dimensions and properties of the British Standard Sections are indicated in **bold type**.

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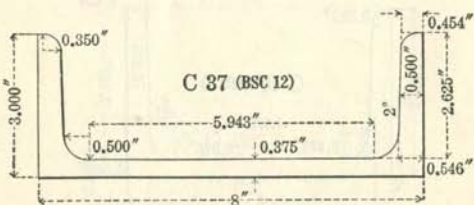
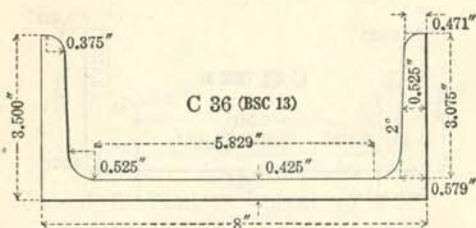
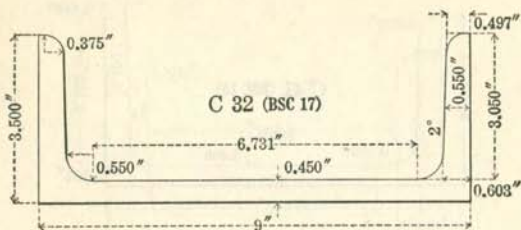
SHIP BUILDING CHANNELS—Continued
American Standard Sections



Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
C 27 (BSC 20)	10	35.1	3.700	$3\frac{45}{64}$	0.675	$\frac{43}{64}$
		31.7	3.600	$3\frac{13}{32}$	0.575	$\frac{27}{64}$
		28.3	3.500	$3\frac{1}{2}$	0.475	$\frac{15}{32}$
		26.6	3.450	$3\frac{29}{64}$	0.425	$\frac{27}{64}$
		24.9	3.400	$3\frac{13}{32}$	0.375	$\frac{3}{8}$
C 28 (BSC 19)	10	25.3	3.550	$3\frac{25}{64}$	0.425	$\frac{27}{64}$
		23.6	3.500	$3\frac{1}{2}$	0.375	$\frac{3}{8}$
		21.9	3.450	$3\frac{29}{64}$	0.325	$\frac{21}{64}$
C 31 (BSC 18)	9	34.7	4.200	$4\frac{13}{64}$	0.675	$\frac{43}{64}$
		31.7	4.100	$4\frac{3}{32}$	0.575	$\frac{27}{64}$
		28.6	4.000	4	0.475	$\frac{15}{32}$
		27.1	3.950	$3\frac{91}{64}$	0.425	$\frac{27}{64}$

Dimensions and properties of the British Standard Sections are indicated in **bold type**.

CHANNELS

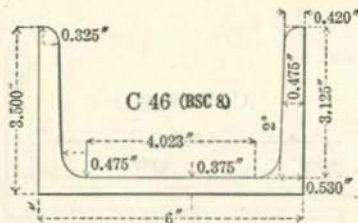
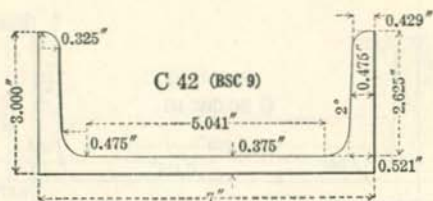
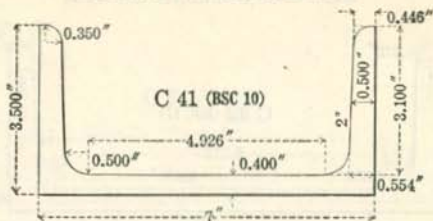
SHIP BUILDING CHANNELS—Continued
American Standard Sections

Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
C 32 (BSC 17)	9	31.6	3.700	$3\frac{45}{64}$	0.650	$2\frac{1}{32}$
		28.5	3.600	$3\frac{19}{32}$	0.550	$3\frac{5}{64}$
		25.4	3.500	$3\frac{1}{2}$	0.450	$2\frac{9}{64}$
		23.9	3.450	$3\frac{29}{64}$	0.400	$1\frac{3}{32}$
C 36 (BSC 13)	8	28.2	3.700	$3\frac{45}{64}$	0.625	$\frac{5}{8}$
		25.5	3.600	$3\frac{19}{32}$	0.525	$1\frac{7}{32}$
		22.8	3.500	$3\frac{1}{2}$	0.425	$2\frac{7}{64}$
		21.4	3.450	$3\frac{29}{64}$	0.375	$\frac{3}{8}$
C 37 (BSC 12)	8	25.5	3.225	$3\frac{7}{32}$	0.600	$1\frac{9}{32}$
		22.7	3.125	$3\frac{1}{8}$	0.500	$\frac{1}{2}$
		20.0	3.025	$3\frac{1}{32}$	0.400	$1\frac{3}{32}$
		19.3	3.000	3	0.375	$\frac{3}{8}$
		18.7	2.975	$2\frac{31}{32}$	0.350	$1\frac{1}{32}$

Dimensions and properties of the British Standard Sections are indicated in bold type.

CARNEGIE STEEL COMPANY

SHIP BUILDING CHANNELS—Continued
American Standard Sections

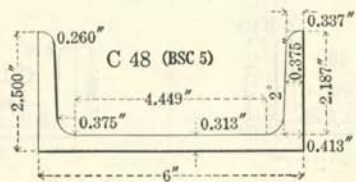
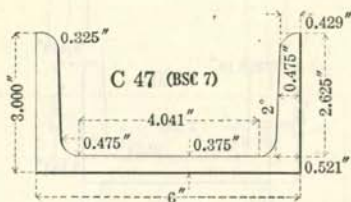
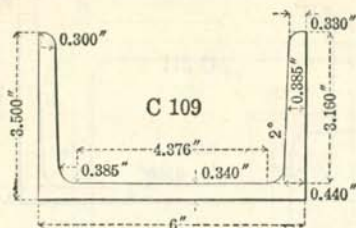


Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
C 41 (BSC 10)	7	25.0	3.700	$3\frac{45}{64}$	0.600	$\frac{19}{32}$
		22.7	3.600	$3\frac{19}{32}$	0.500	$\frac{1}{2}$
		20.3	3.500	$3\frac{1}{2}$	0.400	$\frac{13}{32}$
		19.1	3.450	$3\frac{29}{64}$	0.350	$\frac{11}{32}$
C 42 (BSC 9)	7	20.0	3.100	$3\frac{3}{32}$	0.475	$\frac{15}{32}$
		17.6	3.000	3	0.375	$\frac{3}{8}$
		16.4	2.950	$2\frac{61}{64}$	0.325	$2\frac{1}{64}$
C 46 (BSC 8)	6	22.0	3.700	$3\frac{45}{64}$	0.575	$\frac{27}{64}$
		20.0	3.600	$3\frac{19}{32}$	0.475	$\frac{15}{32}$
		18.0	3.500	$3\frac{1}{2}$	0.375	$\frac{3}{8}$
		16.9	3.450	$3\frac{29}{64}$	0.325	$2\frac{1}{64}$

Dimensions and properties of the British Standard Sections are indicated in **bold type**.

CHANNELS

SHIP BUILDING CHANNELS—Concluded
American Standard Sections

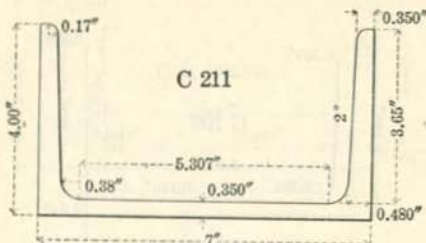


Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
C 109	6	15.3	3.500	3½	0.340	11/32
C 47	6	16.3	3.000	3	0.375	3/8
(BSC 7)		15.1	2.938	2½ 5/16	0.313	5/16
C 48	6	13.3	2.563	2 9/16	0.375	3/8
(BSC 5)		12.0	2.500	2½	0.313	5/16

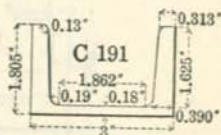
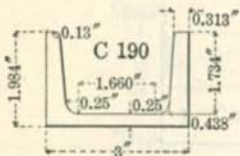
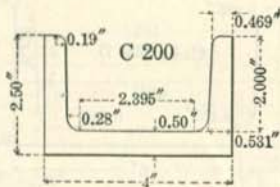
Dimensions and properties of the British Standard Sections are indicated in **bold type**.

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MISCELLANEOUS CAR BUILDING CHANNELS



new

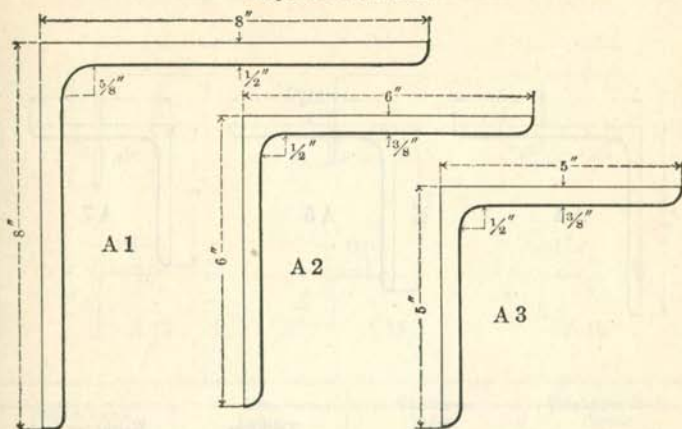


Section Index	Depth of Channel, Inches	Weight per Foot, Pounds	Flange Width, Inches		Web Thickness, Inches	
			Decimal	Fractional	Decimal	Fractional
C 211	7	18.8	4.000	4	0.350	$1\frac{1}{2}$
*C 200	4	13.8	2.500	$2\frac{1}{2}$	0.500	$\frac{1}{2}$
*C 190	3	7.1	1.984	$1\frac{3}{4}$	0.250	$\frac{1}{4}$
*C 191	3	6.5 5.8	1.875 1.805	$1\frac{7}{8}$ $1\frac{13}{16}$	0.250 0.180	$\frac{1}{4}$ $\frac{3}{16}$

*Furnished only by special arrangement.

ANGLES

EQUAL ANGLES

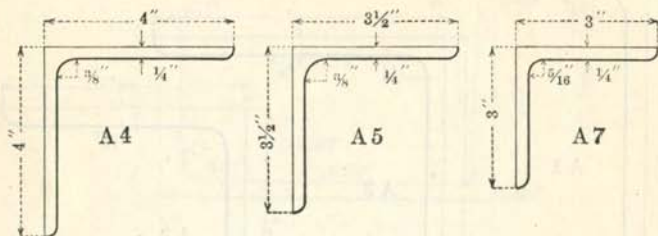


Section Index	Size, Inches	Thickness, Inches	Weight per Foot, Pounds
A 1	8 x 8	1 1/4	56.9
		1 1/4	54.0
		1	51.0
		1 5/16	48.1
		7/8	45.0
		13/16	42.0
		3/4	38.9
		1 1/4	35.8
		5/8	32.7
		9/16	29.6
1/2	26.4		
A 2	6 x 6	1	37.4
		1 5/16	35.3
		7/8	33.1
		13/16	31.0
		3/4	28.7
		1 1/4	26.5
		5/8	24.2
		9/16	21.9
		1/2	19.6
		7/16	17.2
3/8	14.9		
A 3	5 x 5	* 1	30.6
		* 1 5/16	28.9
		* 7/8	27.2
		* 13/16	25.4
		* 3/4	23.6
		* 1 1/4	21.8
		* 5/8	20.0
		* 9/16	18.1
* 1/2	16.2		
* 7/16	14.3		
* 3/8	12.3		

Angles marked * are special.

CARNEGIE STEEL COMPANY

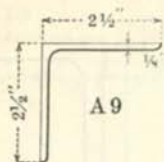
EQUAL ANGLES—Continued



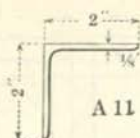
Section Index	Size, Inches	Thickness, Inches	Weight per Foot, Pounds
A 4	4 x 4	*13/16	19.9
		3/4	18.5
		11/16	17.1
		5/8	15.7
		9/16	14.3
		1/2	12.8
		7/16	11.3
		3/8	9.8
		5/16	8.2
		* 1/4	6.6
A 5	3 1/2 x 3 1/2	*13/16	17.1
		* 3/4	16.0
		*11/16	14.8
		5/8	13.6
		9/16	12.4
		1/2	11.1
		7/16	9.8
		3/8	8.5
A 7	3 x 3	5/16	7.2
		* 1/4	5.8
		* 5/8	11.5
		* 9/16	10.4
		1/2	9.4
		7/16	8.3
3/8	7.2		
5/16	6.1		
1/4	4.9		

Angles marked * are special.

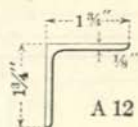
EQUAL ANGLES—Concluded



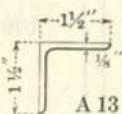
A 9



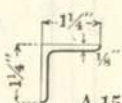
A 11



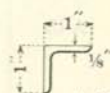
A 12



A 13



A 15



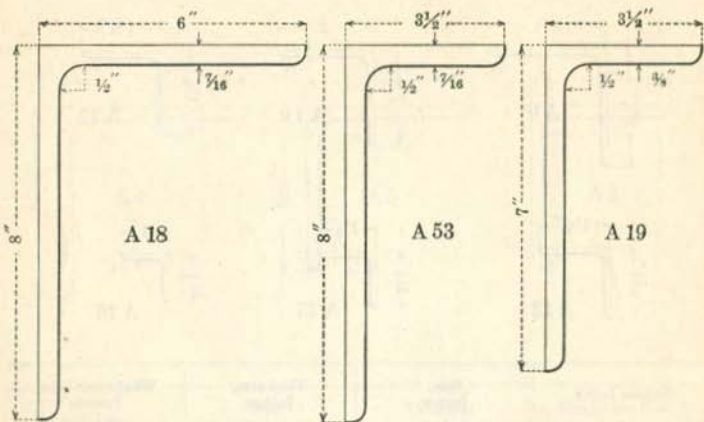
A 16

Section Index	Size, Inches	Thickness, Inches	Weight per Foot, Pounds
A 9	2 1/2 x 2 1/2	* 1/2	7.7
		7/16	6.8
		3/8	5.9
		5/16	5.0
		1/4	4.1
		3/16	3.07
		* 1/8	2.08
A 11	2 x 2	* 7/16	5.3
		3/8	4.7
		5/16	3.92
		1/4	3.19
		3/16	2.44
		* 1/8	1.65
A 12	1 3/4 x 1 3/4	* 7/16	4.6
		* 3/8	3.99
		* 5/16	3.39
		* 1/4	2.77
		* 3/16	2.12
		* 1/8	1.44
A 13	1 1/2 x 1 1/2	* 3/8	3.35
		5/16	2.86
		1/4	2.34
		3/16	1.80
		1/8	1.23
A 15	1 1/4 x 1 1/4	* 5/16	2.33
		* 1/4	1.92
		* 3/16	1.48
		* 1/8	1.01
A 16	1 x 1	* 1/4	1.49
		* 3/16	1.16
		* 1/8	0.80

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UNEQUAL ANGLES

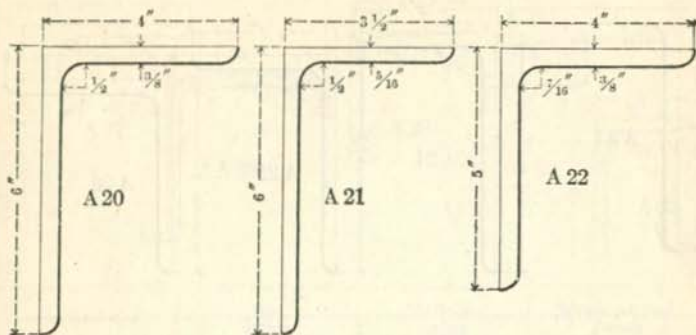


Section Index	Size, Inches	Thickness, Inches	Weight per Foot, Pounds
A 18	8 x 6	*1	44.2
		* 15/16	41.7
		* 7/8	39.1
		* 13/16	36.5
		* 5/8	33.8
		* 11/16	31.2
		* 5/8	28.5
		* 9/16	25.7
		* 1/2	23.0
		* 7/16	20.2
A 53	8 x 3 1/2	*1	35.7
		* 15/16	33.7
		* 7/8	31.7
		* 13/16	29.6
		* 5/8	27.5
		* 11/16	25.3
		* 5/8	23.2
		* 9/16	21.0
		* 1/2	18.7
		* 7/16	16.5
A 19	7 x 3 1/2	*1	32.3
		* 15/16	30.5
		* 7/8	28.7
		* 13/16	26.8
		* 5/8	24.9
		* 11/16	23.0
		* 5/8	21.0
		* 9/16	19.1
		* 1/2	17.0
		* 7/16	15.0
* 5/8	13.0		

Angles marked * are special.

ANGLES

UNEQUAL ANGLES—Continued

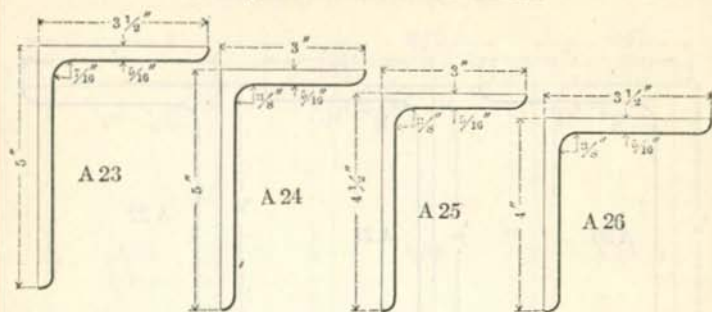


Section Index	Size, Inches	Thickness, Inches	Weight per Foot, Pounds
A 20	6 x 4	*1	30.6
		* 15/16	28.9
		7/8	27.2
		13/16	25.4
		5/8	23.6
		11/16	21.8
		5/8	20.0
		9/16	18.1
		1/2	16.2
		7/16	14.3
A 21	6 x 3 1/2	3/8	12.3
		*1	28.9
		* 15/16	27.3
		7/8	25.7
		13/16	24.0
		5/8	22.4
		11/16	20.6
		5/8	18.9
		9/16	17.1
		1/2	15.3
A 22	5 x 4	7/16	13.5
		5/8	11.7
		* 5/16	9.8
		* 7/8	24.2
		* 13/16	22.7
		* 3/4	21.1
		* 11/16	19.5
		* 5/8	17.8
		* 9/16	16.2
		* 1/2	14.5
* 7/16	12.8		
* 3/8	11.0		

Angles marked * are special.

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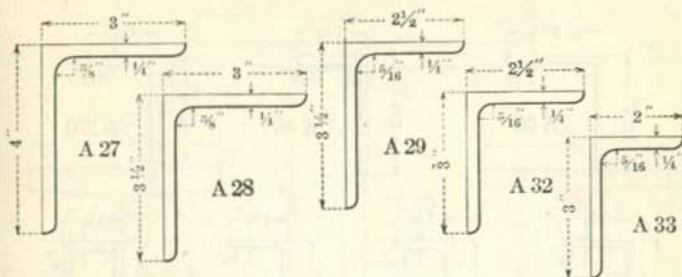
UNEQUAL ANGLES—Continued



Section Index	Size, Inches	Thickness, Inches	Weight per Foot, Pounds
A 23	5 x 3 1/2	* 7/8	22.7
		* 13/16	21.3
		3/4	19.8
		11/16	18.3
		5/8	16.8
		9/16	15.2
		1/2	13.6
		7/16	12.0
		3/8	10.4
		5/16	8.7
A 24	5 x 3	* 13/16	19.9
		* 3/4	18.5
		11/16	17.1
		5/8	15.7
		9/16	14.3
		1/2	12.8
		7/16	11.3
		3/8	9.8
A 25	4 1/2 x 3	5/16	8.2
		* 13/16	18.5
		* 3/4	17.3
		* 11/16	16.0
		* 5/8	14.7
		* 9/16	13.3
		* 1/2	11.9
		* 7/16	10.6
A 26	4 x 3 1/2	* 3/8	9.1
		* 5/16	7.7
		* 13/16	18.5
		* 3/4	17.3
		* 11/16	16.0
		* 5/8	14.7
		* 9/16	13.3
		* 1/2	11.9

Angles marked * are special.

UNEQUAL ANGLES—Continued

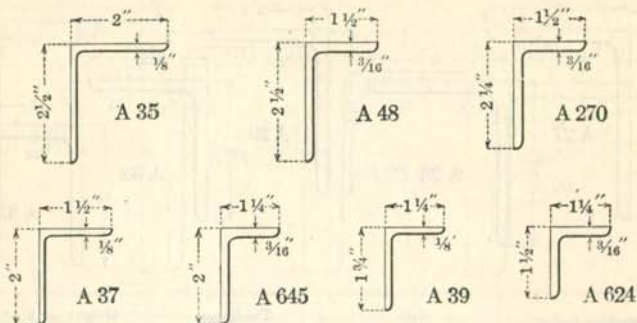


Section Index	Size, Inches	Thickness, Inches	Weight per Foot, Pounds
A 27	4 x 3	*13/16	17.1
		*3/4	16.0
		*11/16	14.8
		5/8	13.6
		9/16	12.4
		1/2	11.1
		7/16	9.8
		3/8	8.5
A 28	3 1/2 x 3	5/16	7.2
		*1/4	5.8
		*13/16	15.8
		*3/4	14.7
		*11/16	13.6
		*5/8	12.5
		9/16	11.4
		1/2	10.2
A 29	3 1/2 x 2 1/2	7/16	9.1
		3/8	7.9
		5/16	6.6
		*1/4	5.4
		*11/16	12.5
		*5/8	11.5
		*9/16	10.4
		1/2	9.4
A 32	3 x 2 1/2	7/16	8.3
		3/8	7.2
		5/16	6.1
		1/4	4.9
		*9/16	9.5
		*1/2	8.5
		7/16	7.6
		3/8	6.6
A 33	3 x 2	5/16	5.6
		1/4	4.5
		*1/2	7.7
		*7/16	6.8
		*3/8	5.9
		*5/16	5.0
		*1/4	4.1

Angles marked * are special.

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UNEQUAL ANGLES—Concluded

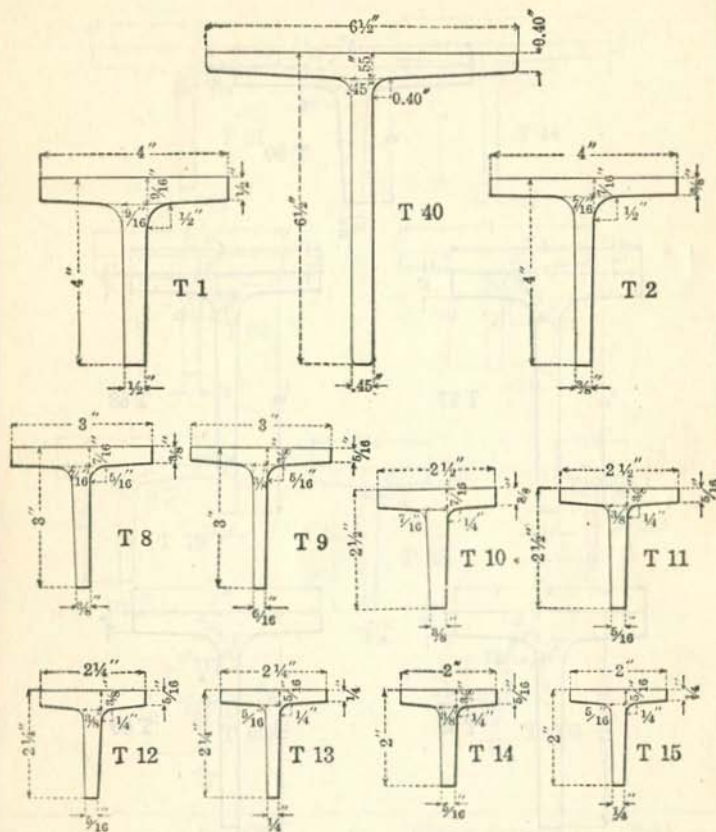


Section Index	Size, Inches	Thickness, Inches	Weight per Foot, Pounds
A 35	2 1/2 x 2	* 1/2	6.8
		* 7/16	6.1
		3/8	5.3
		5/16	4.5
		1/4	3.62
		3/16	2.75
A 48	2 1/2 x 1 1/2	* 1/8	1.86
		* 5/16	3.92
		* 1/4	3.19
		* 3/16	2.44
A 270	2 1/4 x 1 1/2	* 1/2	5.6
		* 7/16	5.0
		* 3/8	4.4
		* 5/16	3.66
		* 1/4	2.98
		* 3/16	2.28
A 37	2 x 1 1/2	* 3/8	3.99
		* 5/16	3.39
		* 1/4	2.77
		* 3/16	2.12
A 645	2 x 1 1/4	* 1/8	1.44
		* 1/4	2.55
		* 3/16	1.96
A 39	1 3/4 x 1 1/4	* 1/4	2.34
		* 5/16	1.80
		* 1/8	1.23
A 624	1 1/2 x 1 1/4	* 5/16	2.59
		* 1/4	2.13
		* 3/16	1.64

Angles marked * are special.

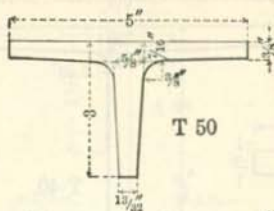
TEES

EQUAL TEES

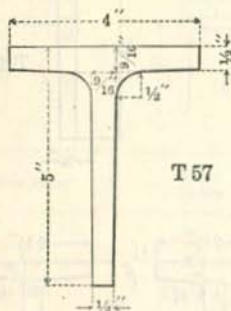


Section Index	Size, Inches		Thickness, Inches		Weight per Foot, Pounds
	Flange	Stem	Flange	Stem	
T 40	6 1/2	6 1/2	0.40 to 0.55	0.45	19.8
T 1	4	4	1/2 to 5/8	1/2 to 5/8	13.5
T 2	4	4	3/8 to 7/16	3/8 to 7/16	10.5
T 8	3	3	3/8 to 7/16	3/8 to 7/16	7.8
T 9	3	3	5/16 to 3/8	5/16 to 3/8	6.7
T 10	2 1/4	2 1/4	3/8 to 7/16	3/8 to 7/16	6.4
T 11	2 1/4	2 1/4	5/16 to 3/8	5/16 to 3/8	5.5
T 12	2 1/4	2 1/4	5/16 to 3/8	5/16 to 3/8	4.9
T 13	2 1/4	2 1/4	1/4 to 5/16	1/4 to 5/16	4.1
T 14	2	2	5/16 to 3/8	5/16 to 3/8	4.3
T 15	2	2	1/4 to 5/16	1/4 to 5/16	3.56

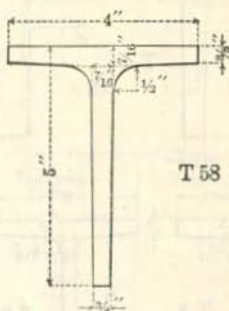
UNEQUAL TEES



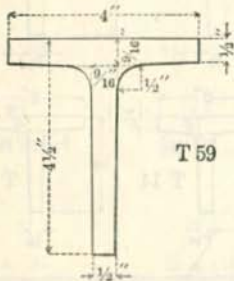
T 50



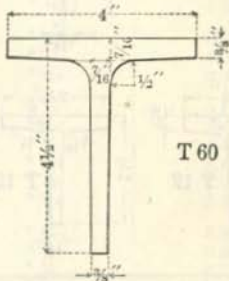
T 57



T 58



T 59

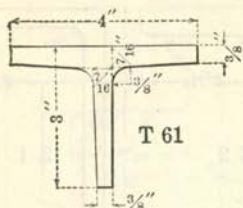


T 60

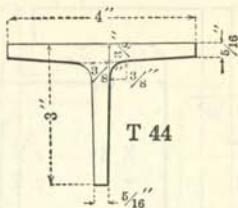
Section Index	Size, Inches		Thickness, Inches		Weight per Foot, Pounds
	Flange	Stem	Flange	Stem	
†T 50	5	3	3/8 to 3/16	1 3/8 to 3/8	11.5
T 57	4	5	1/2 to 3/16	1/2 to 3/16	15.3
T 58	4	5	3/8 to 3/16	3/8 to 3/16	11.9
T 59	4	4 1/2	1/2 to 3/16	1/2 to 3/16	14.4
T 60	4	4 1/2	3/8 to 3/16	3/8 to 3/16	11.2

† T 50 can be rolled with flange 1/2" to 3/16", and stem 3 1/8"; weight 13.6 lbs. per foot.

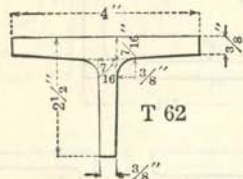
UNEQUAL TEES—Concluded



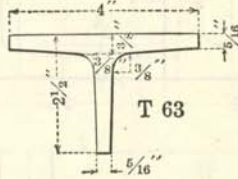
T 61



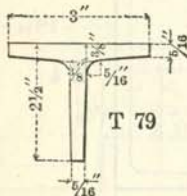
T 44



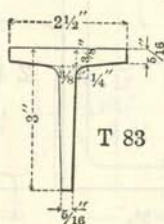
T 62



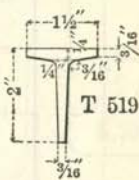
T 63



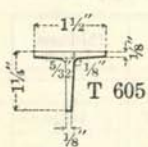
T 79



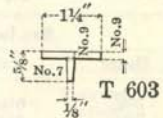
T 83



T 519



T 605



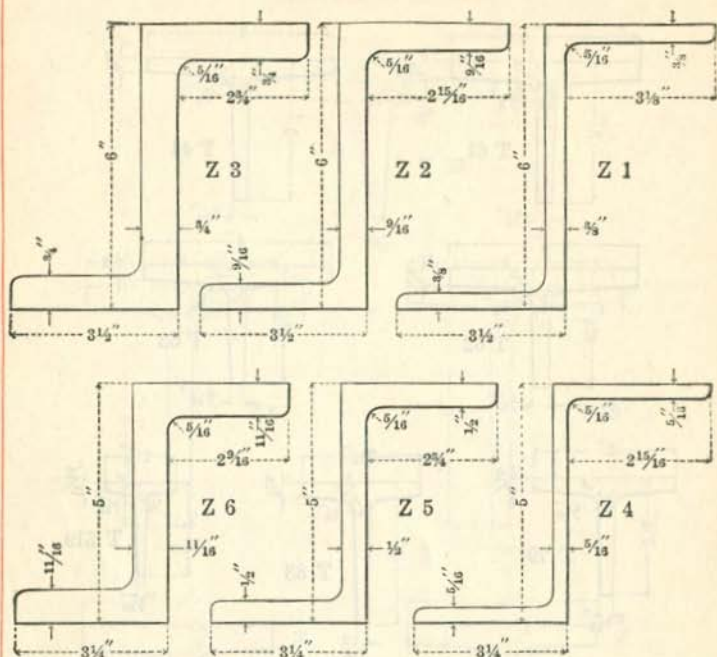
T 603

Section Index	Size, Inches		Thickness, Inches		Weight per Foot, Pounds
	Flange	Stem	Flange	Stem	
T 61	4	3	3/8 to 7/16	3/8 to 7/16	9.2
T 44	4	3	5/16 to 3/8	5/16 to 3/8	7.8
T 62	4	2 1/2	3/8 to 7/16	3/8 to 7/16	8.5
T 63	4	2 1/2	5/16 to 3/8	5/16 to 3/8	7.2
T 79	3	2 1/2	5/16 to 3/8	5/16 to 3/8	6.1
T 83	2 1/2	3	5/16 to 3/8	5/16 to 3/8	6.1
T 519	1 1/2	2	3/16 to 1/4	3/16 to 1/4	2.45
T 605	1 1/2	1 1/4	1/8 to 5/32	1/8 to 5/32	1.25
* T 603	1 1/4	5/8	No. 9	1/8 to No. 7	0 88

* Furnished only by special arrangement.

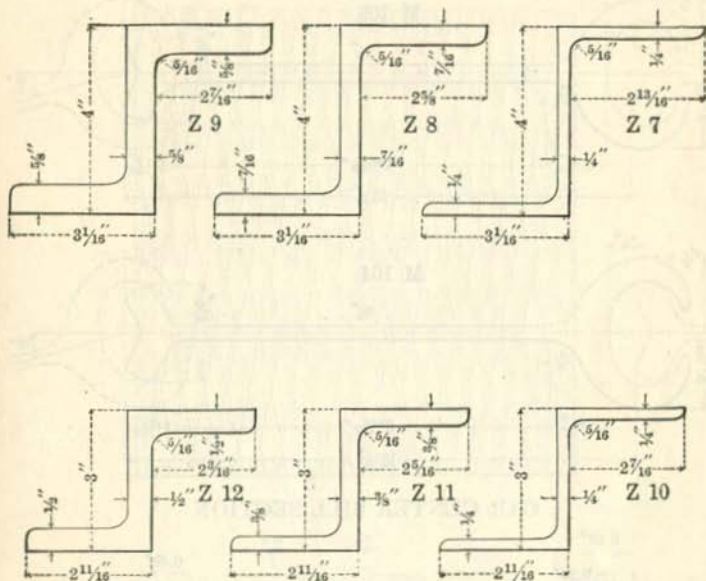
CARNEGIE STEEL COMPANY

ZEES



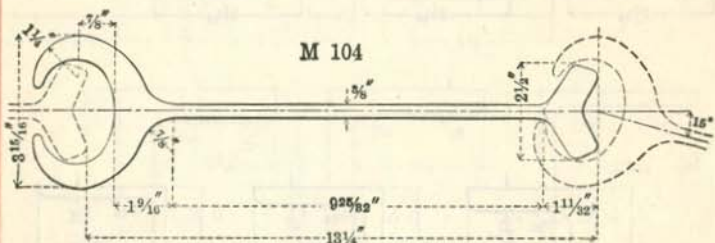
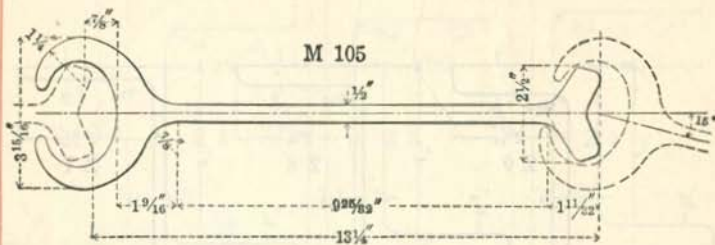
Section Index	Size, Inches			Thickness, Inches	Weight per Foot, Pounds
	Flange	Web	Flange		
Z 3	3 $\frac{3}{8}$	6 $\frac{1}{4}$	3 $\frac{3}{8}$	$\frac{7}{8}$	34.6
	3 $\frac{3}{16}$	6 $\frac{1}{8}$	3 $\frac{3}{16}$	1 $\frac{3}{16}$	32.0
	3 $\frac{1}{2}$	6	3 $\frac{1}{2}$	$\frac{3}{4}$	29.4
Z 2	3 $\frac{3}{8}$	6 $\frac{1}{8}$	3 $\frac{3}{8}$	1 $\frac{1}{16}$	28.1
	3 $\frac{3}{16}$	6 $\frac{1}{16}$	3 $\frac{3}{16}$	$\frac{5}{8}$	25.4
	3 $\frac{1}{2}$	6	3 $\frac{1}{2}$	$\frac{9}{16}$	22.8
Z 1	3 $\frac{3}{8}$	6 $\frac{1}{4}$	3 $\frac{3}{8}$	$\frac{1}{2}$	21.1
	3 $\frac{3}{16}$	6 $\frac{1}{8}$	3 $\frac{3}{16}$	$\frac{7}{16}$	18.4
	3 $\frac{1}{2}$	6	3 $\frac{1}{2}$	$\frac{3}{8}$	15.7
Z 6	3 $\frac{3}{8}$	5 $\frac{1}{4}$	3 $\frac{3}{8}$	1 $\frac{3}{16}$	28.4
	3 $\frac{3}{16}$	5 $\frac{1}{8}$	3 $\frac{3}{16}$	$\frac{3}{4}$	26.0
	3 $\frac{1}{4}$	5	3 $\frac{1}{4}$	1 $\frac{1}{16}$	23.7
Z 5	3 $\frac{3}{8}$	5 $\frac{1}{4}$	3 $\frac{3}{8}$	$\frac{5}{8}$	22.6
	3 $\frac{3}{16}$	5 $\frac{1}{8}$	3 $\frac{3}{16}$	$\frac{9}{16}$	20.2
	3 $\frac{1}{4}$	5	3 $\frac{1}{4}$	$\frac{1}{2}$	17.9
Z 4	3 $\frac{3}{8}$	5 $\frac{1}{4}$	3 $\frac{3}{8}$	$\frac{7}{16}$	16.4
	3 $\frac{3}{16}$	5 $\frac{1}{16}$	3 $\frac{3}{16}$	$\frac{3}{8}$	14.0
	3 $\frac{1}{4}$	5	3 $\frac{1}{4}$	$\frac{5}{16}$	11.6

ZEEES—Concluded

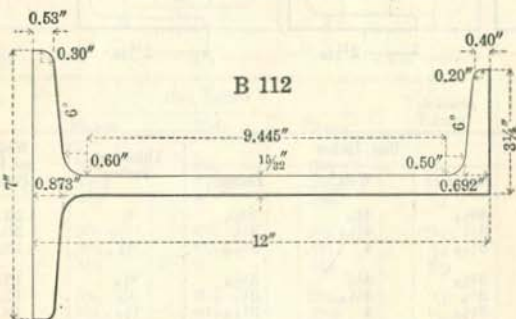


Section Index	Size, Inches			Thickness, Inches	Weight per Foot, Pounds
	Flange	Web	Flange		
Z 9	3 ³ / ₁₆	4 ¹ / ₂	3 ³ / ₁₆	3 ₄	23.0
	3 ¹ / ₂	4 ¹ / ₁₆	3 ¹ / ₂	1 ¹ / ₁₆	20.9
	3 ¹ / ₁₆	4	3 ¹ / ₁₆	5 ₈	18.9
Z 8	3 ³ / ₁₆	4 ¹ / ₂	3 ³ / ₁₆	9 ₁₆	18.0
	3 ¹ / ₂	4 ¹ / ₁₆	3 ¹ / ₂	1 ₂	15.9
	3 ¹ / ₁₆	4	3 ¹ / ₁₆	7 ₁₆	13.8
Z 7	3 ³ / ₁₆	4 ¹ / ₂	3 ³ / ₁₆	3 ₈	12.5
	3 ¹ / ₂	4 ¹ / ₁₆	3 ¹ / ₂	5 ₁₆	10.3
	3 ¹ / ₁₆	4	3 ¹ / ₁₆	1 ₄	8.2
Z 12	2 ³ / ₄	3 ¹ / ₁₆	2 ³ / ₄	9 ₁₆	14.3
	2 ¹ / ₁₆	3	2 ¹ / ₁₆	1 ₂	12.6
Z 11	2 ³ / ₄	3 ¹ / ₁₆	2 ³ / ₄	7 ₁₆	11.5
	2 ¹ / ₁₆	3	2 ¹ / ₁₆	3 ₈	9.8
Z 10	2 ³ / ₄	3 ¹ / ₁₆	2 ³ / ₄	5 ₁₆	8.5
	2 ¹ / ₁₆	3	2 ¹ / ₁₆	1 ₄	6.7

UNITED STATES STEEL SHEET PILING



CAR CENTER SILL SECTION

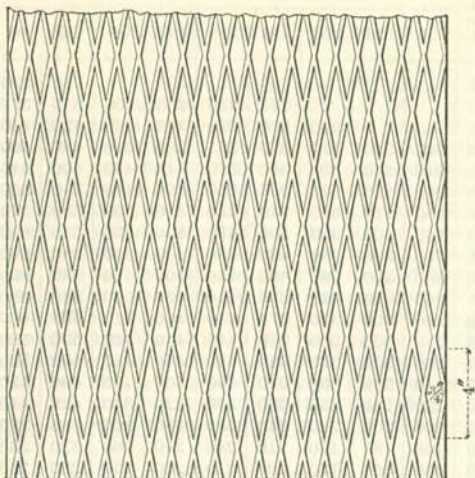


Section Index	Width, Inches	Web Thickness, Inches	Weight per Foot, Pounds
M 105	13 1/4	1/4	42.5
M 104	13 1/4	3/8	38
B 112	7.3 1/4	1 5/32	40.3

Full information as to the properties and uses of these sections is given in separate pamphlets entitled "Steel Sheet Piling" and "Carnegie Car Sill Sections."

FLOOR PLATES

CHECKERED PLATES



Section at Rib

Section Index	Thick-ness, Inches	Width and Length, Inches						Weight per Sq. Foot, Pounds
		Over 6 to 12	Over 12 to 24	Over 24 to 36	Over 36 to 48	Over 48 to 60	Over 60 to 66	
M 56	$\frac{3}{4}$...	120	180	280	264	240	31.6
M 55	$\frac{5}{8}$...	180	200	300	300	260	26.5
M 54	$\frac{1}{2}$	120	240	240	320	360	300	21.4
M 53	$\frac{7}{16}$	120	240	240	340	360	300	18.9
M 52	$\frac{3}{8}$	120	240	300	340	360	300	16.3
M 51	$\frac{5}{16}$	120	240	300	320	360	300	13.8
M 50	$\frac{1}{4}$	120	240	300	320	360	240	11.2
M 49	$\frac{3}{16}$	120	240	300	320	360	240	8.7

Checkered plates of greater lengths than shown in above table may be submitted for special consideration.

CARNEGIE STEEL COMPANY

RECTANGULAR AND CIRCULAR PLATES—Carbon Steel
SHEARED PLATES, THREE-SIXTEENTH INCH AND OVER, EXTREME SIZES

Thick- ness, Inches	Weight, Lbs. per Sq. Ft.	Widths and Lengths in Inches										Diam., Inches
		128	126	120	114	108	102	96	90	84	78	
5/16	7.65								270	320	345	90
3/8	10.20				175	250	280	300	330	375	400	115
7/16	12.75			240	270	320	360	380	420	440	460	120
1/2	15.30	220	240	270	320	365	380	410	450	500	550	130
9/16	17.85	240	270	300	360	370	410	430	460	510	550	130
5/8	20.40	260	270	320	365	400	450	480	510	550	580	130
11/16	22.95	260	270	330	373	420	470	500	530	570	600	130
3/4	25.50	260	300	350	390	450	500	520	540	600	620	130
13/16	28.05	260	300	360	420	450	500	520	540	600	620	130
7/8	30.60	260	300	360	400	450	490	520	540	600	620	130
1	33.15	260	300	340	385	440	490	510	530	600	620	130
1 1/8	35.70	260	300	330	375	440	480	510	530	600	620	130
1 1/4	40.80	250	300	300	340	440	460	500	530	580	600	130
1 1/2	45.90	250	300	300	330	410	440	450	500	550	580	130
1 3/4	51.00	240	270	300	310	380	400	420	490	530	550	130
2	61.20	220	230	260	280	330	320	340	420	440	480	130
2 1/4	71.40	200	200	220	240	280	270	300	380	380	410	130
2 1/2	81.60	180	180	190	210	240	240	260	320	330	360	130
2 3/4	91.80	150	160	170	190	210	210	230	280	295	320	130

Thick- ness, Inches	Weight, Lbs. per Sq. Ft.	72	60	60	54	50	48	42	36	30	24	Diam., Inches
5/16	7.65	375	420	470	480	480	480	480	480	480	480	90
3/8	10.20	430	475	525	530	530	530	530	530	530	530	115
7/16	12.75	480	500	560	550	575	575	550	550	550	580	120
1/2	15.30	600	600	620	620	620	620	600	580	600	600	130
9/16	17.85	600	630	630	640	640	640	600	580	600	600	130
5/8	20.40	610	630	630	640	640	640	600	580	630	600	130
11/16	22.95	620	640	640	640	640	640	600	580	630	600	130
3/4	25.50	620	640	640	640	640	640	600	580	600	600	130
13/16	28.05	620	640	640	640	640	640	600	580	600	580	130
7/8	30.60	620	640	640	640	640	640	600	580	600	580	130
1	33.15	620	640	640	640	640	640	600	580	570	550	130
1 1/8	35.70	620	640	640	640	640	640	600	580	550	550	130
1 1/4	40.80	600	630	630	640	640	640	580	580	520	530	130
1 1/2	45.90	580	620	620	640	640	640	580	580	520	500	130
1 3/4	51.00	550	600	600	600	600	600	560	560	520	450	130
2	61.20	530	600	600	600	600	600	540	540	470	430	130
2 1/4	71.40	450	490	550	550	550	550	540	540	430	380	130
2 1/2	81.60	400	440	480	500	500	500	500	500	400	350	130
2 3/4	91.80	350	390	420	450	450	450	450	450	300	200	130

Plates 48" wide and under can also be rolled on Universal Mills.

For greater length and Universal Mill Sizes, see Universal Mill Plate Table.

Plates of greater dimensions than shown in above table may be submitted for special consideration.

FLAT ROLLED STEEL

RECTANGULAR PLATES—Nickel Steel

SHEARED PLATES, ONE-FOURTH INCH AND OVER, EXTREME SIZES

Thick- ness, Inches	Widths and Lengths in Inches														
	102	96	90	84	78	72	66	60	54	50	48	42	36	30	24
1/4						240	240	260	280	280	280	280	280	260	260
5/16					260	260	270	300	310	310	340	340	340	310	310
3/8		280	340	390	420	450	500	500	500	500	480	450	450	430	430
7/16	260	300	360	400	430	480	520	520	520	520	500	490	490	480	480
1/2	270	320	380	420	460	485	520	520	520	520	500	490	490	480	480
9/16	270	320	380	420	460	485	520	520	520	520	500	490	490	480	480
5/8	270	300	355	390	440	480	520	520	520	520	500	500	500	480	450
11/16	260	300	355	390	440	460	490	500	500	500	500	500	480	480	450
3/4	260	300	355	390	440	450	460	500	500	500	500	500	480	480	450
13/16	260	300	355	390	440	440	460	480	500	500	500	500	480	460	440
7/8	260	300	355	390	440	440	460	480	480	480	480	480	480	450	440
1	260	290	320	370	400	430	440	460	480	480	480	480	440	420	420
1 1/8	250	270	295	330	375	400	410	420	440	440	440	440	440	420	420
1 1/4	240	260	290	315	330	350	360	380	390	400	400	420	420	400	400
1 1/2	230	260	290	290	310	330	350	370	390	390	390	390	380	380	360
1 3/4	220	230	250	270	300	310	330	350	370	390	390	360	340	340	320
2	210	230	250	260	290	295	310	330	350	370	370	340	320	320	290

RECTANGULAR PLATES—Nickel Steel

UNIVERSAL MILL PLATES, ONE-FOURTH INCH AND OVER, EXTREME SIZES

Thick- ness, Inches	Widths and Lengths in Inches										
	48-46	45-41	40-36	35-31	30-26	25-20	19-17	16-15	14-12	11	10-6 1/4
1/4							660	660	660	540	540
5/16	540	540	600	660	720	780	780	780	780	600	600
3/8	720	720	780	840	960	960	1020	1020	1020	900	840
7/16	840	840	960	1020	1080	1080	1020	1020	1020	900	840
1/2	960	960	1080	1140	1200	1200	1020	1020	1020	1020	840
9/16	960	960	1080	1140	1200	1200	1020	1020	1020	1020	840
5/8	900	900	1020	1080	1140	1140	1000	1000	1020	1020	840
3/4	840	840	960	1020	1080	1080	1000	1000	1020	900	840
7/8	780	780	840	960	960	960	1000	1000	1000	900	840
1	720	750	780	816	840	900	1000	1000	1000	900	840
1 1/8	640	667	693	725	744	800	1000	1000	1000	840	840
1 1/4	575	600	624	652	672	720	1000	1000	1000	840	840
1 1/2	525	545	567	593	600	655	970	1000	1000	840	840
1 3/4	480	500	520	544	540	600	890	1000	980	840	840
1 7/8	444	461	480	502	504	554	820	978	980	840	840
1 3/4	410	428	445	466	480	514	765	908	980	720	720
1 7/8	384	400	416	435	444	480	710	847	968	660	720
2	360	375	390	408	420	450	670	794	908	600	720

All sizes of Rectangular Nickel Steel Plates given in above tables under 1/2" thick should be specified to gage only. Plates 1/2" thick and over can be rolled to either gage or weight per square foot.

CARNEGIE STEEL COMPANY

RECTANGULAR UNIVERSAL PLATES—Carbon Steel

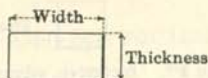
UNIVERSAL MILL PLATES, ONE-FOURTH INCH AND OVER, EXTREME SIZES

Thick- ness, Inches	Weight, Lbs. per Sq. Ft.	Widths and Lengths in Inches										
		48-46	45-41	40-36	35-31	30-26	25-20	19-17	16-15	14-12	11	10-6½
¼	10.20						1020	1020	1020	1020	540	540
⅓	12.75	1020	1020	1140	1260	1320	1320	1080	1080	1080	600	600
⅔	15.30	1200	1200	1320	1380	1380	1380	1080	1080	1080	900	840
½	17.85	1320	1320	1380	1380	1380	1380	1080	1080	1080	900	840
⅝	20.40	1380	1380	1380	1380	1380	1380	1080	1080	1080	1020	840
¾	22.95	1380	1380	1380	1380	1380	1380	1080	1080	1080	1020	840
⅞	25.50	1380	1380	1380	1380	1380	1380	1080	1080	1080	1020	840
1	30.60	1353	1357	1363	1372	1380	1380	1080	1080	1080	900	840
1¼	35.70	1160	1163	1169	1177	1188	1203	1080	1080	1080	900	840
1½	40.80	1015	1018	1023	1030	1039	1052	1080	1080	1080	900	840
1¾	45.90	903	905	910	916	924	936	1080	1080	1080	840	840
1⅝	51.00	812	814	818	824	832	842	1071	1080	1080	840	840
1⅞	56.10	738	740	744	749	756	766	973	1080	1080	840	840
2	61.20	677	679	682	687	693	702	892	1059	1080	840	840
2¼	66.30	625	626	629	634	640	648	823	978	1080	840	840
2½	71.40	580	581	584	588	594	601	765	908	1038	720	720
2¾	76.50	541	543	545	549	554	561	714	847	968	660	720
3	81.60	507	509	511	515	519	526	669	794	907	600	720

Plates of greater dimensions than shown in above table may be submitted for special consideration.

FLAT ROLLED STEEL

SQUARE EDGE FLATS



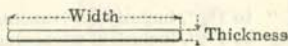
<p>$\frac{3}{8}$" to 1" wide, x $\frac{1}{8}$" to $\frac{3}{8}$"</p> <p>Over 1" to 2" wide, x $\frac{1}{8}$" to $1\frac{1}{4}$"</p> <p>Over 2" to $4\frac{1}{4}$" wide, x $\frac{1}{4}$" to $2\frac{1}{4}$"</p> <p>Over $4\frac{1}{4}$" to $4\frac{3}{4}$" wide, x $\frac{1}{4}$" to 2"</p> <p>5" to $5\frac{1}{4}$" wide, x $\frac{1}{4}$" to 3"</p> <p>$5\frac{3}{8}$" wide, x $\frac{1}{4}$" to $2\frac{3}{4}$"</p> <p>$5\frac{1}{2}$" to $5\frac{5}{8}$" wide, x $\frac{1}{4}$" to $2\frac{3}{4}$"</p> <p>$5\frac{3}{4}$" to 6" wide, x $\frac{1}{4}$" to $2\frac{3}{8}$"</p>	<p>$6\frac{1}{8}$" to $6\frac{3}{8}$" wide, x $\frac{1}{4}$" to $2\frac{1}{2}$"</p> <p>$6\frac{1}{2}$" to $6\frac{5}{8}$" wide, x $\frac{1}{4}$" to $2\frac{3}{8}$"</p> <p>$6\frac{3}{4}$" wide, x $\frac{1}{4}$" to $2\frac{1}{4}$"</p> <p>$6\frac{7}{8}$" to $7\frac{1}{4}$" wide, x $\frac{1}{4}$" to 2"</p> <p>$7\frac{3}{8}$" wide, x $\frac{1}{4}$" to $1\frac{3}{4}$"</p> <p>$7\frac{1}{2}$" to $7\frac{3}{4}$" wide, x $\frac{1}{4}$" to $1\frac{3}{8}$"</p> <p>$7\frac{7}{8}$" to 8" wide, x $\frac{1}{4}$" to $1\frac{3}{8}$"</p>
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Sizes not listed will be considered.

NUT STEEL FLATS

All sizes of Nut Steel Flats within the range of Square Edge Flats can be furnished. Some of the smaller sizes can be furnished in coils.

BAND EDGE FLATS



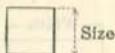
$\frac{3}{8}$ "	wide, x No. 18 to No. 4 B. W. G.
$\frac{7}{16}$ "	wide, x No. 19 to No. 4 B. W. G.
$\frac{1}{2}$ "	wide, x No. 22 to No. 4 B. W. G.
$\frac{9}{16}$ to 1"	wide, x No. 23 to No. 4 B. W. G.
$1\frac{1}{16}$ to 2"	wide, x No. 21 to No. 4 B. W. G.
$2\frac{1}{16}$ to 3"	wide, x No. 20 to No. 1 B. W. G.
$3\frac{1}{16}$ to $3\frac{1}{2}$ "	wide, x No. 19 to No. 1 B. W. G.
$3\frac{3}{16}$ to 4"	wide, x No. 18 to No. 1 B. W. G.
$4\frac{1}{16}$ to $4\frac{1}{2}$ "	wide, x No. 17 to No. 1 B. W. G.
$4\frac{3}{16}$ to $5\frac{1}{16}$ "	wide, x No. 16 to No. 1 B. W. G.
$5\frac{1}{8}$ to $6\frac{3}{4}$ "	wide, x No. 15 to No. 0 B. W. G.
$6\frac{3}{16}$ to 9"	wide, x No. 14 to No. 0 B. W. G.
$9\frac{1}{8}$ to 12"	wide, x No. 12 to No. 0 B. W. G.
$12\frac{1}{8}$ to 15"	wide, x No. 10 to No. 0 B. W. G.
$15\frac{1}{8}$ to 16"	wide, x No. 8 to No. 0 B. W. G.

Sizes not listed will be considered.

SKELP

All sizes within the range of Sheared Plates, Universal Mill Plates and Band Edge Flats can be furnished.

SQUARES



Size $\frac{1}{4}$ " to 2", inclusive, advancing by 64ths.

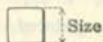
Size $2\frac{1}{32}$ " to $3\frac{1}{2}$ ", inclusive, advancing by 32ds.

Size $3\frac{3}{16}$ " to $5\frac{1}{2}$ ", inclusive, advancing by 16ths.

Squares can also be rolled to decimal dimensions, if so arranged.

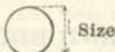
Squares $\frac{3}{8}$ " and smaller can be furnished in coils.

ROUND CORNERED SQUARES



Size $\frac{1}{4}$ " to $\frac{3}{4}$ ", inclusive, advancing by 64ths.

ROUNDS



Size $\frac{1}{4}$ " to $1\frac{3}{4}$ ", inclusive, advancing by 64ths.

Size $1\frac{25}{32}$ " to $3\frac{1}{2}$ ", inclusive, advancing by 32ds.

Size $3\frac{3}{16}$ " to 7", inclusive, advancing by 16ths.

Rounds can also be rolled to decimal dimensions, if so arranged.

Rounds $\frac{3}{8}$ " and smaller can be furnished in coils.

HALF ROUNDS



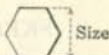
Size

Size $\frac{3}{8}$ " to $\frac{7}{8}$ ", inclusive, advancing by 64ths.

Size $1\frac{1}{16}$ " to $1\frac{3}{4}$ ", inclusive, advancing by 16ths.

Size 2", 2 $\frac{1}{2}$ ", 3".

HEXAGONS



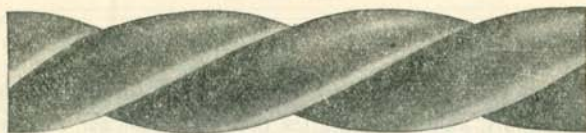
Size $\frac{3}{8}$ " to $1\frac{1}{16}$ ", inclusive, advancing by 32ds.

Size $1\frac{1}{4}$ " to $3\frac{3}{16}$ ", inclusive, advancing by 16ths.

CONCRETE BARS

CONCRETE REINFORCEMENT BARS

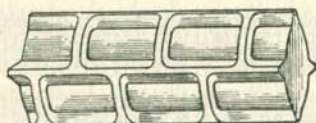
COLD TWISTED SQUARE BARS



Section Index	Size, Inches	Weight per Foot, Pounds	Section Index	Size, Inches	Weight per Foot, Pounds
			$\frac{7}{8}$	0.7656	2.603
	4.0000	13.600	$1\frac{3}{16}$	0.6602	2.245
$1\frac{1}{8}$	3.5156	11.953	$\frac{3}{4}$	0.5625	1.913
$1\frac{3}{8}$	3.0625	10.413	$1\frac{1}{16}$	0.4727	1.607
$1\frac{5}{8}$	2.6406	8.978	$\frac{5}{8}$	0.3906	1.328
$1\frac{7}{8}$	2.2500	7.650	$\frac{9}{16}$	0.3164	1.076
$1\frac{9}{8}$	1.8906	6.428	$\frac{1}{2}$	0.2500	0.850
$1\frac{1}{4}$	1.5625	5.313	$\frac{5}{16}$	0.1914	0.651
$1\frac{3}{4}$	1.2656	4.303	$\frac{3}{8}$	0.1406	0.478
1	1.0000	3.400	$\frac{5}{16}$	0.0977	0.332
$1\frac{1}{16}$	0.8789	2.988	$\frac{1}{4}$	0.0625	0.213

Cold twisted bars will conform to Manufacturers' Standard Specifications, unless otherwise specified.

CUP BARS



Section Index	Size, Inches	Weight per Foot, Pounds	Section Index	Sizes, Inches	Weight per Foot, Pounds
*M 1528	$1\frac{1}{2}$	7.65	*M 1533	$\frac{7}{8}$	2.60
*M 1530	$1\frac{3}{4}$	5.31	*M 1534	$\frac{3}{4}$	1.91
*M 1531	$1\frac{5}{8}$	4.30	*M 1535	$\frac{5}{8}$	1.33
*M 1532	1	3.40	*M 1536	$\frac{1}{2}$	0.85
			*M 1537	$\frac{3}{8}$	0.48

* Furnished only by special arrangement.

CARNEGIE STEEL COMPANY

SQUARE AND ROUND BARS

WEIGHTS AND AREAS

Size, Inches	Weight, Lbs. per Foot		Area, Square Inches		Size, Inches	Weight, Lbs. per Foot		Area, Square Inches	
	□	○	□	○		□	○	□	○
0					3	30.60	24.03	9.000	7.069
1/16	.013	.010	.0039	.0031	3/16	31.89	25.05	9.379	7.366
1/8	.053	.042	.0156	.0123	1/4	33.20	26.08	9.766	7.670
3/16	.120	.094	.0352	.0276	5/16	34.54	27.13	10.160	7.980
1/4	.213	.167	.0625	.0491	3/8	35.91	28.21	10.563	8.296
5/16	.332	.261	.0977	.0767	7/16	37.31	29.30	10.973	8.618
3/8	.478	.376	.1406	.1105	1/2	38.73	30.42	11.391	8.946
7/16	.651	.511	.1914	.1503	5/8	40.18	31.55	11.816	9.281
1/2	.850	.668	.2500	.1963	3/4	41.65	32.71	12.250	9.621
9/16	1.076	.845	.3164	.2485	7/8	43.15	33.89	12.691	9.968
5/8	1.328	1.043	.3906	.3068	15/16	44.68	35.09	13.141	10.321
11/16	1.607	1.262	.4727	.3712	1	46.23	36.31	13.598	10.680
3/4	1.913	1.502	.5625	.4418	1 1/16	47.81	37.55	14.063	11.045
13/16	2.245	1.763	.6602	.5185	1 1/8	49.42	38.81	14.535	11.416
7/8	2.603	2.044	.7656	.6013	1 1/4	51.05	40.10	15.016	11.793
15/16	2.988	2.347	.8789	.6903	1 1/2	52.71	41.40	15.504	12.177
1	3.400	2.670	1.0000	.7854	1 3/4	54.40	42.73	16.000	12.566
1/16	3.838	3.015	1.1289	.8866	1 5/8	56.11	44.07	16.504	12.962
1/8	4.303	3.380	1.2656	.9940	1 3/4	57.85	45.44	17.016	13.364
3/16	4.795	3.766	1.4102	1.1075	1 7/8	59.62	46.83	17.535	13.772
1/4	5.313	4.172	1.5625	1.2272	2	61.41	48.23	18.063	14.186
5/16	5.857	4.600	1.7227	1.3530	2 1/16	63.23	49.66	18.598	14.607
3/8	6.428	5.049	1.8906	1.4849	2 1/8	65.08	51.11	19.141	15.033
7/16	7.026	5.518	2.0664	1.6230	2 1/4	66.95	52.58	19.691	15.466
1/2	7.650	6.008	2.2500	1.7671	2 1/2	68.85	54.07	20.250	15.904
9/16	8.301	6.519	2.4414	1.9175	2 3/8	70.78	55.59	20.816	16.349
5/8	8.978	7.051	2.6406	2.0739	2 1/2	72.73	57.12	21.391	16.800
11/16	9.682	7.604	2.8477	2.2365	2 5/8	74.71	58.67	21.973	17.257
3/4	10.413	8.178	3.0625	2.4053	3	76.71	60.25	22.563	17.721
13/16	11.170	8.773	3.2852	2.5802	3 1/16	78.74	61.85	23.160	18.190
7/8	11.953	9.388	3.5156	2.7612	3 1/8	80.80	63.46	23.766	18.665
15/16	12.763	10.024	3.7539	2.9483	3 1/4	82.89	65.10	24.379	19.147
2	13.600	10.681	4.0000	3.1416	3 1/2	85.00	66.76	25.000	19.635
1/16	14.463	11.359	4.2539	3.3410	3 3/8	87.14	68.44	25.629	20.129
1/8	15.353	12.058	4.5156	3.5466	3 1/2	89.30	70.14	26.266	20.629
3/16	16.270	12.778	4.7852	3.7583	3 3/4	91.49	71.86	26.910	21.135
1/4	17.213	13.519	5.0625	3.9761	4	93.71	73.60	27.563	21.648
5/16	18.182	14.280	5.3477	4.2000	4 1/16	95.96	75.36	28.223	22.166
3/8	19.178	15.062	5.6406	4.4301	4 1/8	98.23	77.15	28.891	22.691
7/16	20.201	15.866	5.9414	4.6664	4 1/4	100.53	78.95	29.566	23.221
1/2	21.250	16.690	6.2500	4.9087	4 1/2	102.85	80.78	30.250	23.758
9/16	22.326	17.534	6.5664	5.1572	4 3/8	105.20	82.62	30.941	24.301
5/8	23.428	18.400	6.8906	5.4119	4 1/2	107.58	84.49	31.641	24.850
11/16	24.557	19.287	7.2227	5.6727	4 3/4	109.98	86.38	32.348	25.406
3/4	25.713	20.195	7.5625	5.9396	5	112.41	88.29	33.063	25.967
13/16	26.895	21.123	7.9102	6.2126	5 1/16	114.87	90.22	33.785	26.535
7/8	28.103	22.072	8.2656	6.4918	5 1/8	117.35	92.17	34.516	27.109
15/16	29.338	23.042	8.6289	6.7771	5 1/4	119.86	94.14	35.254	27.688
3	30.600	24.033	9.0000	7.0686	6	122.40	96.13	36.000	28.274

WEIGHTS AND AREAS OF SQUARES AND ROUNDS

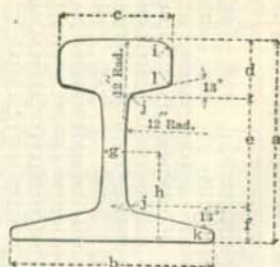
SQUARE AND ROUND BARS

WEIGHTS AND AREAS

Size, Inches	Weight, Lbs. per Foot		Area, Square Inches		Size, Inches	Weight, Lbs. per Foot		Area, Square Inches	
	□	○	□	○		□	○	□	○
6	122.40	96.13	36.000	28.274	9	275.40	216.30	81.000	63.617
$\frac{1}{16}$	124.96	98.15	36.754	28.866	$\frac{1}{16}$	279.24	219.31	82.129	64.504
$\frac{3}{16}$	127.55	100.18	37.516	29.465	$\frac{3}{16}$	283.10	222.35	83.266	65.397
$\frac{1}{8}$	130.17	102.23	38.285	30.069	$\frac{1}{8}$	286.99	225.41	84.410	66.296
$\frac{5}{16}$	132.81	104.31	39.063	30.680	$\frac{5}{16}$	290.91	228.48	85.563	67.201
$\frac{3}{8}$	135.48	106.41	39.848	31.296	$\frac{3}{8}$	294.86	231.58	86.723	68.112
$\frac{7}{16}$	138.18	108.53	40.641	31.919	$\frac{7}{16}$	298.83	234.70	87.891	69.029
$\frac{1}{2}$	140.90	110.66	41.441	32.548	$\frac{1}{2}$	302.83	237.84	89.066	69.953
$\frac{9}{16}$	143.65	112.82	42.250	33.183	$\frac{9}{16}$	306.85	241.00	90.250	70.882
$\frac{5}{8}$	146.43	115.00	43.066	33.824	$\frac{5}{8}$	310.90	244.18	91.441	71.818
$\frac{11}{16}$	149.23	117.20	43.891	34.472	$\frac{11}{16}$	314.98	247.38	92.641	72.760
$\frac{3}{4}$	152.06	119.43	44.723	35.125	$\frac{3}{4}$	319.08	250.61	93.848	73.708
$\frac{7}{8}$	154.91	121.67	45.563	35.785	$\frac{7}{8}$	323.21	253.85	95.063	74.662
$\frac{15}{16}$	157.79	123.93	46.410	36.450	$\frac{15}{16}$	327.37	257.12	96.285	75.622
7	160.70	126.22	47.266	37.122	$\frac{7}{8}$	331.55	260.40	97.516	76.589
$\frac{1}{16}$	163.64	128.52	48.129	37.800	$\frac{1}{16}$	335.76	263.71	98.754	77.561
$\frac{1}{8}$	166.60	130.85	49.000	38.485	10	340.00	267.04	100.000	78.540
$\frac{3}{16}$	169.59	133.19	49.879	39.175	$\frac{1}{16}$	344.26	270.38	101.254	79.525
$\frac{1}{4}$	172.60	135.56	50.766	39.871	$\frac{1}{8}$	348.55	273.75	102.516	80.516
$\frac{5}{16}$	175.64	137.95	51.660	40.574	$\frac{3}{16}$	352.87	277.14	103.785	81.513
$\frac{3}{8}$	178.71	140.36	52.563	41.282	$\frac{1}{4}$	357.21	280.55	105.063	82.516
$\frac{7}{16}$	181.81	142.79	53.473	41.997	$\frac{5}{16}$	361.58	283.99	106.348	83.525
$\frac{1}{2}$	184.93	145.24	54.391	42.718	$\frac{3}{8}$	365.98	287.44	107.641	84.541
$\frac{9}{16}$	188.07	147.71	55.316	43.445	$\frac{1}{2}$	370.40	290.91	108.941	85.563
$\frac{5}{8}$	191.25	150.21	56.250	44.179	$\frac{3}{4}$	374.85	294.41	110.250	86.590
$\frac{11}{16}$	194.45	152.72	57.191	44.918	$\frac{5}{8}$	379.33	297.92	111.566	87.624
$\frac{3}{4}$	197.68	155.26	58.141	45.664	$\frac{3}{8}$	383.83	301.46	112.891	88.664
$\frac{7}{8}$	200.93	157.81	59.098	46.415	$\frac{1}{2}$	388.36	305.02	114.223	89.710
$\frac{15}{16}$	204.21	160.39	60.063	47.173	$\frac{3}{4}$	392.91	308.59	115.563	90.763
8	207.52	162.99	61.035	47.937	$\frac{7}{8}$	397.49	312.19	116.910	91.821
$\frac{1}{16}$	210.85	165.60	62.016	48.707	$\frac{15}{16}$	402.10	315.81	118.266	92.886
$\frac{1}{8}$	214.21	168.24	63.004	49.483	$\frac{7}{8}$	406.74	319.45	119.629	93.957
$\frac{3}{16}$	217.60	170.90	64.000	50.265	11	411.40	323.11	121.000	95.033
$\frac{1}{4}$	221.01	173.58	65.004	51.054	$\frac{1}{16}$	416.09	326.80	122.379	96.116
$\frac{5}{16}$	224.45	176.29	66.016	51.849	$\frac{1}{8}$	420.80	330.50	123.766	97.205
$\frac{3}{8}$	227.92	179.01	67.035	52.649	$\frac{3}{16}$	425.54	334.22	125.160	98.301
$\frac{7}{16}$	231.41	181.75	68.063	53.456	$\frac{1}{4}$	430.31	337.97	126.563	99.402
$\frac{1}{2}$	234.93	184.52	69.098	54.269	$\frac{5}{16}$	435.11	341.73	127.973	100.510
$\frac{9}{16}$	238.48	187.30	70.141	55.088	$\frac{3}{8}$	439.93	345.52	129.391	101.623
$\frac{5}{8}$	242.05	190.11	71.191	55.914	$\frac{1}{2}$	444.78	349.33	130.816	102.743
$\frac{11}{16}$	245.65	192.93	72.250	56.745	$\frac{3}{4}$	449.65	353.16	132.250	103.869
$\frac{3}{4}$	249.28	195.78	73.316	57.583	$\frac{5}{8}$	454.55	357.00	133.691	105.001
$\frac{7}{8}$	252.93	198.65	74.391	58.426	$\frac{3}{8}$	459.48	360.87	135.141	106.139
$\frac{15}{16}$	256.61	201.54	75.473	59.276	$\frac{1}{2}$	464.43	364.76	136.598	107.284
$\frac{3}{4}$	260.31	204.45	76.563	60.132	$\frac{3}{4}$	469.41	368.68	138.063	108.434
$\frac{7}{8}$	264.04	207.38	77.660	60.994	$\frac{15}{16}$	474.42	372.61	139.535	109.591
$\frac{15}{16}$	267.80	210.33	78.766	61.863	$\frac{7}{8}$	479.45	376.56	141.016	110.754
9	271.59	213.31	79.879	62.737	$\frac{15}{16}$	484.51	380.54	142.504	111.923
$\frac{1}{16}$	275.40	216.30	81.000	63.617	12	489.00	384.53	144.000	113.098

CARNEGIE STEEL COMPANY

A. S. C. E. RAILS



Section Index	Weight per Yard, Pounds	a	b	c	d	e	f	g	h	i	j	k	l
		In.	In.	In.	In.	In.	In.	In.	In.	In.	In.	In.	In.

HEAVY RAILS

10040	100	5 $\frac{1}{16}$	5 $\frac{1}{16}$	2 $\frac{3}{4}$	1 $\frac{45}{64}$	3 $\frac{3}{64}$	3 $\frac{1}{32}$	9 $\frac{1}{16}$	2 $\frac{65}{128}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
9040	90	5 $\frac{1}{16}$	5 $\frac{1}{16}$	2 $\frac{5}{8}$	1 $\frac{19}{32}$	2 $\frac{55}{64}$	3 $\frac{5}{64}$	9 $\frac{1}{16}$	2 $\frac{45}{128}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
8540	85	5 $\frac{1}{16}$	5 $\frac{1}{16}$	2 $\frac{9}{16}$	1 $\frac{39}{64}$	2 $\frac{1}{4}$	3 $\frac{7}{64}$	9 $\frac{1}{16}$	2 $\frac{17}{64}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
8040	80	5	5	2 $\frac{1}{2}$	1 $\frac{1}{2}$	2 $\frac{1}{8}$	7 $\frac{1}{8}$	3 $\frac{5}{64}$	2 $\frac{3}{16}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
7540	75	4 $\frac{13}{16}$	4 $\frac{13}{16}$	2 $\frac{17}{32}$	1 $\frac{27}{64}$	2 $\frac{35}{64}$	2 $\frac{7}{32}$	1 $\frac{7}{32}$	2 $\frac{15}{128}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
7040	70	4 $\frac{1}{8}$	4 $\frac{1}{8}$	2 $\frac{1}{16}$	1 $\frac{11}{32}$	2 $\frac{15}{64}$	1 $\frac{1}{16}$	3 $\frac{3}{64}$	2 $\frac{3}{64}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
6540	65	4 $\frac{7}{16}$	4 $\frac{7}{16}$	2 $\frac{17}{32}$	1 $\frac{9}{32}$	2 $\frac{1}{8}$	2 $\frac{5}{32}$	3 $\frac{1}{2}$	1 $\frac{1}{32}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
6040	60	4 $\frac{1}{4}$	4 $\frac{1}{4}$	2 $\frac{3}{8}$	1 $\frac{7}{32}$	2 $\frac{17}{64}$	4 $\frac{9}{64}$	3 $\frac{1}{64}$	1 $\frac{115}{128}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
5540	55	4 $\frac{1}{16}$	4 $\frac{1}{16}$	2 $\frac{1}{4}$	1 $\frac{11}{64}$	2 $\frac{11}{64}$	2 $\frac{3}{32}$	1 $\frac{5}{32}$	1 $\frac{103}{128}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
5040	50	3 $\frac{7}{8}$	3 $\frac{7}{8}$	2 $\frac{1}{8}$	1 $\frac{1}{8}$	2 $\frac{1}{16}$	1 $\frac{1}{16}$	7 $\frac{1}{16}$	1 $\frac{23}{32}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$

LIGHT RAILS

4540	45	3 $\frac{11}{16}$	3 $\frac{11}{16}$	2	1 $\frac{1}{16}$	1 $\frac{21}{32}$	2 $\frac{1}{32}$	2 $\frac{7}{64}$	1 $\frac{41}{64}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
4040	40	3 $\frac{1}{2}$	3 $\frac{1}{2}$	1 $\frac{7}{8}$	1 $\frac{1}{64}$	1 $\frac{25}{64}$	5 $\frac{1}{8}$	2 $\frac{5}{64}$	1 $\frac{71}{128}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
3540	35	3 $\frac{5}{16}$	3 $\frac{5}{16}$	1 $\frac{3}{4}$	3 $\frac{1}{64}$	1 $\frac{25}{64}$	2 $\frac{7}{64}$	2 $\frac{3}{64}$	1 $\frac{15}{32}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
3040	30	3 $\frac{1}{8}$	3 $\frac{1}{8}$	1 $\frac{11}{16}$	7 $\frac{1}{8}$	1 $\frac{23}{32}$	1 $\frac{7}{32}$	2 $\frac{1}{64}$	1 $\frac{29}{64}$	5 $\frac{1}{16}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
2540	25	2 $\frac{3}{4}$	2 $\frac{3}{4}$	1 $\frac{1}{2}$	2 $\frac{5}{32}$	1 $\frac{13}{64}$	3 $\frac{1}{64}$	1 $\frac{9}{64}$	1 $\frac{29}{128}$	3 $\frac{1}{4}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$
2040	20	2 $\frac{1}{8}$	2 $\frac{1}{8}$	1 $\frac{11}{32}$	2 $\frac{9}{32}$	1 $\frac{15}{32}$	7 $\frac{1}{16}$	3 $\frac{1}{4}$	1 $\frac{11}{64}$	3 $\frac{1}{4}$	3 $\frac{1}{16}$		3 $\frac{1}{16}$
1640	16	2 $\frac{3}{8}$	2 $\frac{3}{8}$	1 $\frac{11}{64}$	4 $\frac{1}{64}$	1 $\frac{23}{64}$	3 $\frac{1}{8}$	7 $\frac{1}{32}$	1 $\frac{7}{128}$	3 $\frac{1}{16}$	3 $\frac{1}{16}$		3 $\frac{1}{16}$
*1440	14	2 $\frac{1}{16}$	2 $\frac{1}{16}$	1 $\frac{1}{16}$	5 $\frac{1}{8}$	1 $\frac{19}{32}$	1 $\frac{1}{32}$	3 $\frac{1}{4}$	3 $\frac{5}{64}$	5 $\frac{1}{16}$	3 $\frac{1}{16}$		3 $\frac{1}{16}$
1240	12	2	2	1	9 $\frac{1}{16}$	1 $\frac{9}{32}$	1 $\frac{11}{32}$	3 $\frac{1}{16}$	3 $\frac{5}{64}$	5 $\frac{1}{16}$	3 $\frac{1}{16}$		3 $\frac{1}{16}$
*1040	10	1 $\frac{3}{4}$	1 $\frac{3}{4}$	1 $\frac{5}{16}$	3 $\frac{3}{64}$	1 $\frac{15}{16}$	1 $\frac{9}{64}$	3 $\frac{1}{16}$	4 $\frac{9}{64}$	5 $\frac{1}{16}$	3 $\frac{1}{16}$		3 $\frac{1}{16}$
* 840	8	1 $\frac{15}{16}$	1 $\frac{15}{16}$	1 $\frac{13}{16}$	1 $\frac{7}{32}$	1 $\frac{1}{16}$	9 $\frac{1}{32}$	5 $\frac{1}{32}$	1 $\frac{1}{16}$	5 $\frac{1}{16}$	3 $\frac{1}{16}$		3 $\frac{1}{16}$

*Not rolled by Carnegie Steel Company.

A. S. C. E. RAILS

A. S. C. E. SPLICE BARS

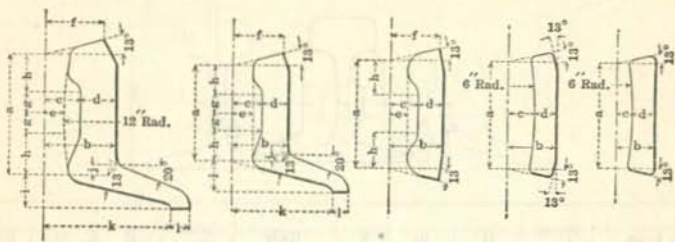
E10040 to S5540

S5040 to S3040

S2540

S2040

S1640 to S840



Section Index	Weight per Foot, Unfinished Pounds	a	b	c	d	e	f	g	h	i	j	k	l
		In.	In.	In.	In.	In.	In.	In.	In.	In.	In.	In.	In.

HEAVY RAILS

S 10040	15.8	3 ⁵ / ₆₄	1 ²⁹ / ₃₂	2 ⁷ / ₃₂	7 ⁵ / ₈	1 ⁵ / ₃₂	1 ³ / ₈	1/2	1 ⁷ / ₁₂₈	2 ⁷ / ₃₂	9 ⁵ / ₃₂	3 ¹ / ₈	1/2
S 9040	13.5	2 ⁵⁵ / ₆₄	1 ⁹ / ₈	1 ³ / ₁₆	1 ³ / ₁₆	1 ⁵ / ₃₂	1 ⁵ / ₁₆	1/2	1 ¹⁹ / ₁₂₈	5 ¹ / ₆₄	1 ⁵ / ₆₄	2 ¹⁵ / ₁₆	1/2
S 8540	12.4	2 ³ / ₄	1 ³⁷ / ₆₄	5 ¹ / ₆₄	2 ⁵ / ₃₂	1 ⁵ / ₃₂	1 ⁹ / ₃₂	1/2	7 ⁵ / ₈	4 ⁹ / ₆₄	7 ⁵ / ₃₂	2 ²⁷ / ₃₂	1/2
S 8040	11.5	2 ⁷ / ₈	1 ¹⁷ / ₃₂	2 ⁵ / ₃₂	3/4	2 ⁹ / ₆₄	1 ¹ / ₄	7 ¹ / ₁₆	7 ⁵ / ₈	3/4	3 ¹ / ₁₆	2 ³ / ₄	7 ¹ / ₁₆
S 7540	10.7	2 ³⁹ / ₆₄	1 ³¹ / ₆₄	4 ⁹ / ₆₄	2 ³ / ₃₂	7 ¹ / ₁₆	1 ¹⁹ / ₆₄	7 ¹ / ₁₆	10 ⁷ / ₁₂₈	2 ³ / ₃₂	2 ¹ / ₁₂₈	2 ²¹ / ₃₂	7 ¹ / ₁₆
S 7040	10.0	2 ¹⁹ / ₃₂	1 ²⁷ / ₆₄	4 ⁷ / ₆₄	1 ¹ / ₁₆	2 ⁷ / ₆₄	1 ³ / ₃₂	7 ¹ / ₁₆	5 ¹ / ₆₄	2 ³ / ₃₂	1 ¹ / ₆₄	2 ¹ / ₂	7 ¹ / ₁₆
S 6540	9.2	2 ³ / ₈	1 ²⁹ / ₆₄	4 ⁵ / ₆₄	2 ¹ / ₃₂	1 ³ / ₃₂	1 ¹³ / ₆₄	7 ¹ / ₁₆	3/4	1 ¹ / ₁₆	5 ⁵ / ₃₂	2 ¹³ / ₃₂	7 ¹ / ₁₆
S 6040	8.4	2 ¹⁷ / ₆₄	1 ¹⁹ / ₆₄	4 ³ / ₆₄	5 ⁸ / ₆₄	2 ⁹ / ₆₄	1 ⁹ / ₁₆	7 ¹ / ₁₆	8 ⁹ / ₁₂₈	4 ³ / ₆₄	2 ¹ / ₁₂₈	2 ⁹ / ₁₆	3 ⁵ / ₈
S 5540	7.5	2 ¹³ / ₆₄	1 ¹⁵ / ₆₄	4 ¹ / ₆₄	1 ⁹ / ₃₂	3 ⁸ / ₆₄	1 ³ / ₈	7 ¹ / ₁₆	8 ³ / ₁₂₈	3 ⁵ / ₈	5 ⁵ / ₃₂	2 ⁷ / ₃₂	3 ⁵ / ₈
S 5040	6.6	2 ¹ / ₁₆	1 ³ / ₈	1 ⁹ / ₃₂	1 ⁷ / ₃₂	3 ⁸ / ₆₄	1 ³ / ₃₂	1 ³ / ₃₂	5 ⁸ / ₆₄	3 ⁵ / ₈	9 ⁵ / ₆₄	2 ¹ / ₁₆	3 ⁵ / ₈

LIGHT RAILS

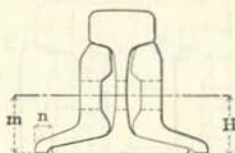
S 4540	5.8	1 ³¹ / ₃₂	1 ³ / ₆₄	3 ⁵ / ₆₄	1/2	2 ³ / ₆₄	3 ¹ / ₃₂	1 ³ / ₃₂	2 ⁷ / ₆₄	1 ⁹ / ₃₂	7 ⁵ / ₆₄	1 ³¹ / ₃₂	3 ⁵ / ₈
S 4040	5.0	1 ⁵⁹ / ₆₄	3 ¹ / ₃₂	1/2	1 ⁷ / ₃₂	1 ¹ / ₃₂	2 ⁹ / ₃₂	1 ³ / ₃₂	6 ⁷ / ₁₂₈	9 ¹ / ₁₆	9 ¹ / ₂₈	1 ⁷ / ₈	5 ¹ / ₁₆
S 3540	4.6	1 ²⁹ / ₃₂	5 ⁷ / ₆₄	2 ⁹ / ₆₄	7 ¹ / ₁₆	5 ¹ / ₁₆	2 ⁷ / ₃₂	1 ¹ / ₃₂	3 ⁵ / ₆₄	3 ³ / ₆₄	7 ⁵ / ₆₄	1 ²⁵ / ₃₂	5 ¹ / ₁₆
S 3040	3.97	1 ²³ / ₃₂	2 ⁷ / ₃₂	7 ¹ / ₁₆	1 ³ / ₃₂	5 ¹ / ₁₆	2 ⁵ / ₃₂	1 ³ / ₃₂	2 ⁹ / ₆₄	1/2	5 ⁵ / ₆₄	1 ¹¹ / ₁₆	5 ¹ / ₁₆
S 2540	2.20	1 ³¹ / ₆₄	3/4	1 ³ / ₃₂	1 ¹ / ₃₂	9 ⁵ / ₃₂	1 ¹ / ₁₆	9 ⁵ / ₃₂	5 ⁹ / ₁₂₈				
S 2040	1.87	1 ¹⁹ / ₃₂	1 ¹ / ₁₆	3 ⁵ / ₆₄	5 ¹ / ₁₆								
S 1640	1.70	1 ²⁹ / ₆₄	3 ⁷ / ₆₄	1 ⁷ / ₆₄	5 ¹ / ₁₆								
*S 1440	1.36	1 ³ / ₃₂	1 ⁷ / ₃₂	7 ⁵ / ₃₂	5 ¹ / ₁₆								
S 1240	1.36	1 ³ / ₃₂	1 ³ / ₃₂	7 ⁵ / ₃₂	5 ¹ / ₁₆								
*S 1040	0.99	1 ⁵ / ₁₆	1 ⁵ / ₃₂	7 ⁵ / ₃₂	1/4								
*S 840	0.75	1 ³ / ₁₆	7 ¹ / ₁₆	7 ⁵ / ₃₂	7 ⁵ / ₃₂								

*Not rolled by Carnegie Steel Company.

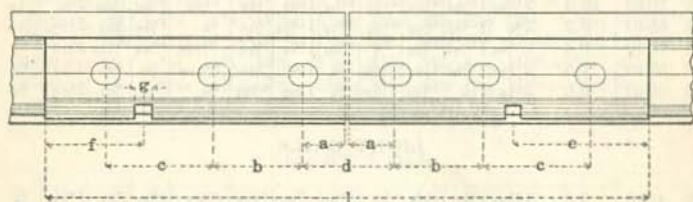
CARNEGIE STEEL COMPANY

A. S. C. E. RAILS

DIMENSIONS FOR STANDARD DRILLING AND PUNCHING IN INCHES



Heavy Rails		Splice Bars		H	m	* n	Light Rails		Splice Bars		H	m	n
	10040	S 10040	2 ⁶⁵ / ₁₂₈	2 ⁴⁹ / ₁₂₈	1 ³ / ₁₆	1 ³ / ₁₆	4540	S 4540	1 ⁴ / ₁₆	1 ³⁷ / ₆₄	1 ³⁷ / ₆₄	9	1/2
	9040	S 9040	2 ⁴⁵ / ₁₂₈	2 ²⁹ / ₁₂₈	1 ³ / ₁₆	1 ³ / ₁₆	4040	S 4040	1 ³ / ₁₆	1 ³⁷ / ₆₄	1 ³⁷ / ₆₄	9	1/2
	8540	S 8540	2 ¹⁷ / ₆₄	2 ⁹ / ₆₄	1 ³ / ₁₆	1 ³ / ₁₆	3540	S 3540	1 ¹⁵ / ₆₄	1 ¹³ / ₃₂	1 ¹³ / ₃₂	9	1/2
	8040	S 8040	2 ³ / ₁₆	2 ¹ / ₁₆	3/4	3/4	3040	S 3040	1 ²⁵ / ₆₄	1 ²³ / ₆₄	1 ²³ / ₆₄	9	1/2
	7540	S 7540	2 ¹⁵ / ₁₂₈	1 ¹²⁷ / ₁₂₈	3/4	3/4	2540	S 2540	1 ²⁹ / ₁₂₈	1 ¹¹ / ₆₄	1 ¹¹ / ₆₄	9	1/2
	7040	S 7040	2 ³ / ₆₄	1 ¹⁰ / ₆₄	1 ¹ / ₁₆	1 ¹ / ₁₆	2040	S 2040	1 ¹¹ / ₆₄	5 ⁷ / ₆₄	5 ⁷ / ₆₄	9	1/2
	6540	S 6540	1 ²¹ / ₃₂	1 ⁷ / ₈	1 ¹ / ₁₆	1 ¹ / ₁₆	1640	S 1640	1 ⁷ / ₁₂₈	5 ⁷ / ₆₄	5 ⁷ / ₆₄	9	1/2
	6040	S 6040	1 ¹¹ / ₁₂₈	1 ¹⁰³ / ₁₂₈	5/8	5/8	1440	S 1440	5 ⁷ / ₆₄	4 ⁹ / ₆₄	4 ⁹ / ₆₄	9	1/2
	5540	S 5540	1 ¹⁰³ / ₁₂₈	1 ⁹¹ / ₁₂₈	5/8	5/8	1240	S 1240	5 ⁷ / ₆₄	4 ⁹ / ₆₄	4 ⁹ / ₆₄	9	1/2
	5040	S 5040	1 ²³ / ₃₂	1 ²¹ / ₃₂	9/16	9/16	1040	S 1040	4 ⁹ / ₆₄	1 ¹ / ₁₆	1 ¹ / ₁₆	9	1/2
							840	S 840	1 ¹ / ₁₆			9	1/2

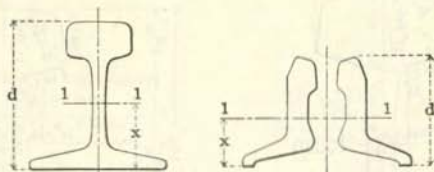


Rails, Lb. per Yd.	Size of Hole		a	b	c	d	e	f	g	l
	Rail	Splice Bar								
100 to 90	1 ¹ / ₄	1 ¹ / ₈ x 1 ¹ / ₈	2 ¹ / ₂	5	6	5 ¹ / ₁₆	7 ¹ / ₂	5 ¹ / ₂	1 ¹ / ₁₆	34
85 to 75	1 ¹ / ₈	1 x 1 ¹ / ₄	2 ¹ / ₂	5	6	5 ¹ / ₁₆	7 ¹ / ₂	5 ¹ / ₂	1 ¹ / ₁₆	34
72 to 70	1	3/8 x 1 ¹ / ₈	2 ¹ / ₂	5	6	5 ¹ / ₁₆	7 ¹ / ₂	5 ¹ / ₂	1 ¹ / ₁₆	34
67 to 50	1	3/8 x 1 ¹ / ₈	2 ¹ / ₂	5	5 ¹ / ₁₆	6 ¹ / ₂	2 ¹ / ₂	1 ¹ / ₁₆	24	
45 to 40	3/4	1 ¹ / ₁₆ x 1 ¹ / ₈	2 ¹ / ₂	5	5 ¹ / ₁₆	3 ⁷ / ₈	1 ¹ / ₈	3/4	20	
35 to 30	3/4	1 ¹ / ₁₆ x 3 ¹ / ₃₂	2	4	4 ¹ / ₈				16 ¹ / ₂	
25 to 12	5/8	9/16 x 3/4	2	4	4 ¹ / ₈				16 ¹ / ₂	
10 to 8	1/2	7/16 x 3/8	2	4	4 ¹ / ₈				16 ¹ / ₂	

A. S. C. E. RAILS

A. S. C. E. RAILS

ELEMENTS OF RAILS AND SPLICE BARS



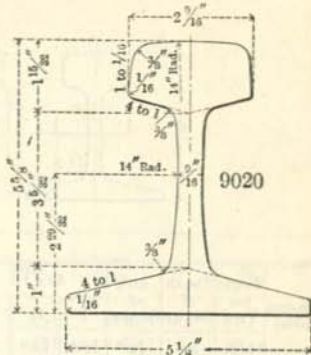
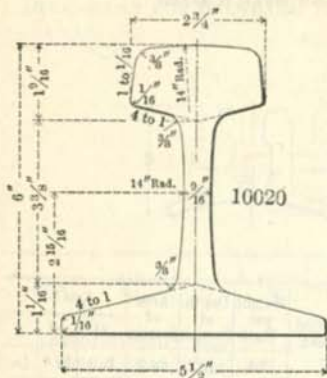
Section Index	Weight per Yard	Depth of Section	Area of Section	Axis 1-1			Section Index	Weight per Foot	Depth of Section	Area of Section	Axis 1-1		
				I	S	x					*I	*S	x
	Lbs.	In.	In. ²	In. ⁴	In. ³	In.		Lbs.	In.	In. ²	In. ⁴	In. ³	In.
HEAVY RAILS							HEAVY RAIL SPLICE BARS						
10040	100	5 $\frac{3}{4}$	9.84	43.97	14.55	2.73	S 10040	15.8	4 $\frac{7}{8}$	4.65	13.43	5.82	1.91
9040	90	5 $\frac{5}{8}$	8.83	34.39	12.19	2.55	S 9040	13.5	3 $\frac{1}{2}$	3.97	10.30	4.79	1.81
8540	85	5 $\frac{1}{2}$	8.33	30.07	11.08	2.47	S 8540	12.4	3 $\frac{1}{4}$	3.65	8.43	4.02	1.71
8040	80	5	7.86	26.38	10.07	2.38	S 8040	11.5	3 $\frac{1}{8}$	3.38	7.39	3.75	1.68
7540	75	4 $\frac{7}{8}$	7.33	22.86	9.10	2.30	S 7540	10.7	3 $\frac{1}{2}$	3.15	6.02	3.28	1.65
7040	70	4 $\frac{5}{8}$	6.81	19.70	8.19	2.22	S 7040	10.0	3 $\frac{1}{8}$	2.95	5.82	3.15	1.61
6540	65	4 $\frac{3}{8}$	6.33	16.90	7.37	2.14	S 6540	9.2	3 $\frac{1}{8}$	2.71	4.85	2.73	1.56
6040	60	4 $\frac{1}{4}$	5.93	14.56	6.62	2.05	S 6040	8.4	3 $\frac{1}{8}$	2.47	4.04	2.38	1.51
5540	55	4 $\frac{1}{8}$	5.38	12.03	5.75	1.97	S 5540	7.5	3 $\frac{1}{8}$	2.21	3.41	2.07	1.41
5040	50	3 $\frac{7}{8}$	4.87	9.94	4.98	1.88	S 5040	6.6	2 $\frac{1}{2}$	1.95	2.72	1.74	1.37
LIGHT RAILS							LIGHT RAIL SPLICE BARS						
4540	45	3 $\frac{1}{2}$	4.40	8.13	4.25	1.78	S 4540	5.8	2 $\frac{3}{8}$	1.70			1.29
4040	40	3 $\frac{1}{4}$	3.94	6.57	3.62	1.68	S 4040	5.0	2 $\frac{1}{4}$	1.47			1.27
3540	35	3 $\frac{1}{8}$	3.44	5.17	3.02	1.60	S 3540	4.6	2 $\frac{1}{8}$	1.35			1.19
3040	30	3 $\frac{1}{8}$	3.00	4.06	2.53	1.52	S 3040	3.97	2 $\frac{1}{8}$	1.17			1.10
2540	25	2 $\frac{3}{4}$	2.39	2.50	1.77	1.33	S 2540	2.20	1 $\frac{5}{8}$	0.65			0.90
2040	20	2 $\frac{1}{2}$	2.00	1.94	1.43	1.27	S 2040	1.87	1 $\frac{3}{8}$	0.55			0.86
1640	16	2 $\frac{1}{8}$	1.55	1.24	1.01	1.15	S 1640	1.70	1 $\frac{3}{8}$	0.50			0.79
1440	14	2 $\frac{1}{8}$	1.34	0.76	0.73	1.02	S 1440	1.36	1 $\frac{3}{8}$	0.40			0.65
1240	12	2	1.18	0.66	0.63	0.96	S 1240	1.36	1 $\frac{3}{8}$	0.40			0.65
1040	10	1 $\frac{3}{4}$	0.96	0.40	0.46	0.87	S 1040	0.99	1 $\frac{3}{8}$	0.29			0.56
840	8	1 $\frac{1}{8}$	0.77	0.26	0.32	0.75	S 840	0.75	1 $\frac{1}{8}$	0.22			0.49

* Moment of Inertia and Section Modulus are given for pair of Splice Bars.

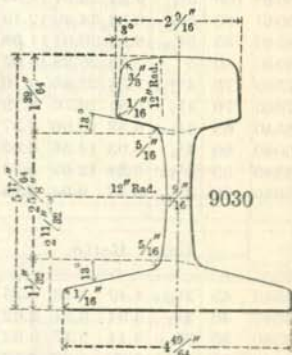
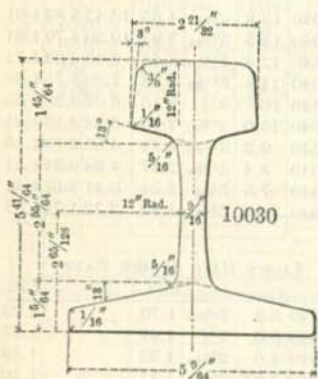
CARNEGIE STEEL COMPANY

AMERICAN RAILWAY ASSOCIATION RAILS

SERIES A



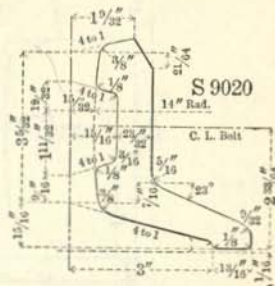
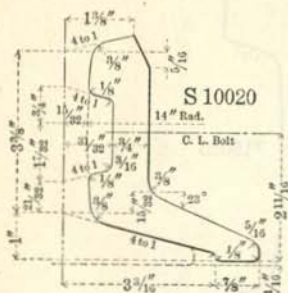
SERIES B



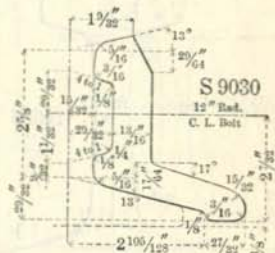
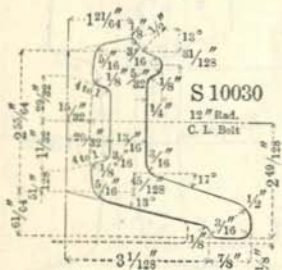
Section Index	Weight per Yard	Area of Section	Moment of Inertia	Section Modulus		Neutral Axis	
				Base	Head	Base	Head
		Lb.	In. ²	In. ⁴	In. ³	In. ³	In.
10020	100	9.84	48.94	17.82	15.04	2 3/4	3 1/4
9020	90	8.82	38.70	15.21	12.56	2 29/64	3 7/64
10030	100	9.85	41.30	15.73	13.70	2 7/8	3 3/64
9030	90	8.87	32.30	13.21	11.45	2 7/16	2 29/64

AMERICAN RAILWAY ASSOCIATION SPLICE BARS

SERIES A



SERIES B

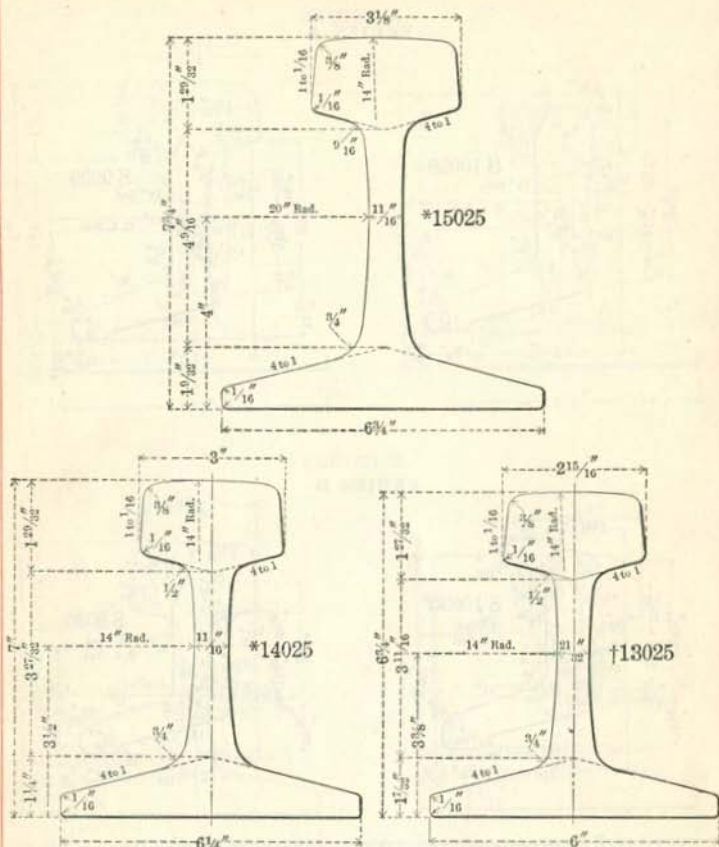


Section Index	Weight per Foot	Area of Section	Moment of Inertia	Section Modulus		Neutral Axis	
				Bottom	Top	Bottom	Top
				I/x	I/y	x	y
	Lb.	In. ²	In. ⁴	In. ³	In. ³	In.	In.
S 10020	19.0	5.60	21.30	10.54	7.88	2 1/64	2 45/64
S 9020	16.6	4.90	16.10	8.43	6.36	1 29/32	2 17/32
*S 10030	16.9	4.98	14.34	7.84	6.30	1 33/64	2 9/32
*S 9030	14.4	4.24	10.16	6.08	4.71	1 43/64	2 5/32

Moment of Inertia and Section Modulus are given for pair of Splice Bars.

* Not rolled by Carnegie Steel Company.

AMERICAN RAILWAY ENGINEERING ASSOCIATION RAILS

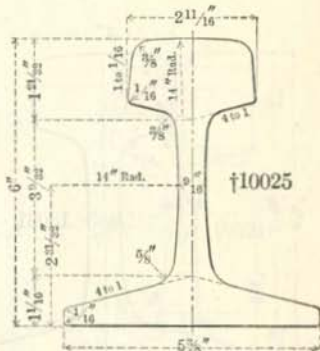
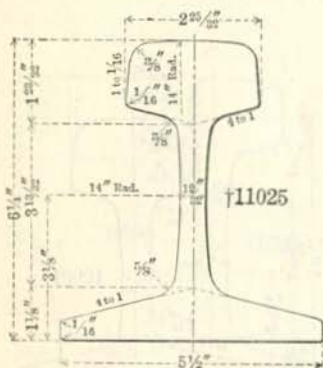
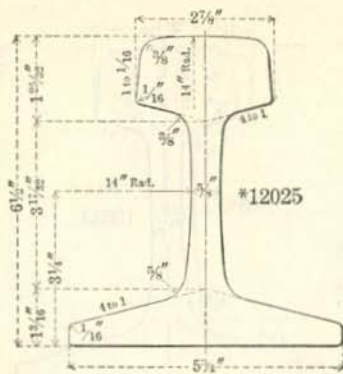


Section Index	Weight per Yard	Area of Section In. ²	Moment of Inertia In. ⁴	Section Modulus		Neutral Axis	
				Base	Head	Base	Head
				I/x In. ³	I/y In. ³	x In.	y In.
*15025	150.45	14.75	121.1	35.1	28.2	3 ²⁹ / ₆₄	4 ¹⁹ / ₆₄
*14025	138.52	13.58	89.2	28.4	23.1	3 ⁵ / ₄	3 ⁵⁵ / ₆₄
†13025	129.64	12.71	77.4	25.0	20.8	3 ³ / ₈	3 ²⁹ / ₃₂

*Not rolled by C. S. Co., I. S. Co., or T. C. I. & R. R. Co.

†Rolled by C. S. Co. and I. S. Co.

AMERICAN RAILWAY ENGINEERING ASSOCIATION RAILS

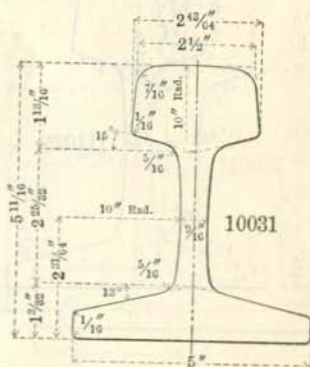
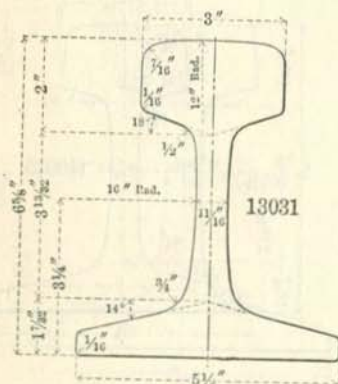
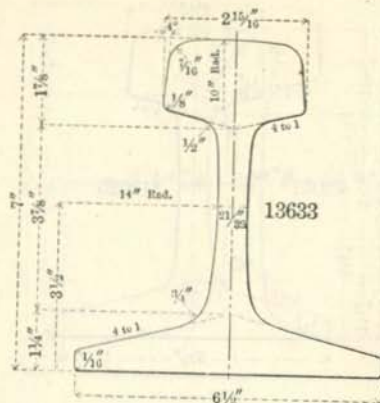


Section Index	Weight per Yard	Area of Section In. ²	Moment of Inertia In. ⁴	Section Modulus		Neutral Axis	
				Base	Head	Base	Head
				1/x	1/y	x	y
	Lb.			In. ³	In. ³	In.	In.
*12025	120.87	11.85	67.6	23.1	18.9	2 ⁵⁹ / ₆₄	3 ³⁷ / ₆₄
†11025	110.36	10.82	57.0	20.1	16.7	2 ⁵³ / ₆₄	3 ²⁷ / ₆₄
†10025	101.49	9.95	49.0	17.8	15.1	2 ⁵ / ₄	3 ¹ / ₄

*Not rolled by C. S. Co., I. S. Co., or T. C. I. & R. R. Co.
 †Rolled by C. S. Co., I. S. Co., and T. C. I. & R. R. Co.

CARNEGIE STEEL COMPANY

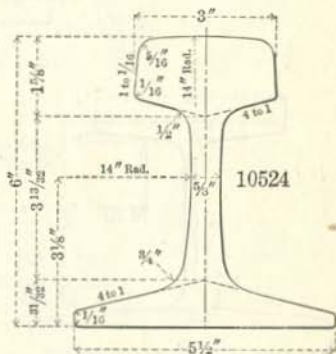
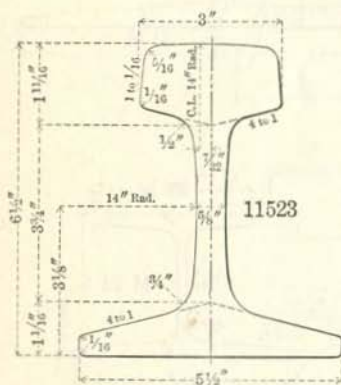
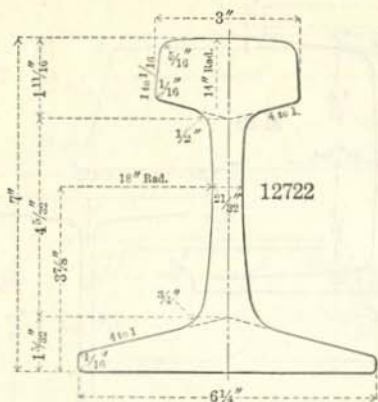
MISCELLANEOUS RAILS



Section Index	Weight per Yard	Area of Section A	Moment of Inertia I	Section Modulus		Neutral Axis	
				Base	Head	Base	Head
				I/x	I/y	x	y
	Lb.	In. ²	In. ⁴	In. ³	In. ³	In.	In.
13633	136	13.35	86.57	28.27	21.98	3 1/16	3 15/16
13031	130	12.70	72.80	23.50	20.60	3 3/32	3 17/32
10031	100	9.98	42.71	16.06	14.10	2 21/32	3 1/32

MISCELLANEOUS RAILS

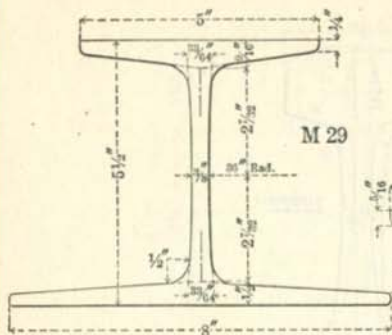
MISCELLANEOUS RAILS—Concluded



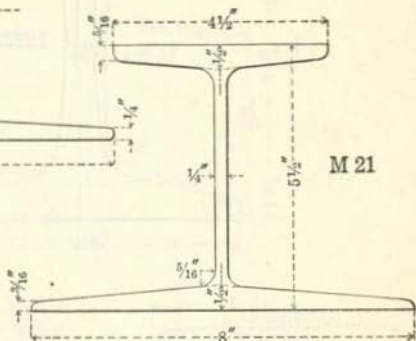
Section Index	Weight per Yard	Area of Section A	Moment of Inertia I	Section Modulus		Neutral Axis	
				Base	Head	Base	Head
				I/x	I/y	x	y
	Lb.	In. ²	In. ⁴	In. ³	In. ³	In.	In.
12722	127.3	12.48	83.70	27.00	21.46	3 ³ / ₃₂	3 ²⁹ / ₃₂
11523	115	11.24	64.00	21.33	18.24	3	3 ¹ / ₂
10524	105	10.26	49.86	17.30	15.96	2 ⁷ / ₈	3 ¹ / ₄

CARNEGIE STEEL COMPANY

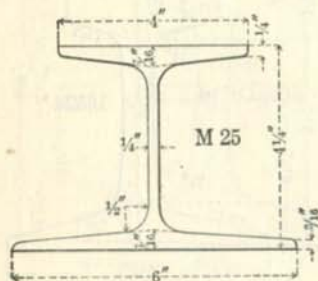
CROSS TIE SECTIONS



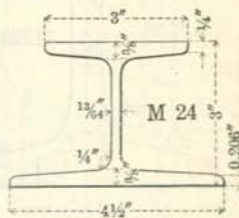
M 29



M 21



M 25



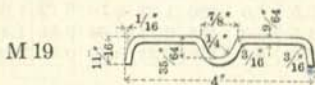
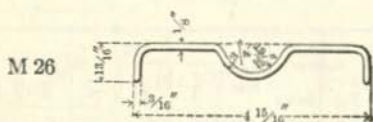
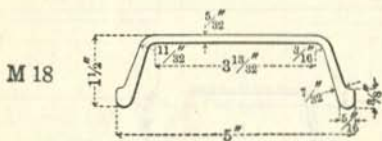
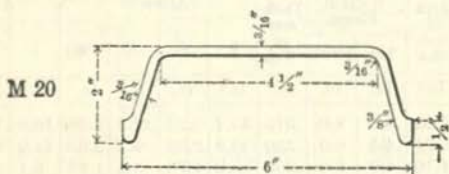
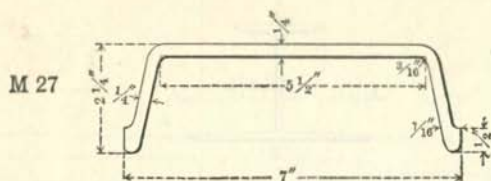
M 24

Section Index	Depth, Inches	Width of Flanges		Web Thickness, Inches	Weight per Foot, Pounds
		Top, Inches	Bottom, Inches		
M 29	5 1/2	5	8	3/8	24.0
M 21	5 1/2	4 1/2	8	1/4	20.0
M 25	4 1/4	4	6	1/4	14.5
M 24	3	3	4 1/2	1 3/64	9.4

Full information as to uses of steel cross ties is given in a separate pamphlet on Steel Cross Ties.

STEEL CROSS TIES

CROSS TIE SECTIONS—Concluded



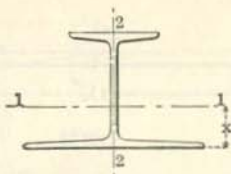
Section Index	Depth, Inches	Width, Inches	Web Thickness, Inches	Weight per Foot, Pounds
M 27	2 1/4	7	1/4	9.0
M 20	2	6	3/16	6.0
M 18	1 1/2	5	5/32	4.0
M 26	1 13/16	4 15/16	1/4	3.20
M 19	1 1/16	4	3/64	2.50

Full information as to uses of steel cross ties is given in a separate pamphlet on Steel Cross Ties.

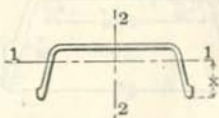
CARNEGIE STEEL COMPANY

ELEMENTS OF CROSS TIES

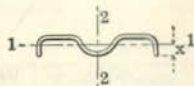
New



Section Index	Depth of Section In.	Wt. per Foot Lbs.	Area of Section In. ²	Width of Flange		Thick-ness of Web In.	Axis 1-1				Axis 2-2		
				Top	Bottom		I	r	S	x	I	r	S
				In.	In.		In. ³	In.	In. ³	In. ³	In. ³	In. ⁴	In.
M 29	5.50	24.0	7.01	5.0	8.0	.375	35.4	2.25	11.3	2.38	16.8	1.55	4.2
M 21	5.50	20.0	5.71	4.5	8.0	.250	30.9	2.33	9.7	2.33	14.9	1.62	3.7
M 25	4.25	14.5	4.10	4.0	6.0	.250	13.0	1.78	5.5	1.88	6.1	1.22	2.0
M 24	3.00	9.4	2.77	3.0	4.5	.203	4.2	1.24	2.5	1.32	2.9	1.03	1.3



Section Index	Depth of Section In.	Wt. per Foot Lbs.	Area of Section In. ²	Width of Section		Thick-ness In.	Axis 1-1				Axis 2-2		
				Top	Bottom		I	r	S	x	I	r	S
				In.	In.		In. ⁴	In.	In. ³	In.	In. ⁴	In.	In. ⁴
M 27	2.25	9.0	2.62	5.5	7.0	.250	1.28	0.70	0.79	1.62	16.8	2.53	4.8
M 20	2.00	6.0	1.72	4.5	6.0	.188	0.71	0.64	0.51	1.41	8.4	2.22	2.8
M 18	1.50	4.0	1.21	3.4	5.0	.156	0.31	0.50	0.31	1.00	3.6	1.73	1.5



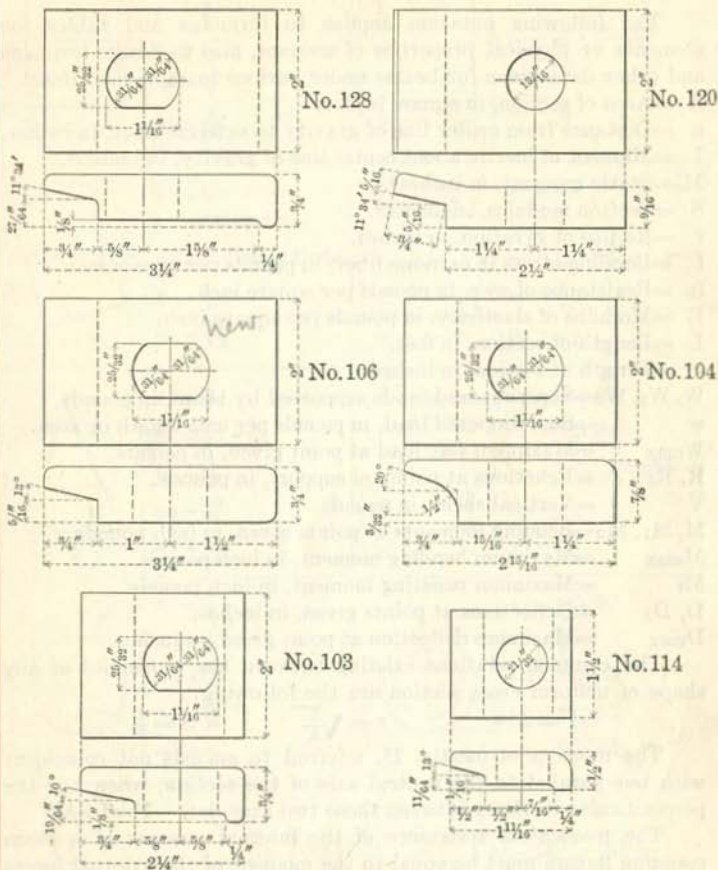
Section Index	Depth of Section In.	Wt. per Foot Lbs.	Area of Section In. ²	Width of Section In.	Thick-ness In.	Axis 1-1				Axis 2-2		
						I	r	S	x	I	r	S
						In. ⁴	In.	In. ³	In.	In. ⁴	In.	In. ⁵
M 26	1 ⁷ / ₁₆	3.20	0.97	4 ¹⁵ / ₁₆	.125	0.059	0.25	0.110	0.54	2.44	1.58	0.99
M 19	1 ³ / ₁₆	2.51	0.74	4	.141	0.024	0.18	0.057	0.43	1.15	1.25	0.58

RAIL CLIPS

new

RAIL CLIPS

new



Clip Number	Weight per Foot, Pounds	Finished Weight, Pounds		Fastenings for	
		Clip Only	Clip, Bolt, Nut	Tie Sections	Rail Sections
128	6.0	0.87	1.53	M21, M25, M29	100 to 60 lb. A. R. A.-B
120	5.6	0.81	1.25	M18, M20, M27	40 to 25 lb. A. S. C. E.
106	7.2	1.05	1.71	M21, M25, M29	100 to 60 lb. A. S. C. E.
104	7.2	1.02	1.68	M21, M25, M29	100 to 60 lb. Angle Bars
103	4.3	0.58	1.24	M21, M25, M29	100 to 55 lb. A. S. C. E.
114	2.24	0.24	0.65	M24	50 to 22 lb. A. S. C. E.

Clips can be furnished with $\frac{25}{32}$ " diameter holes.

GENERAL NOTATION IN FORMULAS

The following notation applies to formulas and tables for elements or physical properties of sections, also to flexure formulas and other data given for beams under various loading conditions:

- A =Area of section, in square inches.
 n =Distance from center line of gravity to extreme fiber, in inches.
 I =Moment of inertia about center line of gravity, in inches⁴.
 M_s=Static moment, in inches³.
 S =Section modulus, in inches³.
 r =Radius of gyration, in inches.
 f =Bending stress in extreme fiber, in pounds per square inch.
 f_b =Resistance of web, in pounds per square inch.
 E =Modulus of elasticity, in pounds per square inch.
 L =Length of section, in feet.
 l =Length of section, in inches.
 W, W₁, W₂=Superimposed loads supported by beam, in pounds.
 w =Superimposed load, in pounds per unit length or area.
 W_{max} =Maximum safe load at point given, in pounds.
 R, R₁ =Reactions at points of support, in pounds.
 V =Vertical shear, in pounds.
 M, M₁, M₂=Bending moments at points given, in inch pounds.
 M_{max} =Maximum bending moment, in inch pounds.
 M_r =Maximum resisting moment, in inch pounds.
 D, D₁ =Deflections at points given, in inches.
 D_{max} =Maximum deflection at point given, in inches.

The common relations existing between the properties of any shape of uniform cross section are the following:

$$I = Ar^2 \qquad r = \sqrt{\frac{I}{A}} \qquad S = \frac{I}{n}$$

The moment of inertia, I¹, referred to an axis not coincident with but parallel to the neutral axis of the section, when z is the perpendicular distance between these two axes, is: $I^1 = I + Az^2$

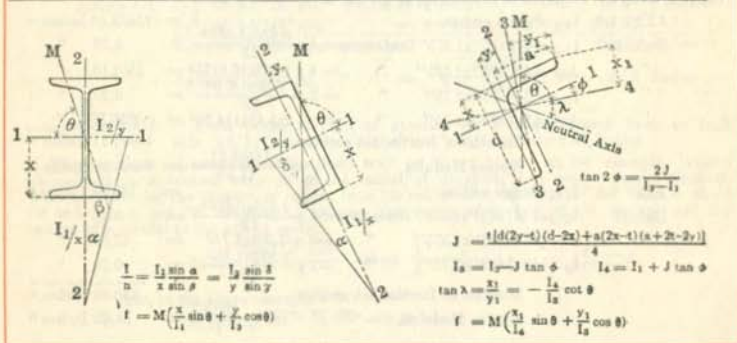
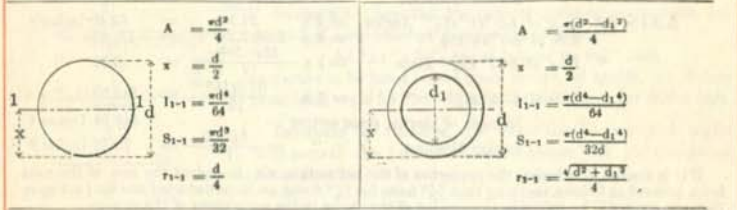
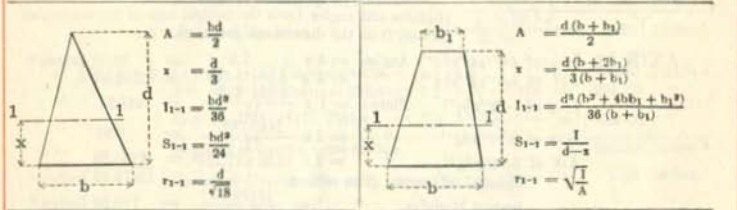
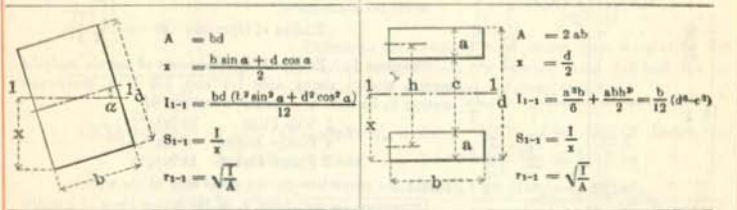
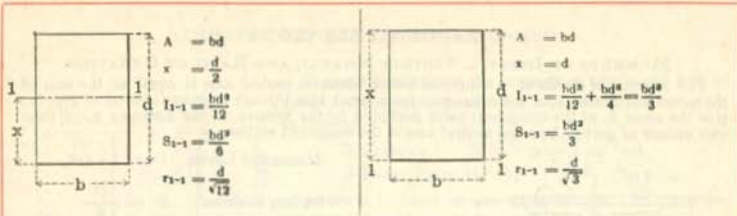
The moment of resistance of the internal stresses of a beam resisting flexure must be equal to the moment of the external forces producing bending.

$$M_r = M_{max} = f \frac{I}{n} = f S.$$

The moment of resistance and the bending moment must, therefore, be expressed in same units of moment, force x length, generally in inch-pounds.

The modulus of elasticity is the ratio between unit stress and the elongation caused by that stress in one unit of length, up to the elastic limit; for steel the modulus of elasticity is 29,000,000 pounds per square inch.

ELEMENTS OF SECTIONS



COMPOUND SECTIONS

MOMENTS OF INERTIA, SECTION MODULI, AND RADII OF GYRATION

The moment of inertia of a compound section about its neutral axis is equal to the sum of the moment of inertia, I , of the component parts about axes through their own centers of gravity, plus the areas A , of the component parts multiplied by the squares of the distances z , of their own centers of gravity from the neutral axis of the compound section, or

$$\text{Moment of Inertia } I^1 = I + Az^2$$

$$\text{Section Modulus } S^1 = \frac{I^1}{n}$$

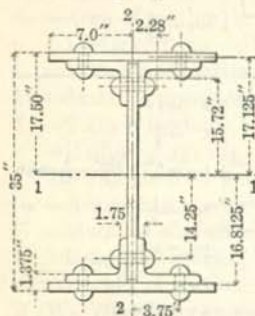
$$\text{Radius of Gyration } r^1 = \sqrt{\frac{I^1}{A^1}}$$

EXAMPLE 1. Required the moments of inertia and the section moduli about axes 1-1 and 2-2 of a compound section to be used as a girder, composed of

- 1 Web Plate 33''x $\frac{1}{2}$ ''
- 4 Flange Angles 6''x4''x $\frac{5}{8}$ ''
- 2 Flange Plates 14''x $\frac{3}{4}$ ''

basing the properties on the gross area of the section.

Determine the distances, z , of the center lines of gravity of plates and angles, from the neutral axes of the compound section, from the dimensions given, then for

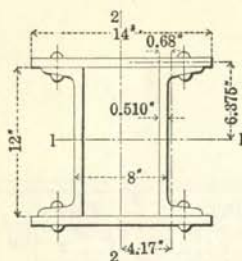


AXIS 1-1		I_{1-1} of 4-6''x4''x $\frac{5}{8}$ '' Angles	= 4 x 7.5	= 30.00 Inches ⁴
		Az^2 of 4-6''x4''x $\frac{5}{8}$ ''	= 4 x 5.86x15.72 ²	= 5792.46 "
		I_{1-1} of 1-33''x $\frac{1}{2}$ '' Plate	= 1 x $\frac{0.50x33^3}{12}$	= 1497.38 "
		I_{1-1} of 2-14''x $\frac{3}{4}$ ''	= 2 x $\frac{14x0.75^3}{12}$	= 0.98 "
		Az^2 of 2-14''x $\frac{3}{4}$ ''	= 2 x 10.50 x 17.125 ²	= 6158.58 "
		Moment of Inertia, gross section		13479.40 Inches ⁴
		Section Modulus, " "	= $\frac{13479.40}{17.50}$	= 770.26 Inches ³
AXIS 2-2		I_{2-2} of 4-6''x4''x $\frac{5}{8}$ '' Angles	= 4 x 21.1	= 84.40 Inches ⁴
		Az^2 of 4-6''x4''x $\frac{5}{8}$ ''	= 4 x 5.86x2.28 ²	= 121.85 "
		I_{2-2} of 1-33''x $\frac{1}{2}$ '' Plate	= 1 x $\frac{33x0.50^3}{12}$	= 0.34 "
		I_{2-2} of 2-14''x $\frac{3}{4}$ ''	= 2 x $\frac{0.75x14^3}{12}$	= 343.00 "
		Moment of Inertia, gross section		549.59 Inches ⁴
		Section Modulus, " "	= $\frac{549.59}{7}$	= 78.51 Inches ³

If it is desired to calculate the properties of the net section, viz., to deduct the area of the rivet holes, proceed as follows, assuming that $\frac{3}{8}$ '' holes for $\frac{3}{4}$ '' rivets are to be deducted and that not more than one rivet will be driven in any one leg of the angles in the same plane of the section.

AXIS 1-1		I_{1-1} of gross section		13479.40 Inches ⁴
Deduct		I_{1-1} of 4-0.875''x1.375'' Rectangles	= 4 x $\frac{0.875x1.375^3}{12}$	= 0.76 "
		Az^2 of 4-0.875''x1.375''	= 4 x 1.203x16.8125 ²	= 1360.16 "
		I_{1-1} of 2-0.875''x1.75''	= 2 x $\frac{1.75x0.875^3}{12}$	= 0.20 "
		Az^2 of 2-0.875''x1.75''	= 2x1.531x14.25 ²	= 621.77 "
		Moment of Inertia, net section		11496.51 Inches ⁴
		Section Modulus, " "	= $\frac{11496.51}{17.50}$	= 656.94 Inches ³
AXIS 2-2		I_{2-2} of gross section		549.59 Inches ⁴
Deduct		I_{2-2} of 4-0.875''x1.375'' Rectangles	= 4 x $\frac{1.375x0.875^3}{12}$	= 0.31 "
		Az^2 of 4-0.875''x1.375''	= 4 x 1.203x3.75 ²	= 67.67 "
		I_{2-2} of 2-0.875''x1.75''	= 2 x $\frac{0.875x1.75^3}{12}$	= 0.78 "
		Moment of Inertia, net section		480.83 Inches ⁴
		Section Modulus, " "	= $\frac{480.83}{7}$	= 68.69 Inches ³

COMPOUND SECTIONS—Concluded



EXAMPLE 2. Required the moments of inertia and radii of gyration about axes 1-1 and 2-2 of a column section composed as follows:—

- 2 Channels 12" x 30 pounds per foot,
2 Flange Plates 14" x 3/4",

properties to be based on the gross section, no deduction being made for holes.

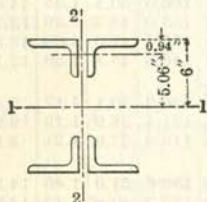
Determine the distances, z , of center lines of gravity for the various sections from the neutral axes 1-1 and 2-2, in accordance with the dimensions given, then for

AXIS 1-1	I_{1-1} of 2-12" Channels 30 lbs.	= 2 x 161.2	= 322.40 Inches ⁴
	I_{1-1} of 2-14" x 3/4" Plates	= 2 x $\frac{14 \times 0.75^3}{12}$	= 0.98 "
	Az^2 of 2-14" x 3/4" "	= 2 x 10.5 x 6.375 ²	= 853.45 "
	Moment of Inertia, gross section		1176.83 Inches ⁴

$$\text{Radius of Gyration, " " } = \sqrt{\frac{1176.83}{38.58}} = 5.52 \text{ Inches}$$

AXIS 2-2	I_{2-2} of 2-12" Channels 30 lbs.	= 2 x 5.2	= 10.40 Inches ⁴
	Az^2 of 2-12" Channels 30 lbs.	= 2 x 8.79 x 4.17 ²	= 305.70 "
	I_{2-2} of 2-14" x 3/4" Plates	= 2 x $\frac{0.75 \times 14^3}{12}$	= 343.00 "
	Moment of Inertia, gross section		659.10 Inches ⁴

$$\text{Radius of Gyration, " " } = \sqrt{\frac{659.10}{38.58}} = 4.13 \text{ Inches}$$



EXAMPLE 3. Required the radii of gyration about axes 1-1 and 2-2 of a strut section composed as follows:—

- 4-6" x 4" x 3/8" Angles latticed by 5/16" bars,

properties to be based on the gross section of angles, no deductions being made for rivet holes nor any allowance for lattice bars.

Determine the distances, z , of center lines of gravity of angles from neutral axes 1-1 and 2-2 in accordance with the dimensions given, then for

AXIS 1-1	I_{1-1} of 4-6" x 4" x 3/8" Angles	= 4 x 4.9	= 19.60 Inches ⁴
	Az^2 of 4-6" x 4" x 3/8" "	= 4 x 3.61 x 5.06 ²	= 369.72 "
	Moment of Inertia, gross section		389.32 Inches ⁴

$$\text{Radius of Gyration, " " } = \sqrt{\frac{389.32}{14.44}} = 5.19 \text{ Inches}$$

AXIS 2-2 From tables of radii of gyration for 2 angles placed back to back axis 2-2, 3/8" apart, r_{2-2} of 4-6" x 4" x 3/8" angles = 2.97 Inches.

Where sections are assembled without any web or flange plates, as, for example, latticed channel columns or latticed angle struts, the radius of gyration, r_{1-1} can be readily obtained, without considering the moment of inertia, from the radius of gyration, r of one section about its neutral axis, and the distance, z , between the center of gravity of the section and the neutral axis parallel to the axis of section.

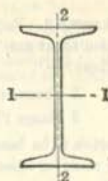
$$r_{1-1} = \sqrt{\frac{I + Az^2}{A}}, \text{ where } \frac{I}{A} = r^2, \text{ and } r_{1-1} = \sqrt{r^2 + z^2}$$

Thus, in the above example,

$$r_{1-1} = \sqrt{1.17^2 + 5.06^2} = 5.19 \text{ Inches}$$

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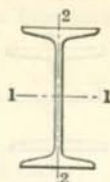
ELEMENTS OF STRUCTURAL BEAMS



Section Index	Depth of Beam	Weight per Foot	Area of Section	Width of Flange	Thickness of Web	Axis 1-1			Axis 2-2		
						I	r	S	i	r	S
						In. ⁴	In.	In. ³	In. ⁴	In.	In. ³
B 18	24	120.0	35.13	8.048	0.798	3010.8	9.26	250.9	84.9	1.56	21.1
		115.0	33.67	7.987	0.737	2940.5	9.35	245.0	82.8	1.57	20.7
		110.0	32.18	7.925	0.675	2869.1	9.44	239.1	80.6	1.58	20.3
		105.9	30.98	7.875	0.625	2811.5	9.53	234.3	78.9	1.60	20.0
B 1	24	100.0	29.25	7.247	0.747	2371.8	9.05	197.6	48.4	1.29	13.4
		95.0	27.79	7.186	0.686	2301.5	9.08	191.8	47.0	1.30	13.0
		90.0	26.30	7.124	0.624	2230.1	9.21	185.8	45.5	1.32	12.8
		85.0	24.84	7.063	0.563	2159.8	9.33	180.0	44.2	1.33	12.5
		79.9	23.33	7.000	0.500	2087.2	9.46	173.9	42.9	1.36	12.2
B 2	20	100.0	29.20	7.273	0.873	1648.3	7.51	164.8	52.4	1.34	14.4
		95.0	27.74	7.200	0.800	1599.7	7.59	160.0	50.5	1.35	14.0
		90.0	26.26	7.126	0.726	1550.3	7.68	155.0	48.7	1.36	13.7
		85.0	24.80	7.053	0.653	1501.7	7.78	150.2	47.0	1.38	13.3
		81.4	23.74	7.000	0.600	1466.3	7.86	146.6	45.8	1.39	13.1
B 3	20	75.0	21.90	6.391	0.641	1263.5	7.60	126.3	30.1	1.17	9.4
		70.0	20.42	6.317	0.567	1214.2	7.71	121.4	28.9	1.19	9.2
		65.4	19.08	6.250	0.500	1169.5	7.83	116.9	27.9	1.21	8.9
B 19	18	90.0	26.29	7.236	0.796	1256.5	6.91	139.6	51.9	1.40	14.3
		85.0	24.81	7.154	0.714	1216.6	7.00	135.2	49.8	1.42	14.0
		80.0	23.34	7.072	0.632	1176.8	7.10	130.8	47.9	1.43	13.6
		75.6	22.04	7.000	0.560	1141.8	7.20	126.9	46.3	1.45	13.2
B 4	18	70.0	20.46	6.251	0.711	917.5	6.70	101.9	24.5	1.09	7.8
		65.0	18.98	6.169	0.629	877.7	6.80	97.5	23.4	1.11	7.6
		60.0	17.50	6.087	0.547	837.8	6.92	93.1	22.3	1.13	7.3
		54.7	15.94	6.000	0.460	795.5	7.07	88.4	21.2	1.15	7.1
B 6	15	75.0	21.85	6.278	0.868	687.2	5.61	91.6	30.6	1.18	9.8
		70.0	20.38	6.180	0.770	659.6	5.69	87.9	28.8	1.19	9.3
		65.0	18.91	6.082	0.672	632.1	5.78	84.3	27.2	1.20	8.9
		60.8	17.68	6.000	0.590	609.0	5.87	81.2	26.0	1.21	8.7
B 7	15	55.0	16.06	5.738	0.648	508.7	5.63	67.8	17.0	1.03	5.9
		50.0	14.59	5.640	0.550	481.1	5.74	64.2	16.0	1.05	5.7
		45.0	13.12	5.542	0.452	453.6	5.88	60.5	15.0	1.07	5.4
		42.9	12.49	5.500	0.410	441.8	5.95	58.9	14.6	1.08	5.3

ELEMENTS OF SECTIONS

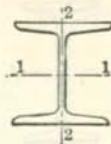
ELEMENTS OF STRUCTURAL BEAMS—Concluded



Section Index	Depth of Beam	Weight per Foot	Area of Section	Width of Flange	Thickness of Web	Axis 1-1			Axis 2-2		
						I	r	S	I	r	S
						In. ⁴	In.	In. ³	In. ⁴	In.	In. ³
B 8	12	55.0	16.04	5.600	0.810	319.3	4.46	53.2	17.3	1.04	6.2
		50.0	14.57	5.477	0.687	301.6	4.55	50.3	16.0	1.05	5.8
		45.0	13.10	5.355	0.565	284.1	4.66	47.3	14.8	1.06	5.5
		40.8	11.84	5.250	0.460	268.9	4.77	44.8	13.8	1.08	5.3
B 9	12	35.0	10.20	5.078	0.428	227.0	4.72	37.8	10.0	0.99	3.9
		31.8	9.26	5.000	0.350	215.8	4.83	36.0	9.5	1.01	3.8
B 10	10	40.0	11.69	5.091	0.741	158.0	3.68	31.6	9.4	0.90	3.7
		35.0	10.22	4.944	0.594	145.8	3.78	29.2	8.5	0.91	3.4
		30.0	8.75	4.797	0.447	133.5	3.91	26.7	7.6	0.93	3.2
		25.4	7.38	4.660	0.310	122.1	4.07	24.4	6.9	0.97	3.0
B 11	9	35.0	10.22	4.764	0.724	111.3	3.30	24.7	7.3	0.84	3.0
		30.0	8.76	4.601	0.561	101.4	3.40	22.5	6.4	0.85	2.8
		25.0	7.28	4.437	0.397	91.4	3.54	20.3	5.6	0.88	2.5
		21.8	6.32	4.330	0.290	84.9	3.67	18.9	5.2	0.90	2.4
B 12	8	25.5	7.43	4.262	0.532	68.1	3.03	17.0	4.7	0.80	2.2
		23.0	6.71	4.171	0.441	64.2	3.09	16.0	4.4	0.81	2.1
		20.5	5.97	4.079	0.349	60.2	3.18	15.1	4.0	0.82	2.0
		18.4	5.34	4.000	0.270	56.9	3.26	14.2	3.8	0.84	1.9
B 13	7	20.0	5.83	3.860	0.450	41.9	2.68	12.0	3.1	0.74	1.6
		17.5	5.09	3.755	0.345	38.9	2.77	11.1	2.9	0.76	1.6
		15.3	4.43	3.660	0.250	36.2	2.86	10.4	2.7	0.78	1.5
B 14	6	17.25	5.02	3.565	0.465	26.0	2.28	8.7	2.3	0.68	1.3
		14.75	4.29	3.443	0.343	23.8	2.36	7.9	2.1	0.69	1.2
		12.5	3.61	3.330	0.230	21.8	2.46	7.3	1.8	0.72	1.1
B 15	5	14.75	4.29	3.284	0.494	15.0	1.87	6.0	1.7	0.63	1.0
		12.25	3.56	3.137	0.347	13.5	1.95	5.4	1.4	0.63	0.91
		10.0	2.87	3.000	0.210	12.1	2.05	4.8	1.2	0.65	0.82
B 16	4	10.5	3.05	2.870	0.400	7.1	1.52	3.5	1.0	0.57	0.70
		9.5	2.76	2.796	0.326	6.7	1.56	3.3	0.91	0.58	0.65
		8.5	2.46	2.723	0.253	6.3	1.60	3.2	0.83	0.58	0.61
		7.7	2.21	2.660	0.190	6.0	1.64	3.0	0.77	0.59	0.58
B 17	3	7.5	2.17	2.509	0.349	2.9	1.15	1.9	0.59	0.52	0.47
		6.5	1.88	2.411	0.251	2.7	1.19	1.8	0.51	0.52	0.43
		5.7	1.64	2.330	0.170	2.5	1.23	1.7	0.46	0.53	0.40

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ELEMENTS OF H BEAMS



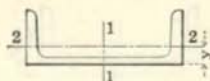
Section Index	Depth of Beam	Weight per Foot	Area of Section	Width of Flange	Thickness of Web	Axis 1-1			Axis 2-2		
						I	r	S	I	r	S
						In. ⁴	In.	In. ³	In. ⁴	In.	In. ³
H 4	8	37.7	11.00	8.125	0.500	120.8	3.31	30.2	36.9	1.83	9.1
		34.3	10.00	8.000	0.375	115.5	3.40	28.9	35.1	1.87	8.8
		32.6	9.50	7.938	0.313	112.8	3.45	28.2	34.2	1.90	8.6
H 3	6	26.7	7.76	6.125	0.438	47.4	2.47	15.8	15.7	1.42	5.1
		24.1	7.01	6.000	0.313	45.1	2.54	15.0	14.7	1.45	4.9
		22.8	6.63	5.938	0.250	44.0	2.58	14.7	14.2	1.46	4.8
H 2	5	18.9	5.47	5.000	0.313	23.8	2.08	9.5	7.8	1.20	3.1
H 1	4	13.8	3.99	4.000	0.313	10.7	1.64	5.3	3.6	0.95	1.8

ELEMENTS OF U. S. STEEL SHEET PILING SECTIONS



Section Index	Dimensions				Weight per Foot	Area of Section	Axis 1-1		
	b	c	d	t			I	r	S
	In.	In.	In.	In.			In. ⁴	In.	In. ³
M 105	13 1/4	3 15/16	2 1/2	1/2	42.5	12.51	8.56	0.83	4.35
M 104	13 1/4	3 15/16	2 1/2	3/8	38	11.30	8.50	0.87	4.32

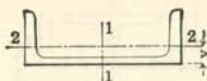
ELEMENTS OF SECTIONS

ELEMENTS OF STRUCTURAL CHANNELS
American Standard Sections

Section Index	Depth of Channel	Weight per Foot	Area of Section	Width of Flange	Thick-ness of Web	Axis 1-1			Axis 2-2			
						I	r	S	I	r	S	y
						In. ⁴	In.	In. ³	In. ⁴	In.	In. ³	In.
C 1	15	55.0	16.11	3.814	0.814	429.0	5.16	57.2	12.1	0.87	4.1	0.82
		50.0	14.64	3.716	0.716	401.4	5.24	53.6	11.2	0.87	3.8	0.80
		45.0	13.17	3.618	0.618	373.9	5.33	49.8	10.3	0.88	3.6	0.79
		40.0	11.70	3.520	0.520	346.3	5.44	46.2	9.3	0.89	3.4	0.78
		35.0	10.23	3.422	0.422	318.7	5.58	42.5	8.4	0.91	3.2	0.79
		33.9	9.90	3.400	0.400	312.6	5.62	41.7	8.2	0.91	3.2	0.79
C 2	12	40.0	11.73	3.415	0.755	196.5	4.09	32.8	6.6	0.75	2.5	0.72
		35.0	10.26	3.292	0.632	178.8	4.18	29.8	5.9	0.76	2.3	0.69
		30.0	8.79	3.170	0.510	161.2	4.28	26.9	5.2	0.77	2.1	0.68
		25.0	7.32	3.047	0.387	143.5	4.43	23.9	4.5	0.79	1.9	0.68
		20.7	6.03	2.940	0.280	128.1	4.61	21.4	3.9	0.81	1.7	0.70
C 3	10	35.0	10.27	3.180	0.820	115.2	3.34	23.0	4.6	0.67	1.9	0.69
		30.0	8.80	3.033	0.673	103.0	3.42	20.6	4.0	0.67	1.7	0.65
		25.0	7.33	2.886	0.526	90.7	3.52	18.1	3.4	0.68	1.5	0.62
		20.0	5.86	2.739	0.379	78.5	3.66	15.7	2.8	0.70	1.3	0.61
		15.3	4.47	2.600	0.240	66.9	3.87	13.4	2.3	0.72	1.2	0.64
C 4	9	25.0	7.33	2.812	0.612	70.5	3.10	15.7	3.0	0.64	1.4	0.61
		20.0	5.86	2.648	0.448	60.6	3.22	13.5	2.4	0.65	1.2	0.59
		15.0	4.39	2.485	0.285	50.7	3.40	11.3	1.9	0.67	1.0	0.59
		13.4	3.89	2.430	0.230	47.3	3.49	10.5	1.8	0.67	0.97	0.61
		21.25	6.23	2.619	0.579	47.6	2.77	11.9	2.2	0.60	1.1	0.59
C 5	8	18.75	5.49	2.527	0.487	43.7	2.82	10.9	2.0	0.60	1.0	0.57
		16.25	4.76	2.435	0.395	39.8	2.89	9.9	1.8	0.61	0.94	0.56
		13.75	4.02	2.343	0.303	35.8	2.99	9.0	1.5	0.62	0.86	0.56
		11.5	3.36	2.260	0.220	32.3	3.10	8.1	1.3	0.63	0.79	0.58
		19.75	5.79	2.509	0.629	33.1	2.39	9.4	1.8	0.56	0.96	0.58
C 6	7	17.25	5.05	2.404	0.524	30.1	2.44	8.6	1.6	0.56	0.86	0.55
		14.75	4.32	2.299	0.419	27.1	2.51	7.7	1.4	0.57	0.79	0.53
		12.25	3.58	2.194	0.314	24.1	2.59	6.9	1.2	0.58	0.71	0.53
		9.8	2.85	2.090	0.210	21.1	2.72	6.0	0.98	0.59	0.63	0.55
		15.5	4.54	2.279	0.559	19.5	2.07	6.5	1.3	0.53	0.73	0.55
C 7	6	13.0	3.81	2.157	0.437	17.3	2.13	5.8	1.1	0.53	0.65	0.52
		10.5	3.07	2.034	0.314	15.1	2.22	5.0	0.87	0.53	0.57	0.50
		8.2	2.39	1.920	0.200	13.0	2.34	4.3	0.70	0.54	0.50	0.52
		11.5	3.36	2.032	0.472	10.4	1.76	4.1	0.82	0.49	0.54	0.51
		9.0	2.63	1.885	0.325	8.8	1.83	3.5	0.64	0.49	0.45	0.48
C 8	5	6.7	1.95	1.750	0.190	7.4	1.95	3.0	0.48	0.50	0.38	0.49
		7.25	2.12	1.720	0.320	4.5	1.47	2.3	0.44	0.46	0.35	0.46
		6.25	1.82	1.647	0.247	4.1	1.50	2.1	0.38	0.45	0.32	0.46
		5.4	1.56	1.580	0.180	3.8	1.56	1.9	0.32	0.45	0.29	0.46
		6.0	1.75	1.596	0.356	2.1	1.08	1.4	0.31	0.42	0.27	0.46
C 9	4	5.0	1.46	1.498	0.258	1.8	1.12	1.2	0.25	0.41	0.24	0.44
		4.1	1.19	1.410	0.170	1.6	1.17	1.1	0.20	0.41	0.21	0.44

CARNEGIE STEEL COMPANY

ELEMENTS OF SHIP BUILDING CHANNELS
American Standard Sections



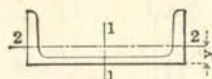
Section Index	Depth of Channel	Wt. per Foot	Area of Section	Width of Flange	Thick-ness of Web	Axis 1-1			Axis 2-2			
						I	r	S	I	r	S	y
						In. ⁴	In.	In. ³	In. ⁴	In.	In. ³	In.
†C 60	18	58.0	16.98	4.200	.700	670.7	6.29	74.5	18.5	1.04	5.6	0.88
		51.9	15.18	4.100	.600	622.1	6.40	69.1	17.1	1.06	5.3	0.87
		45.8	13.38	4.000	.500	573.5	6.55	63.7	15.8	1.09	5.1	0.89
		42.7	12.48	3.950	.450	549.2	6.64	61.0	15.0	1.10	4.9	0.90
C 21 (BSC 26)	12	44.7	13.05	4.200	.725	245.0	4.33	40.8	16.8	1.14	5.3	1.04
		40.6	11.85	4.100	.625	230.6	4.41	38.4	15.5	1.15	5.1	1.04
		36.5	10.65	4.000	.525	216.2	4.51	36.0	14.2	1.16	4.8	1.06
		34.5	10.05	3.950	.475	209.0	4.57	34.8	13.5	1.16	4.7	1.07
C 171 (BSC 25)	12	41.1	12.00	3.700	.700	217.8	4.26	36.3	11.3	0.97	4.0	0.89
		37.0	10.80	3.600	.600	203.4	4.34	33.9	10.3	0.98	3.8	0.89
		32.9	9.60	3.500	.500	189.0	4.44	31.5	9.4	0.99	3.6	0.89
		30.9	9.00	3.450	.450	181.8	4.50	30.3	8.9	0.99	3.5	0.90
C 26 (BSC 21)	10	37.0	10.81	4.200	.675	146.3	3.68	29.3	14.9	1.18	4.8	1.10
		33.6	9.81	4.100	.575	138.0	3.75	27.6	13.7	1.18	4.6	1.11
		30.2	8.81	4.000	.475	129.7	3.84	25.9	12.5	1.19	4.3	1.13
		28.5	8.31	3.950	.425	125.5	3.89	25.1	11.8	1.19	4.2	1.15
C 27 (BSC 20)	10	35.1	10.23	3.700	.675	133.6	3.61	26.7	10.4	1.01	3.8	0.95
		31.7	9.23	3.600	.575	125.2	3.69	25.0	9.5	1.01	3.6	0.95
		28.3	8.23	3.500	.475	116.9	3.77	23.4	8.6	1.02	3.4	0.96
		26.6	7.73	3.450	.425	112.7	3.82	22.5	8.1	1.02	3.3	0.97
	24.9	7.23	3.400	.375	108.6	3.88	21.7	7.6	1.03	3.2	0.98	
C 28 (BSC 19)	10	25.3	7.38	3.550	.425	106.0	3.79	21.2	7.9	1.04	3.0	0.94
		23.6	6.88	3.500	.375	101.8	3.85	20.4	7.5	1.04	2.9	0.96
		21.9	6.38	3.450	.325	97.6	3.91	19.5	7.0	1.05	2.8	0.98
C 31 (BSC 18)	9	34.7	10.13	4.200	.675	113.0	3.34	25.1	14.5	1.20	4.8	1.15
		31.7	9.23	4.100	.575	106.9	3.40	23.8	13.3	1.20	4.5	1.16
		28.6	8.33	4.000	.475	100.9	3.48	22.4	12.1	1.20	4.3	1.18
		27.1	7.88	3.950	.425	97.8	3.52	21.7	11.4	1.20	4.2	1.20
C 32 (BSC 17)	9	31.6	9.21	3.700	.650	99.4	3.29	22.1	9.7	1.03	3.6	0.98
		28.5	8.31	3.600	.550	93.4	3.35	20.7	8.8	1.03	3.4	0.98
		25.4	7.41	3.500	.450	87.3	3.43	19.4	8.0	1.04	3.2	1.00
		23.9	6.96	3.450	.400	84.3	3.48	18.7	7.5	1.04	3.1	1.01
C 36 (BSC 13)	8	28.2	8.23	3.700	.625	71.8	2.95	18.0	9.0	1.05	3.4	1.02
		25.5	7.43	3.600	.525	67.6	3.02	16.9	8.2	1.05	3.2	1.02
		22.8	6.63	3.500	.425	63.3	3.09	15.8	7.4	1.05	3.0	1.04
		21.4	6.23	3.450	.375	61.2	3.13	15.3	6.9	1.05	2.9	1.05
C 37 (BSC 12)	8	25.5	7.43	3.225	.600	62.6	2.90	15.6	5.8	0.89	2.5	0.86
		22.7	6.63	3.125	.500	58.3	2.97	14.6	5.3	0.89	2.3	0.85
		20.0	5.83	3.025	.400	54.0	3.05	13.5	4.7	0.90	2.2	0.86
		19.3	5.63	3.000	.375	53.0	3.07	13.2	4.5	0.90	2.1	0.87
	18.7	5.43	2.975	.350	51.9	3.09	13.0	4.4	0.90	2.1	0.88	

Dimensions and properties of the British Standard Sections are indicated in **bold type**.
†C 60 is not an American Standard Section; profile is shown with Structural Channels.

ELEMENTS OF SECTIONS

ELEMENTS OF SHIP BUILDING CHANNELS

American Standard Sections



Section Index	Depth of Channel	Wt. per Foot	Area of Section	Width of Flange	Thick-ness of Web	Axis 1-1			Axis 2-2			
						I	r	S	I	r	S	y
						In. ⁴	In. ³	In. ³	In. ⁴	In.	In. ³	In.
C 41 (BSC 10)	7	25.0	7.30	3.7000	0.600	49.9	2.62	14.3	8.3	1.07	3.2	1.06
		22.7	6.60	3.6000	0.500	47.1	2.67	13.5	7.5	1.07	3.0	1.07
		20.3	5.90	3.5000	0.400	44.2	2.74	12.6	6.7	1.07	2.8	1.09
		19.1	5.55	3.4500	0.350	42.8	2.78	12.2	6.3	1.07	2.7	1.11
C 42 (BSC 9)	7	20.0	5.82	3.1000	0.475	40.2	2.63	11.5	4.7	0.90	2.1	0.88
		17.6	5.12	3.0000	0.375	37.3	2.70	10.7	4.2	0.90	2.0	0.90
		16.4	4.77	2.9500	0.325	35.9	2.74	10.2	3.9	0.90	1.9	0.91
C 46 (BSC 8)	6	22.0	6.42	3.7000	0.575	33.0	2.27	11.0	7.6	1.09	2.9	1.12
		20.0	5.82	3.6000	0.475	31.2	2.32	10.4	6.9	1.09	2.8	1.13
		18.0	5.22	3.5000	0.375	29.4	2.38	9.8	6.1	1.08	2.6	1.15
		16.9	4.92	3.4500	0.325	28.5	2.41	9.5	5.7	1.08	2.5	1.17
C 109	6	15.3	4.48	3.5000	0.340	25.3	2.38	8.4	5.1	1.08	2.1	1.08
C 47 (BSC 7)	6	16.3	4.75	3.0000	0.375	25.8	2.33	8.6	4.0	0.91	1.9	0.95
		15.1	4.37	2.9380	0.313	24.7	2.38	8.2	3.6	0.91	1.8	0.97
C 48 (BSC 5)	6	13.3	3.90	2.5630	0.375	19.7	2.25	6.6	2.1	0.74	1.2	0.71
		12.0	3.52	2.5000	0.313	18.6	2.30	6.2	2.0	0.75	1.1	0.72

Dimensions and properties of the British Standard Sections are indicated in **bold type**.

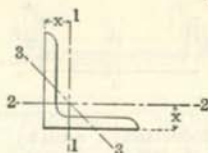
ELEMENTS OF CAR BUILDING CHANNELS

†C 20	13	50.0	14.66	4.4120	0.787	312.9	4.62	48.1	16.7	1.07	4.9	0.98
		45.0	13.18	4.2980	0.673	292.0	4.71	44.9	15.3	1.08	4.6	0.97
		40.0	11.71	4.1850	0.560	271.4	4.82	41.7	13.9	1.09	4.3	0.97
		37.0	10.82	4.1170	0.492	258.9	4.89	39.8	13.0	1.10	4.2	0.98
		35.0	10.24	4.0720	0.447	250.7	4.95	38.6	12.5	1.10	4.0	0.99
		31.8	9.30	4.0000	0.375	237.5	5.05	36.5	11.6	1.11	3.9	1.01
†C 170	12	50.0	14.64	4.1350	0.835	268.1	4.28	44.7	17.8	1.10	5.8	1.06
		48.6	14.22	4.1000	0.800	263.0	4.30	43.8	17.3	1.10	5.7	1.05
		46.6	13.62	4.0500	0.750	255.8	4.33	42.6	16.6	1.11	5.5	1.05
		44.5	13.02	4.0000	0.700	248.6	4.37	41.4	16.0	1.11	5.4	1.05
		40.0	11.70	3.8900	0.590	232.8	4.46	38.8	14.5	1.12	5.1	1.05
		35.0	10.23	3.7670	0.467	215.1	4.59	35.8	12.9	1.12	4.8	1.07
C 211	7	18.8	5.48	4.0000	0.350	42.9	2.80	12.2	8.3	1.23	3.0	1.23
C 200	4	13.8	4.00	2.5000	0.500	8.8	1.49	4.4	2.2	0.74	1.4	0.86
C 190	3	7.1	2.06	1.9840	0.250	2.8	1.17	1.9	0.75	0.60	0.60	0.72
C 191	3	6.5	1.89	1.8750	0.250	2.6	1.17	1.7	0.63	0.58	0.52	0.67
		5.8	1.68	1.8050	0.180	2.4	1.20	1.6	0.53	0.56	0.47	0.68

†Profiles of C 20 and C 170 are shown with Structural Channels.

CARNEGIE STEEL COMPANY

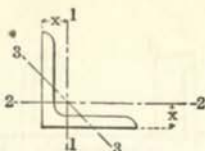
ELEMENTS OF EQUAL ANGLES



Section Index	Size	Thickness	Weight per Foot	Area of Section	Axis 1-1 and Axis 2-2				Axis 3-3
					I	r	S	x	r min.
	Inches	In.	Pounds	In. ²	In. ⁴	In.	In. ³	In.	In.
A 1	8 x 8	1 1/4	56.9	16.73	98.0	2.42	17.5	2.41	1.55
		1 1/10	54.0	15.87	93.5	2.43	16.7	2.39	1.56
		1	51.0	15.00	89.0	2.44	15.8	2.37	1.56
		15/16	48.1	14.12	84.3	2.44	14.9	2.34	1.56
		7/8	45.0	13.23	79.6	2.45	14.0	2.32	1.56
		13/16	42.0	12.34	74.7	2.46	13.1	2.30	1.57
		3/4	38.9	11.44	69.7	2.47	12.2	2.28	1.57
		11/16	35.8	10.53	64.6	2.48	11.2	2.25	1.58
		5/8	32.7	9.61	59.4	2.49	10.3	2.23	1.58
		9/16	29.6	8.68	54.1	2.50	9.3	2.21	1.58
1/2	26.4	7.75	48.6	2.51	8.4	2.19	1.58		
A 2	6 x 6	1	37.4	11.00	35.5	1.80	8.6	1.86	1.16
		15/16	35.3	10.37	33.7	1.80	8.1	1.84	1.16
		7/8	33.1	9.73	31.9	1.81	7.6	1.82	1.17
		13/16	31.0	9.09	30.1	1.82	7.2	1.80	1.17
		3/4	28.7	8.44	28.2	1.83	6.7	1.78	1.17
		11/16	26.5	7.78	26.2	1.83	6.2	1.75	1.17
		5/8	24.2	7.11	24.2	1.84	5.7	1.73	1.17
		9/16	21.9	6.43	22.1	1.85	5.1	1.71	1.18
		1/2	19.6	5.75	19.9	1.86	4.6	1.68	1.18
		7/16	17.2	5.06	17.7	1.87	4.1	1.66	1.19
3/8	14.9	4.36	15.4	1.88	3.5	1.64	1.19		
A 3	5 x 5	1	30.6	9.00	19.6	1.48	5.8	1.61	0.96
		15/16	28.9	8.50	18.7	1.48	5.5	1.59	0.96
		7/8	27.2	7.98	17.8	1.49	5.2	1.57	0.96
		13/16	25.4	7.47	16.8	1.50	4.9	1.55	0.97
		3/4	23.6	6.94	15.7	1.50	4.5	1.52	0.97
		11/16	21.8	6.40	14.7	1.51	4.2	1.50	0.97
		5/8	20.0	5.86	13.6	1.52	3.9	1.48	0.97
		9/16	18.1	5.31	12.4	1.53	3.5	1.46	0.98
		1/2	16.2	4.75	11.3	1.54	3.2	1.43	0.98
		7/16	14.3	4.18	10.0	1.55	2.8	1.41	0.98
3/8	12.3	3.61	8.7	1.56	2.4	1.39	0.99		
A 4	4 x 4	1 1/16	19.9	5.84	8.1	1.18	3.0	1.29	0.77
		3/4	18.5	5.44	7.7	1.19	2.8	1.27	0.77
		11/16	17.1	5.03	7.2	1.19	2.6	1.25	0.77
		5/8	15.7	4.61	6.7	1.20	2.4	1.23	0.77
		9/16	14.3	4.18	6.1	1.21	2.2	1.21	0.78
		1/2	12.8	3.75	5.6	1.22	2.0	1.18	0.78
		7/16	11.3	3.31	5.0	1.23	1.8	1.16	0.78
		3/4	9.8	2.86	4.4	1.23	1.5	1.14	0.79
		5/16	8.2	2.40	3.7	1.24	1.3	1.12	0.79
		1/4	6.6	1.94	3.0	1.25	1.0	1.09	0.79

ELEMENTS OF SECTIONS

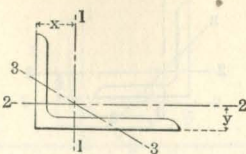
ELEMENTS OF EQUAL ANGLES—Concluded



Section Index	Size	Thickness	Weight per Foot	Area of Section	Axis 1-1 and Axis 2-2				Axis 3-3
					I	r	S	x	r min.
	Inches	In.	Pounds	In. ²	In. ⁴	In.	In. ³	In.	In.
A 5	3½ x 3½	1¼	17.1	5.03	5.3	1.02	2.3	1.17	0.67
		¾	16.0	4.69	5.0	1.03	2.1	1.15	0.67
		11⁄16	14.8	4.34	4.7	1.04	2.0	1.12	0.67
		5⁄8	13.6	3.98	4.3	1.04	1.8	1.10	0.68
		9⁄16	12.4	3.62	4.0	1.05	1.6	1.08	0.68
		1⁄2	11.1	3.25	3.6	1.06	1.5	1.06	0.68
		7⁄16	9.8	2.87	3.3	1.07	1.3	1.04	0.68
		3⁄8	8.5	2.48	2.9	1.07	1.2	1.01	0.69
		5⁄16	7.2	2.09	2.5	1.08	0.98	0.99	0.69
		1⁄4	5.8	1.69	2.0	1.09	0.79	0.97	0.69
A 7	3 x 3	5⁄8	11.5	3.36	2.6	0.88	1.3	0.98	0.57
		9⁄16	10.4	3.06	2.4	0.89	1.2	0.95	0.58
		1⁄2	9.4	2.75	2.2	0.90	1.1	0.93	0.58
		7⁄16	8.3	2.43	2.0	0.91	0.95	0.91	0.58
		3⁄8	7.2	2.11	1.8	0.91	0.83	0.89	0.58
		5⁄16	6.1	1.78	1.5	0.92	0.71	0.87	0.59
		1⁄4	4.9	1.44	1.2	0.93	0.58	0.84	0.59
		1⁄2	7.7	2.25	1.2	0.74	0.73	0.81	0.47
		7⁄16	6.8	2.00	1.1	0.75	0.65	0.78	0.48
		3⁄8	5.9	1.73	0.98	0.75	0.57	0.76	0.48
A 9	2½ x 2½	5⁄16	5.0	1.47	0.85	0.76	0.48	0.74	0.49
		1⁄4	4.1	1.19	0.70	0.77	0.39	0.72	0.49
		3⁄16	3.07	0.90	0.55	0.78	0.30	0.69	0.49
		1⁄8	2.08	0.61	0.38	0.79	0.20	0.67	0.50
		7⁄16	5.3	1.56	0.54	0.59	0.40	0.66	0.39
		3⁄8	4.7	1.36	0.48	0.59	0.35	0.64	0.39
A 11	2 x 2	5⁄16	3.92	1.15	0.42	0.60	0.30	0.61	0.39
		1⁄4	3.19	0.94	0.35	0.61	0.25	0.59	0.39
		3⁄16	2.44	0.71	0.28	0.62	0.19	0.57	0.40
		1⁄8	1.65	0.48	0.19	0.63	0.13	0.55	0.40
		7⁄16	4.6	1.34	0.35	0.51	0.30	0.59	0.33
		3⁄8	3.99	1.17	0.31	0.51	0.26	0.57	0.34
A 12	1¾ x 1¾	5⁄16	3.39	1.00	0.27	0.52	0.23	0.55	0.34
		1⁄4	2.77	0.81	0.23	0.53	0.19	0.53	0.34
		3⁄16	2.12	0.62	0.18	0.54	0.14	0.51	0.35
		1⁄8	1.44	0.42	0.13	0.55	0.10	0.48	0.35
		3⁄8	3.35	0.98	0.19	0.44	0.19	0.51	0.29
		5⁄16	2.86	0.84	0.16	0.44	0.16	0.49	0.29
A 13	1½ x 1½	1⁄4	2.34	0.69	0.14	0.45	0.13	0.47	0.29
		3⁄16	1.80	0.53	0.11	0.46	0.10	0.44	0.29
		1⁄8	1.23	0.36	0.08	0.46	0.07	0.42	0.30
		5⁄16	2.33	0.68	0.09	0.36	0.11	0.42	0.24
		1⁄4	1.92	0.56	0.08	0.37	0.09	0.40	0.24
		3⁄16	1.48	0.43	0.06	0.38	0.07	0.38	0.24
A 15	1¼ x 1¼	1⁄8	1.01	0.30	0.04	0.38	0.05	0.35	0.25
		3⁄16	1.49	0.44	0.04	0.29	0.06	0.34	0.19
		5⁄16	1.16	0.34	0.03	0.30	0.04	0.32	0.19
		1⁄8	0.80	0.23	0.02	0.31	0.03	0.30	0.19
		3⁄16	1.49	0.44	0.04	0.29	0.06	0.34	0.19
		5⁄16	1.16	0.34	0.03	0.30	0.04	0.32	0.19
A 16	1 x 1	1⁄8	0.80	0.23	0.02	0.31	0.03	0.30	0.19

CARNEGIE STEEL COMPANY

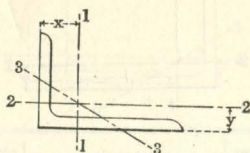
ELEMENTS OF UNEQUAL ANGLES



Section Index	Size	Thickness	Weight per Foot	Area of Section	Axis 1-1				Axis 2-2				Axis 3-3
					I	r	S	x	I	r	S	y	r min.
	Inches	In.	Lbs.	In. ²	In. ⁴	In.	In. ³	In.	In. ⁴	In.	In. ³	In.	In.
A 18 8 x 6	1	44.2	13.00	80.8	2.49	15.1	2.65	38.8	1.73	8.9	1.65	1.28	
	15/16	41.7	12.25	76.6	2.50	14.3	2.63	36.8	1.73	8.4	1.63	1.28	
	7/8	39.1	11.48	72.3	2.51	13.4	2.61	34.9	1.74	7.9	1.61	1.28	
	13/16	36.5	10.72	67.9	2.52	12.5	2.59	32.8	1.75	7.4	1.59	1.29	
	3/4	33.8	9.94	63.4	2.53	11.7	2.56	30.7	1.76	6.9	1.56	1.29	
	11/16	31.2	9.15	58.8	2.54	10.8	2.54	28.6	1.77	6.4	1.54	1.29	
	5/8	28.5	8.36	54.1	2.54	9.9	2.52	26.3	1.77	5.9	1.52	1.30	
	9/16	25.7	7.56	49.3	2.55	8.9	2.50	24.0	1.78	5.3	1.50	1.30	
	1/2	23.0	6.75	44.3	2.56	8.0	2.47	21.7	1.79	4.8	1.47	1.30	
7/16	20.2	5.93	39.2	2.57	7.1	2.45	19.3	1.80	4.2	1.45	1.30		
A 53 8 x 3 1/2	1	35.7	10.50	66.2	2.51	13.7	3.17	7.8	0.86	3.0	0.92	0.73	
	15/16	33.7	9.90	62.9	2.52	12.9	3.14	7.4	0.87	2.9	0.89	0.73	
	7/8	31.7	9.30	59.4	2.53	12.2	3.12	7.1	0.87	2.7	0.87	0.73	
	13/16	29.6	8.68	55.9	2.54	11.4	3.10	6.7	0.88	2.5	0.85	0.73	
	3/4	27.5	8.06	52.3	2.55	10.6	3.07	6.3	0.88	2.3	0.82	0.73	
	11/16	25.3	7.43	48.5	2.56	9.8	3.05	5.9	0.89	2.2	0.80	0.73	
	5/8	23.2	6.80	44.7	2.57	9.0	3.03	5.4	0.90	2.0	0.78	0.74	
	9/16	21.0	6.15	40.8	2.57	8.2	3.00	5.0	0.90	1.8	0.75	0.74	
	1/2	18.7	5.50	36.7	2.58	7.3	2.98	4.5	0.91	1.6	0.73	0.74	
7/16	16.5	4.84	32.5	2.59	6.4	2.95	4.1	0.92	1.5	0.70	0.74		
A 19 7 x 3 1/2	1	32.3	9.50	45.4	2.19	10.6	2.71	7.5	0.89	3.0	0.96	0.74	
	15/16	30.5	8.97	43.1	2.19	10.0	2.69	7.2	0.89	2.8	0.94	0.74	
	7/8	28.7	8.42	40.8	2.20	9.4	2.66	6.8	0.90	2.6	0.91	0.74	
	13/16	26.8	7.87	38.4	2.21	8.8	2.64	6.5	0.91	2.5	0.89	0.74	
	3/4	24.9	7.31	36.0	2.22	8.2	2.62	6.1	0.91	2.3	0.87	0.74	
	11/16	23.0	6.75	33.5	2.23	7.6	2.60	5.7	0.92	2.1	0.85	0.74	
	5/8	21.0	6.17	30.9	2.24	7.0	2.57	5.3	0.93	2.0	0.82	0.75	
	9/16	19.1	5.59	28.2	2.25	6.3	2.55	4.9	0.93	1.8	0.80	0.75	
	1/2	17.0	5.00	25.4	2.25	5.7	2.53	4.4	0.94	1.6	0.78	0.75	
7/16	15.0	4.40	22.6	2.26	5.0	2.50	4.0	0.95	1.4	0.75	0.76		
3/8	13.0	3.80	19.6	2.27	4.3	2.48	3.5	0.96	1.3	0.73	0.76		
A 20 6 x 4	1	30.6	9.00	30.8	1.85	8.0	2.17	10.8	1.09	3.8	1.17	0.85	
	15/16	28.9	8.50	29.3	1.86	7.6	2.14	10.3	1.10	3.6	1.14	0.85	
	7/8	27.2	7.98	27.7	1.86	7.2	2.12	9.8	1.11	3.4	1.12	0.86	
	13/16	25.4	7.47	26.1	1.87	6.7	2.10	9.2	1.11	3.2	1.10	0.86	
	3/4	23.6	6.94	24.5	1.88	6.2	2.08	8.7	1.12	3.0	1.08	0.86	
	11/16	21.8	6.40	22.8	1.89	5.8	2.06	8.1	1.13	2.8	1.06	0.86	
	5/8	20.0	5.86	21.1	1.90	5.3	2.03	7.5	1.13	2.5	1.03	0.86	
	9/16	18.1	5.31	19.3	1.90	4.8	2.01	6.9	1.14	2.3	1.01	0.87	
	1/2	16.2	4.75	17.4	1.91	4.3	1.99	6.3	1.15	2.1	0.99	0.87	
7/16	14.3	4.18	15.5	1.92	3.8	1.96	5.6	1.16	1.8	0.96	0.87		
3/8	12.3	3.61	13.5	1.93	3.3	1.94	4.9	1.17	1.6	0.94	0.88		

ELEMENTS OF SECTIONS

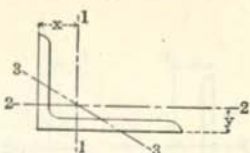
ELEMENTS OF UNEQUAL ANGLES—Continued



Section Index	Size	Thickness	Weight per Foot	Area of Section	Axis 1-1				Axis 2-2				Axis 3-3	
					I	r	S	x	I	r	S	y	r _{min.}	
	Inches	In.	Lbs.	In. ²	In. ⁴	In.	In. ³	In.	In. ⁴	In	In. ³	In.	In.	
A 21	6 x 3 1/2	1	28.9	8.50	29.2	1.85	7.8	2.26	7.2	0.92	2.9	1.01	0.74	
		15/16	27.3	8.03	27.8	1.86	7.4	2.24	6.9	0.93	2.7	0.99	0.74	
		7/8	25.7	7.55	26.4	1.87	7.0	2.22	6.6	0.93	2.6	0.97	0.75	
		13/16	24.0	7.06	24.9	1.88	6.6	2.20	6.2	0.94	2.4	0.95	0.75	
		3/4	22.4	6.56	23.3	1.89	6.1	2.18	5.8	0.94	2.3	0.93	0.75	
		11/16	20.6	6.06	21.7	1.89	5.6	2.15	5.5	0.95	2.1	0.90	0.75	
		5/8	18.9	5.55	20.1	1.90	5.2	2.13	5.1	0.96	1.9	0.88	0.75	
		9/16	17.1	5.03	18.4	1.91	4.7	2.11	4.7	0.96	1.8	0.86	0.75	
		1/2	15.3	4.50	16.6	1.92	4.2	2.08	4.3	0.97	1.6	0.83	0.76	
		7/16	13.5	3.97	14.8	1.93	3.7	2.06	3.8	0.98	1.4	0.81	0.76	
3/8	11.7	3.42	12.9	1.94	3.3	2.04	3.3	0.99	1.2	0.79	0.77			
5/16	9.8	2.87	10.9	1.95	2.7	2.01	2.9	1.00	1.0	0.76	0.77			
A 22	5 x 4	7/8	24.2	7.11	16.4	1.52	5.0	1.71	9.2	1.14	3.3	1.21	0.84	
		13/16	22.7	6.65	15.5	1.53	4.7	1.68	8.7	1.15	3.1	1.18	0.84	
		3/4	21.1	6.19	14.6	1.54	4.4	1.66	8.2	1.15	2.9	1.16	0.84	
		11/16	19.5	5.72	13.6	1.54	4.1	1.64	7.7	1.16	2.7	1.14	0.84	
		5/8	17.8	5.23	12.6	1.55	3.7	1.62	7.1	1.17	2.5	1.12	0.84	
		9/16	16.2	4.75	11.6	1.56	3.4	1.60	6.6	1.18	2.3	1.10	0.85	
		1/2	14.5	4.25	10.5	1.57	3.1	1.57	6.0	1.18	2.0	1.07	0.85	
		7/16	12.8	3.75	9.3	1.58	2.7	1.55	5.3	1.19	1.8	1.05	0.85	
3/8	11.0	3.23	8.1	1.59	2.3	1.53	4.7	1.20	1.6	1.03	0.86			
A 23	5 x 3 1/2	7/8	22.7	6.67	15.7	1.53	4.9	1.79	6.2	0.96	2.5	1.04	0.75	
		13/16	21.3	6.25	14.8	1.54	4.6	1.77	5.9	0.97	2.4	1.02	0.75	
		3/4	19.8	5.81	13.9	1.55	4.3	1.75	5.6	0.98	2.2	1.00	0.75	
		11/16	18.3	5.37	13.0	1.56	4.0	1.72	5.2	0.98	2.1	0.97	0.75	
		5/8	16.8	4.92	12.0	1.56	3.7	1.70	4.8	0.99	1.9	0.95	0.75	
		9/16	15.2	4.47	11.0	1.57	3.3	1.68	4.4	1.00	1.7	0.93	0.75	
		1/2	13.6	4.00	10.0	1.58	3.0	1.66	4.0	1.01	1.6	0.91	0.75	
		7/16	12.0	3.53	8.9	1.59	2.6	1.63	3.6	1.01	1.4	0.88	0.76	
		3/8	10.4	3.05	7.8	1.60	2.3	1.61	3.2	1.02	1.2	0.86	0.76	
		5/16	8.7	2.56	6.6	1.61	1.9	1.59	2.7	1.03	1.0	0.84	0.76	
A 24	5 x 3	13/16	19.9	5.84	14.0	1.55	4.5	1.86	3.7	0.80	1.7	0.86	0.64	
		3/4	18.5	5.44	13.2	1.55	4.2	1.84	3.5	0.80	1.6	0.84	0.64	
		11/16	17.1	5.03	12.3	1.56	3.9	1.82	3.3	0.81	1.5	0.82	0.64	
		5/8	15.7	4.61	11.4	1.57	3.5	1.80	3.1	0.81	1.4	0.80	0.64	
		9/16	14.3	4.18	10.4	1.58	3.2	1.77	2.8	0.82	1.3	0.77	0.65	
		1/2	12.8	3.75	9.5	1.59	2.9	1.75	2.6	0.83	1.1	0.75	0.65	
		7/16	11.3	3.31	8.4	1.60	2.6	1.73	2.3	0.84	1.0	0.73	0.65	
		3/8	9.8	2.86	7.4	1.61	2.2	1.70	2.0	0.84	0.89	0.70	0.65	
5/16	8.2	2.40	6.3	1.61	1.9	1.68	1.8	0.85	0.75	0.68	0.66			

CARNEGIE STEEL COMPANY

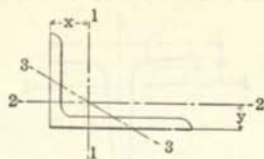
ELEMENTS OF UNEQUAL ANGLES—Continued



Section Index	Size	Thickness	Weight per Foot	Area of Section	Axis 1-1				Axis 2-2				Axis 3-3 rmin.
					I	r	S	x	I	r	S	y	
					In. ⁴	In.	In. ³	In.	In. ⁴	In.	In. ³	In.	
A 25	4 1/2 x 3	1 3/16	18.5	5.43	10.3	1.38	3.6	1.65	3.6	0.81	1.7	0.90	0.64
		3/4	17.3	5.06	9.7	1.39	3.4	1.63	3.4	0.82	1.6	0.88	0.64
		1 1/16	16.0	4.68	9.1	1.39	3.1	1.60	3.2	0.83	1.5	0.85	0.64
		5/8	14.7	4.30	8.4	1.40	2.9	1.58	3.0	0.83	1.4	0.83	0.64
		7/16	13.3	3.90	7.8	1.41	2.6	1.56	2.8	0.85	1.3	0.81	0.64
		1/2	11.9	3.50	7.0	1.42	2.4	1.54	2.5	0.85	1.1	0.79	0.65
		3/8	10.6	3.09	6.3	1.43	2.1	1.51	2.3	0.85	1.0	0.76	0.65
		1/4	9.1	2.67	5.5	1.44	1.8	1.49	2.0	0.86	0.88	0.74	0.66
A 26	4 x 3 1/2	1 3/16	18.5	5.43	7.8	1.19	2.9	1.36	5.5	1.01	2.3	1.11	0.72
		3/4	17.3	5.06	7.3	1.20	2.8	1.34	5.2	1.01	2.1	1.09	0.72
		1 1/16	16.0	4.68	6.9	1.21	2.6	1.32	4.9	1.02	2.0	1.07	0.72
		5/8	14.7	4.30	6.4	1.22	2.4	1.29	4.5	1.03	1.8	1.04	0.72
		7/16	13.3	3.90	5.9	1.23	2.1	1.27	4.2	1.03	1.7	1.02	0.72
		1/2	11.9	3.50	5.3	1.23	1.9	1.25	3.8	1.04	1.5	1.00	0.72
		3/8	10.6	3.09	4.8	1.24	1.7	1.23	3.4	1.05	1.3	0.98	0.72
		1/4	9.1	2.67	4.2	1.25	1.5	1.21	3.0	1.06	1.2	0.96	0.73
A 27	4 x 3	1 3/16	17.1	5.03	7.3	1.21	2.9	1.44	3.5	0.83	1.7	0.94	0.64
		3/4	16.0	4.69	6.9	1.22	2.7	1.42	3.3	0.84	1.6	0.92	0.64
		1 1/16	14.8	4.34	6.5	1.22	2.5	1.39	3.1	0.84	1.5	0.89	0.64
		5/8	13.6	3.98	6.0	1.23	2.3	1.37	2.9	0.85	1.4	0.87	0.64
		7/16	12.4	3.62	5.6	1.24	2.1	1.35	2.7	0.86	1.2	0.85	0.64
		1/2	11.1	3.25	5.0	1.25	1.9	1.33	2.4	0.86	1.1	0.83	0.64
		3/8	9.8	2.87	4.5	1.25	1.7	1.30	2.2	0.87	1.0	0.80	0.64
		1/4	8.5	2.48	4.0	1.26	1.5	1.28	1.9	0.88	0.87	0.78	0.64
A 28	3 1/2 x 3	1 3/16	7.2	2.09	3.4	1.27	1.2	1.26	1.7	0.89	0.74	0.76	0.65
		3/8	5.8	1.69	2.8	1.28	1.0	1.24	1.4	0.89	0.60	0.74	0.65
		1 1/16	15.8	4.62	5.0	1.04	2.2	1.23	3.3	0.85	1.7	0.98	0.62
		3/4	14.7	4.31	4.7	1.04	2.1	1.21	3.1	0.85	1.5	0.96	0.62
		1 1/16	13.6	4.00	4.4	1.05	1.9	1.19	3.0	0.86	1.4	0.94	0.62
		5/8	12.5	3.67	4.1	1.06	1.8	1.17	2.8	0.87	1.3	0.92	0.62
		7/16	11.4	3.34	3.8	1.07	1.6	1.15	2.5	0.87	1.2	0.90	0.62
		1/2	10.2	3.00	3.5	1.07	1.5	1.13	2.3	0.88	1.1	0.88	0.62
A 29	3 1/2 x 2 1/2	3/8	9.1	2.65	3.1	1.08	1.3	1.10	2.1	0.89	0.98	0.85	0.62
		1/2	7.9	2.30	2.7	1.09	1.1	1.08	1.8	0.90	0.85	0.83	0.62
		3/4	6.6	1.93	2.3	1.10	0.96	1.06	1.6	0.90	0.72	0.81	0.63
		5/8	5.4	1.56	1.9	1.11	0.78	1.04	1.3	0.91	0.58	0.79	0.63
		1 1/16	12.5	3.65	4.1	1.06	1.9	1.27	1.7	0.69	0.99	0.77	0.53
		5/8	11.5	3.36	3.8	1.07	1.7	1.25	1.6	0.69	0.92	0.75	0.53
		7/16	10.4	3.06	3.6	1.08	1.6	1.23	1.5	0.70	0.84	0.73	0.53
		1/2	9.4	2.75	3.2	1.09	1.4	1.20	1.4	0.70	0.76	0.70	0.53
A 29	3 1/2 x 2 1/2	3/8	8.3	2.43	2.9	1.09	1.3	1.18	1.2	0.71	0.68	0.68	0.54
		1/2	7.2	2.11	2.6	1.10	1.1	1.16	1.1	0.72	0.59	0.66	0.54
		3/4	6.1	1.78	2.2	1.11	0.93	1.14	0.94	0.73	0.50	0.64	0.54
		5/8	4.9	1.44	1.8	1.12	0.75	1.11	0.78	0.74	0.41	0.61	0.54

ELEMENTS OF SECTIONS

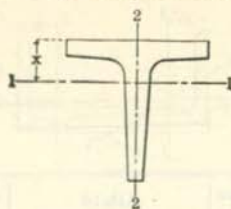
ELEMENTS OF UNEQUAL ANGLES—Concluded



Section Index	Size	Thickness	Weight per Foot	Area of Section	Axis 1-1				Axis 2-2				Axis 3-3
					I	r	S	x	I	r	S	y	rmin.
	Inches	In.	Lbs.	In.	In. ⁴	In.	In. ³	In.	In. ⁴	In.	In. ³	In.	In.
A 32	3 x 2½	⅜	9.5	2.78	2.3	0.91	1.2	1.02	1.4	0.72	0.82	0.77	0.52
		½	8.5	2.50	2.1	0.91	1.0	1.00	1.3	0.72	0.74	0.75	0.52
		⅝	7.6	2.21	1.9	0.92	0.93	0.98	1.2	0.73	0.66	0.73	0.52
		¾	6.6	1.92	1.7	0.93	0.81	0.96	1.0	0.74	0.58	0.71	0.52
		⅞	5.6	1.62	1.4	0.94	0.69	0.93	0.90	0.74	0.49	0.68	0.53
		1	4.5	1.31	1.2	0.95	0.56	0.91	0.74	0.49	0.66	0.53	
A 33	3 x 2	½	7.7	2.25	1.9	0.92	1.0	1.08	0.67	0.55	0.47	0.58	0.43
		⅝	6.8	2.00	1.7	0.93	0.89	1.06	0.61	0.55	0.42	0.56	0.43
		¾	5.9	1.73	1.5	0.94	0.78	1.04	0.54	0.56	0.37	0.54	0.43
		⅞	5.0	1.47	1.3	0.95	0.66	1.02	0.47	0.57	0.32	0.52	0.43
		1	4.1	1.19	1.1	0.95	0.54	0.99	0.39	0.57	0.26	0.49	0.43
A 35	2½ x 2	½	6.8	2.00	1.1	0.75	0.70	0.88	0.64	0.56	0.46	0.63	0.42
		⅝	6.1	1.78	1.0	0.76	0.62	0.85	0.58	0.57	0.41	0.60	0.42
		¾	5.3	1.55	0.91	0.77	0.55	0.83	0.51	0.58	0.36	0.58	0.42
		⅞	4.5	1.31	0.79	0.78	0.47	0.81	0.45	0.58	0.31	0.56	0.42
		1	3.62	1.06	0.65	0.78	0.38	0.79	0.37	0.59	0.25	0.54	0.42
			2.75	0.81	0.51	0.79	0.29	0.76	0.29	0.60	0.20	0.51	0.43
	1.86	0.55	0.35	0.80	0.20	0.74	0.20	0.61	0.13	0.49	0.43		
A 48	2½ x 1½	⅜	3.92	1.15	0.71	0.79	0.44	0.90	0.19	0.41	0.17	0.40	0.32
		½	3.19	0.94	0.59	0.79	0.36	0.88	0.16	0.41	0.14	0.38	0.32
		⅝	2.44	0.72	0.46	0.80	0.28	0.85	0.13	0.42	0.11	0.35	0.33
A 270	2½ x 1½	½	5.6	1.63	0.75	0.68	0.54	0.86	0.26	0.40	0.26	0.48	0.32
		⅝	5.0	1.45	0.68	0.69	0.48	0.83	0.24	0.41	0.23	0.46	0.32
		¾	4.4	1.27	0.61	0.69	0.42	0.81	0.21	0.41	0.20	0.44	0.32
		⅞	3.66	1.07	0.53	0.70	0.36	0.79	0.19	0.42	0.17	0.42	0.32
		1	2.98	0.88	0.44	0.71	0.30	0.77	0.16	0.42	0.14	0.39	0.32
			2.28	0.67	0.34	0.72	0.23	0.75	0.12	0.43	0.11	0.37	0.33
A 37	2 x 1½	⅜	3.99	1.17	0.43	0.61	0.34	0.71	0.21	0.42	0.20	0.46	0.32
		½	3.39	1.00	0.38	0.62	0.29	0.69	0.18	0.42	0.17	0.44	0.32
		⅝	2.77	0.81	0.32	0.62	0.24	0.66	0.15	0.43	0.14	0.41	0.32
		¾	2.12	0.62	0.25	0.63	0.18	0.64	0.12	0.44	0.11	0.39	0.32
		⅞	1.44	0.42	0.17	0.64	0.13	0.62	0.09	0.45	0.08	0.37	0.33
A 645	2 x 1¼	¼	2.55	0.75	0.30	0.63	0.23	0.71	0.09	0.34	0.10	0.33	0.27
		⅝	1.96	0.57	0.23	0.64	0.18	0.69	0.07	0.35	0.08	0.31	0.27
A 39	1¾ x 1¼	¼	2.34	0.69	0.20	0.54	0.18	0.60	0.09	0.35	0.10	0.35	0.27
		⅝	1.80	0.53	0.16	0.55	0.14	0.58	0.07	0.36	0.08	0.33	0.27
		¾	1.23	0.36	0.11	0.56	0.09	0.56	0.05	0.37	0.05	0.31	0.27
A 624	1½ x 1¼	⅝	2.59	0.76	0.16	0.45	0.16	0.52	0.10	0.35	0.11	0.40	0.26
		¾	2.13	0.63	0.13	0.46	0.13	0.50	0.08	0.36	0.09	0.38	0.26
		⅞	1.64	0.48	0.10	0.46	0.10	0.48	0.07	0.37	0.07	0.35	0.26

CARNEGIE STEEL COMPANY

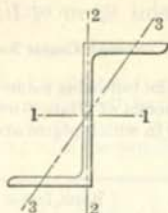
ELEMENTS OF EQUAL AND UNEQUAL TEES



Section Index	Size				Weight per Foot	Area of Section	Axis 1-1				Axis 2-2		
	Flange	Stem	Minimum Thickness				I	r	S	x	I	r	S
			Flange	Stem									
	In.	In.	In.	In.			Lbs.	In. ²	In. ⁴	In.	In. ³	In.	In. ⁴
EQUAL TEES													
T 40	6½	6½	0.40	0.45	19.8	5.80	23.5	2.01	5.0	1.76	10.1	1.32	3.1
T 1	4	4	½	½	13.5	3.97	5.7	1.20	2.0	1.18	2.8	0.84	1.4
T 2	4	4	¾	¾	10.5	3.09	4.5	1.21	1.6	1.13	2.1	0.83	1.1
T 8	3	3	¾	¾	7.8	2.27	1.8	0.90	0.86	0.88	0.90	0.63	0.60
T 9	3	3	⅝	⅝	6.7	1.95	1.6	0.90	0.74	0.86	0.75	0.62	0.50
T 10	2½	2½	¾	¾	6.4	1.87	1.0	0.74	0.59	0.76	0.52	0.53	0.42
T 11	2½	2½	⅝	⅝	5.5	1.60	0.88	0.74	0.50	0.74	0.44	0.52	0.35
T 12	2¼	2¼	⅝	⅝	4.9	1.43	0.65	0.67	0.41	0.68	0.33	0.48	0.29
T 13	2¼	2¼	¼	¼	4.1	1.19	0.52	0.66	0.32	0.65	0.25	0.46	0.22
T 14	2	2	⅝	⅝	4.3	1.26	0.44	0.59	0.31	0.61	0.23	0.43	0.23
T 15	2	2	¼	¼	3.56	1.05	0.37	0.59	0.26	0.59	0.18	0.42	0.18
UNEQUAL TEES													
T 50	5	3	¾	1½	11.5	3.37	2.4	0.84	1.1	0.76	3.9	1.10	1.6
T 57	4	5	½	½	15.3	4.50	10.8	1.55	3.1	1.56	2.8	0.79	1.4
T 58	4	5	¾	¾	11.9	3.49	8.5	1.56	2.4	1.51	2.1	0.78	1.1
T 59	4	4½	½	½	14.4	4.23	7.9	1.37	2.5	1.37	2.8	0.81	1.4
T 60	4	4½	¾	¾	11.2	3.29	6.3	1.39	2.0	1.31	2.1	0.80	1.1
T 61	4	3	¾	¾	9.2	2.68	2.0	0.86	0.90	0.78	2.1	0.89	1.1
T 44	4	3	⅝	⅝	7.8	2.29	1.7	0.87	0.77	0.75	1.8	0.88	0.88
T 62	4	2½	¾	¾	8.5	2.48	1.2	0.69	0.62	0.62	2.1	0.92	1.0
T 63	4	2½	⅝	⅝	7.2	2.12	1.0	0.69	0.53	0.60	1.8	0.91	0.88
T 79	3	2½	⅝	⅝	6.1	1.77	0.94	0.73	0.52	0.68	0.75	0.65	0.50
T 83	2½	3	⅝	⅝	6.1	1.77	1.5	0.92	0.72	0.92	0.44	0.50	0.35
T519	1½	2	⅝	⅝	2.45	0.72	0.27	0.61	0.19	0.63	0.06	0.92	0.08
T605	1½	1¼	¼	¼	1.25	0.37	0.05	0.37	0.05	0.33	0.04	0.32	0.05
T603	1¼	¾	No. 9	¼	0.88	0.26	0.01	0.16	0.01	0.16	0.02	0.31	0.04

ELEMENTS OF SECTIONS

ELEMENTS OF ZEES



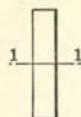
Section Index	Size			Weight per Foot	Area of Section	Axis 1-1			Axis 2-2			Axis 3-3
	Depth	Flanges	Thickness			I	r	S	I	r	S	r min.
	In.	In.	In.			In. ⁴	In.	In. ²	In. ⁴	In.	In. ²	In.
Z 3	6½	3¾	¾	34.6	10.17	50.2	2.22	16.4	19.2	1.37	6.0	0.83
	6½	3¾	11/16	32.0	9.40	46.1	2.22	15.2	17.3	1.36	5.5	0.82
	6	3½	¾	29.4	8.63	42.1	2.21	14.0	15.4	1.34	4.9	0.81
Z 2	6½	3¾	11/16	28.1	8.25	43.2	2.29	14.1	16.3	1.41	5.0	0.84
	6½	3¾	¾	25.4	7.46	38.9	2.28	12.8	14.4	1.39	4.4	0.82
	6	3½	9/16	22.8	6.68	34.6	2.28	11.5	12.6	1.37	3.9	0.81
Z 1	6½	3¾	½	21.1	6.19	34.4	2.36	11.2	12.9	1.44	3.8	0.84
	6½	3¾	7/16	18.4	5.39	29.8	2.35	9.8	11.0	1.43	3.3	0.83
	6	3½	¾	15.7	4.59	25.3	2.35	8.4	9.1	1.41	2.8	0.83
Z 6	5½	3¾	11/16	28.4	8.33	28.7	1.86	11.2	14.4	1.31	4.8	0.76
	5½	3¾	¾	26.0	7.64	26.2	1.85	10.3	12.8	1.30	4.4	0.74
	5	3½	11/16	23.7	6.96	23.7	1.84	9.5	11.4	1.28	3.9	0.73
Z 5	5½	3¾	¾	22.6	6.64	24.5	1.92	9.6	12.1	1.35	3.9	0.76
	5½	3¾	9/16	20.2	5.94	21.8	1.91	8.6	10.5	1.33	3.5	0.75
	5	3½	½	17.9	5.25	19.2	1.91	7.7	9.1	1.31	3.0	0.74
Z 4	5½	3¾	7/16	16.4	4.81	19.1	1.99	7.4	9.2	1.38	2.9	0.77
	5½	3¾	¾	14.0	4.10	16.2	1.99	6.4	7.7	1.37	2.5	0.76
	5	3½	9/16	11.6	3.40	13.4	1.98	5.3	6.2	1.35	2.0	0.75
Z 9	4½	3¾	¾	23.0	6.75	15.0	1.49	7.3	11.2	1.29	4.0	0.68
	4½	3¾	11/16	20.9	6.14	13.5	1.48	6.7	10.0	1.27	3.6	0.67
	4	3½	¾	18.9	5.55	12.1	1.48	6.1	8.7	1.25	3.2	0.66
Z 8	4½	3¾	9/16	18.0	5.27	12.7	1.55	6.2	9.3	1.33	3.2	0.68
	4½	3¾	½	15.9	4.66	11.2	1.55	5.5	8.0	1.31	2.8	0.67
	4	3½	7/16	13.8	4.05	9.7	1.55	4.8	6.7	1.29	2.4	0.66
Z 7	4½	3¾	¾	12.5	3.66	9.6	1.62	4.7	6.8	1.36	2.3	0.69
	4½	3¾	9/16	10.3	3.03	7.9	1.62	3.9	5.5	1.34	1.8	0.68
	4	3½	¾	8.2	2.41	6.3	1.62	3.1	4.2	1.33	1.4	0.67
Z 12	3½	2¾	9/16	14.3	4.18	5.3	1.12	3.4	5.7	1.17	2.3	0.54
	3	2½	½	12.6	3.69	4.6	1.12	3.1	4.9	1.15	2.0	0.53
Z 11	3½	2¾	7/16	11.5	3.36	4.6	1.17	3.0	4.8	1.19	1.9	0.55
	3	2½	¾	9.8	2.86	3.9	1.16	2.6	3.9	1.17	1.6	0.54
Z 10	3½	2¾	9/16	8.5	2.48	3.6	1.21	2.4	3.6	1.21	1.4	0.56
	3	2½	¾	6.7	1.97	2.9	1.21	1.9	2.8	1.19	1.1	0.55

CARNEGIE STEEL COMPANY

MOMENTS OF INERTIA OF RECTANGLES

IN WIDTHS FROM 1/4 TO 5/8 INCH AND 1 INCH

Neutral Axis Through Center Normal to Depth



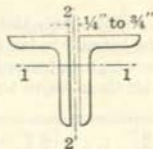
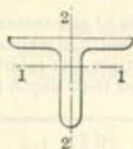
This and the following table may be used in computing the Moments of Inertia of Plate Girders, Columns and other compound sections in which plates are used.

Depth, Inches	Width, Inches							
	1/4	5/16	3/8	7/16	1/2	9/16	5/8	1
1	.021	.026	.031	.037	.042	.047	.052	.083
2	.167	.208	.250	.292	.333	.375	.417	.667
3	.563	.703	.844	.984	1.125	1.266	1.406	2.250
4	1.333	1.667	2.000	2.333	2.667	3.000	3.333	5.333
5	2.604	3.255	3.906	4.557	5.208	5.859	6.510	10.417
6	4.500	5.625	6.750	7.875	9.000	10.125	11.250	18.000
7	7.146	8.932	10.719	12.505	14.292	16.078	17.865	28.583
8	10.667	13.333	16.000	18.667	21.333	24.000	26.667	42.667
9	15.188	18.984	22.781	26.578	30.375	34.172	37.969	60.750
10	20.833	26.042	31.250	36.458	41.667	46.875	52.083	83.333
11	27.729	34.662	41.594	48.526	55.458	62.391	69.323	110.917
12	36.000	45.000	54.000	63.000	72.000	81.000	90.000	144.000
13	45.771	57.214	68.656	80.099	91.542	102.984	114.427	183.083
14	57.167	71.458	85.750	100.042	114.333	128.625	142.917	228.667
15	70.313	87.891	105.469	123.047	140.625	158.203	175.781	281.250
16	85.333	106.667	128.000	149.333	170.667	192.000	213.333	341.333
17	102.354	127.943	153.531	179.120	204.708	230.297	255.885	409.417
18	121.500	151.875	182.250	212.625	243.000	273.375	303.750	486.000
19	142.896	178.620	214.344	250.068	285.792	321.516	357.240	571.583
20	166.667	208.333	250.000	291.667	333.333	375.000	416.667	666.667
21	192.938	241.172	289.406	337.641	385.875	434.109	482.344	771.750
22	221.833	277.292	332.750	388.208	443.667	499.125	554.583	887.333
23	253.479	316.849	380.219	443.589	506.958	570.328	633.698	1013.917
24	288.000	360.000	432.000	504.000	576.000	648.000	720.000	1152.000
25	325.521	406.901	488.281	569.662	651.042	732.422	813.802	1302.083
26	366.167	457.708	549.250	640.792	732.333	823.875	915.417	1464.667
27	410.063	512.578	615.094	717.609	820.125	922.641	1025.156	1640.250
28	457.333	571.667	686.000	800.333	914.667	1029.000	1143.333	1829.333
29	508.104	635.130	762.156	889.182	1016.208	1143.234	1270.260	2032.417
30	562.500	703.125	843.750	984.375	1125.000	1265.625	1406.250	2250.000
32	682.667	853.333	1024.000	1194.667	1365.333	1536.000	1706.667	2730.667
34	818.833	1023.542	1228.250	1432.958	1637.667	1842.375	2047.083	3275.333
36	972.000	1215.000	1458.000	1701.000	1944.000	2187.000	2430.000	3888.000
38	1143.167	1428.958	1714.750	2000.542	2286.333	2572.125	2857.917	4572.667
40	1333.333	1666.667	2000.000	2333.333	2666.667	3000.000	3333.333	5333.333
42	1543.500	1929.375	2315.250	2701.125	3087.000	3472.875	3858.750	6174.000
44	1774.667	2218.333	2662.000	3105.667	3549.333	3993.000	4436.667	7098.667
46	2027.833	2534.792	3041.750	3548.708	4055.667	4562.625	5069.583	8111.333
48	2304.000	2880.000	3456.000	4032.000	4608.000	5184.000	5760.000	9216.000
50	2604.167	3255.208	3906.250	4557.292	5208.333	5859.375	6510.417	10416.667
52	2929.333	3661.667	4394.000	5126.333	5858.667	6591.000	7323.333	11717.333
54	3280.500	4100.625	4920.750	5740.875	6561.000	7381.125	8201.250	13122.000
56	3658.667	4573.333	5488.000	6402.667	7317.333	8232.000	9146.667	14634.667
58	4064.833	5081.042	6097.250	7113.458	8129.667	9145.875	10162.083	16259.333
60	4500.000	5625.000	6750.000	7875.000	9000.000	10125.000	11250.000	18000.000

CARNEGIE STEEL COMPANY

RADI OF GYRATION FOR TWO UNEQUAL ANGLES

Long Legs Vertical

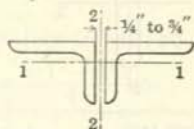
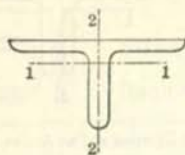


Single Angle			Area of Two Angles, Inches ²	Radii of Gyration of Two Angles, Inches					
Size, Inches	Thick-ness, Inches	Weight, Pounds per Foot		Axis 1-1	Axis 2-2				
					In Contact	$\frac{1}{4}$ " Apart	$\frac{3}{8}$ " Apart	$\frac{1}{2}$ " Apart	$\frac{3}{4}$ " Apart
8 x 6	1	44.2	26.00	2.49	2.39	2.48	2.52	2.57	2.66
	$\frac{3}{4}$	33.8	19.88	2.53	2.35	2.44	2.48	2.52	2.61
	$\frac{1}{2}$	20.2	11.86	2.57	2.31	2.39	2.43	2.48	2.56
8 x 3 $\frac{1}{2}$	1	35.7	21.00	2.51	1.26	1.35	1.40	1.45	1.55
	$\frac{3}{4}$	27.5	16.12	2.55	1.20	1.29	1.34	1.39	1.49
	$\frac{1}{2}$	16.5	9.68	2.59	1.15	1.23	1.28	1.32	1.41
7 x 3 $\frac{1}{2}$	1	32.3	19.00	2.19	1.31	1.40	1.45	1.50	1.60
	$\frac{11}{16}$	23.0	13.50	2.23	1.25	1.34	1.39	1.44	1.53
	$\frac{3}{8}$	13.0	7.60	2.27	1.20	1.28	1.33	1.37	1.46
6 x 4	1	30.6	18.00	1.85	1.60	1.69	1.74	1.79	1.89
	$\frac{11}{16}$	21.8	12.80	1.89	1.55	1.63	1.68	1.73	1.82
	$\frac{3}{8}$	12.3	7.22	1.93	1.50	1.58	1.62	1.67	1.76
6 x 3 $\frac{1}{2}$	1	28.9	17.00	1.85	1.37	1.47	1.51	1.56	1.66
	$\frac{11}{16}$	20.6	12.12	1.89	1.31	1.41	1.45	1.49	1.60
	$\frac{3}{8}$	9.8	5.74	1.95	1.25	1.33	1.37	1.42	1.50
5 x 4	$\frac{7}{8}$	24.2	14.22	1.52	1.66	1.76	1.80	1.85	1.95
	$\frac{3}{4}$	11.0	6.46	1.59	1.58	1.66	1.70	1.75	1.85
	$\frac{1}{2}$	22.7	13.34	1.53	1.42	1.51	1.56	1.61	1.71
5 x 3	$\frac{13}{16}$	19.9	11.68	1.55	1.18	1.27	1.32	1.37	1.47
	$\frac{3}{4}$	8.2	4.80	1.61	1.09	1.17	1.22	1.26	1.35
	$\frac{11}{16}$	18.5	10.86	1.38	1.21	1.31	1.36	1.41	1.51
4 $\frac{1}{2}$ x 3	$\frac{3}{4}$	7.7	4.50	1.44	1.13	1.22	1.26	1.30	1.40
	$\frac{13}{16}$	18.5	10.86	1.19	1.50	1.59	1.64	1.69	1.79
	$\frac{3}{4}$	7.7	4.50	1.26	1.42	1.51	1.55	1.60	1.69
4 x 3	$\frac{13}{16}$	17.1	10.06	1.21	1.25	1.35	1.40	1.45	1.55
	$\frac{3}{4}$	5.8	3.38	1.28	1.16	1.24	1.28	1.33	1.43
	$\frac{11}{16}$	15.8	9.24	1.04	1.30	1.40	1.45	1.50	1.60
3 $\frac{1}{2}$ x 3	$\frac{3}{4}$	5.4	3.12	1.11	1.20	1.29	1.34	1.38	1.48
	$\frac{11}{16}$	12.5	7.30	1.06	1.03	1.13	1.18	1.23	1.33
	$\frac{1}{2}$	4.9	2.88	1.12	0.95	1.04	1.09	1.13	1.23
3 x 2 $\frac{1}{2}$	$\frac{3}{4}$	9.5	5.56	0.91	1.05	1.15	1.20	1.25	1.35
	$\frac{1}{2}$	4.5	2.64	0.95	1.00	1.09	1.13	1.18	1.28
	$\frac{1}{4}$	7.7	4.50	0.92	0.80	0.89	0.94	1.00	1.10
3 x 2	$\frac{1}{4}$	4.1	2.38	0.95	0.74	0.84	0.88	0.93	1.03
	$\frac{1}{2}$	6.8	4.00	0.75	0.84	0.94	0.99	1.04	1.15
	$\frac{3}{4}$	3.62	2.12	0.78	0.80	0.89	0.93	0.98	1.08

ELEMENTS OF SECTIONS

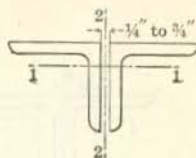
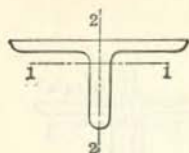
RADI OF GYRATION FOR TWO UNEQUAL ANGLES

Short Legs Vertical



Single Angle			Area of Two Angles, Inches ²	Radii of Gyration of Two Angles, Inches					
Size, Inches	Thickness, Inches	Weight, Pounds per Foot		Axis 1-1	Axis 2-2				
				In Contact	$\frac{1}{4}''$ Apart	$\frac{3}{8}''$ Apart	$\frac{1}{2}''$ Apart	$\frac{3}{4}''$ Apart	
8 x 6	1	44.2	26.00	1.73	3.64	3.73	3.78	3.83	3.92
	$\frac{3}{4}$	33.8	19.88	1.76	3.60	3.69	3.73	3.78	3.87
	$\frac{5}{16}$	20.2	11.86	1.80	3.55	3.64	3.68	3.73	3.82
8 x 3 $\frac{1}{2}$	1	35.7	21.00	0.86	4.04	4.14	4.19	4.24	4.34
	$\frac{3}{4}$	27.5	16.12	0.88	3.99	4.09	4.13	4.18	4.28
	$\frac{5}{16}$	16.5	9.68	0.92	3.93	4.02	4.07	4.12	4.22
7 x 3 $\frac{1}{2}$	1	32.3	19.00	0.89	3.48	3.58	3.63	3.68	3.78
	$\frac{11}{16}$	23.0	13.50	0.92	3.42	3.52	3.57	3.62	3.72
	$\frac{3}{8}$	13.0	7.60	0.96	3.36	3.46	3.50	3.55	3.65
6 x 4	1	30.6	18.00	1.09	2.85	2.95	2.99	3.04	3.14
	$\frac{11}{16}$	21.8	12.80	1.13	2.79	2.89	2.93	2.98	3.08
	$\frac{3}{8}$	12.3	7.22	1.17	2.74	2.83	2.87	2.92	3.02
6 x 3 $\frac{1}{2}$	1	28.9	17.00	0.92	2.92	3.02	3.07	3.12	3.22
	$\frac{11}{16}$	20.6	12.12	0.95	2.87	2.96	3.01	3.06	3.16
	$\frac{3}{8}$	9.8	5.74	1.00	2.81	2.90	2.95	3.00	3.09
5 x 4	$\frac{7}{8}$	24.2	14.22	1.14	2.29	2.38	2.43	2.48	2.58
	$\frac{3}{8}$	11.0	6.46	1.20	2.20	2.29	2.34	2.38	2.48
5 x 3 $\frac{1}{2}$	$\frac{7}{8}$	22.7	13.34	0.96	2.36	2.45	2.50	2.55	2.65
	$\frac{5}{16}$	8.7	5.12	1.03	2.26	2.35	2.39	2.44	2.54
5 x 3	$\frac{13}{16}$	19.9	11.68	0.80	2.42	2.52	2.57	2.62	2.72
	$\frac{5}{16}$	8.2	4.80	0.85	2.33	2.42	2.47	2.52	2.61
4 $\frac{1}{2}$ x 3	$\frac{13}{16}$	18.5	10.86	0.81	2.15	2.25	2.30	2.35	2.45
	$\frac{5}{16}$	7.7	4.50	0.87	2.06	2.15	2.20	2.25	2.34
4 x 3 $\frac{1}{2}$	$\frac{13}{16}$	18.5	10.86	1.01	1.81	1.91	1.96	2.01	2.11
	$\frac{5}{16}$	7.7	4.50	1.07	1.73	1.81	1.86	1.91	2.00
4 x 3	$\frac{13}{16}$	17.1	10.06	0.83	1.88	1.98	2.03	2.08	2.18
	$\frac{1}{4}$	5.8	3.38	0.89	1.78	1.87	1.92	1.96	2.06
3 $\frac{1}{2}$ x 3	$\frac{13}{16}$	15.8	9.24	0.85	1.61	1.71	1.76	1.81	1.91
	$\frac{1}{4}$	5.4	3.12	0.91	1.52	1.61	1.65	1.70	1.80
3 $\frac{1}{2}$ x 2 $\frac{1}{2}$	$\frac{13}{16}$	12.5	7.30	0.69	1.66	1.75	1.80	1.86	1.96
	$\frac{1}{4}$	4.9	2.88	0.74	1.58	1.67	1.71	1.76	1.86
3 x 2 $\frac{1}{2}$	$\frac{9}{16}$	9.5	5.56	0.72	1.37	1.46	1.51	1.56	1.66
	$\frac{1}{4}$	4.5	2.64	0.75	1.31	1.40	1.45	1.50	1.59
3 x 2	$\frac{1}{2}$	7.7	4.50	0.55	1.42	1.52	1.57	1.62	1.72
	$\frac{1}{4}$	4.1	2.38	0.57	1.38	1.47	1.52	1.57	1.67
2 $\frac{1}{2}$ x 2	$\frac{1}{2}$	6.8	4.00	0.56	1.15	1.25	1.30	1.35	1.46
	$\frac{1}{4}$	3.62	2.12	0.59	1.11	1.20	1.25	1.30	1.40

RADII OF GYRATION FOR TWO EQUAL ANGLES



Single Angle			Area of Two Angles, Inches ²	Radii of Gyration of Two Angles, Inches					
Size, Inches	Thick-ness, Inches	Weight, Pounds per Foot		Axis 1-1	Axis 2-2				
					In Contact	$\frac{1}{4}$ " Apart	$\frac{3}{8}$ " Apart	$\frac{1}{2}$ " Apart	$\frac{3}{4}$ " Apart
8 x 8	$1\frac{1}{8}$	56.9	33.46	2.42	3.42	3.51	3.55	3.60	3.69
	$1\frac{5}{16}$	42.0	24.68	2.46	3.37	3.46	3.50	3.55	3.64
	$\frac{1}{2}$	26.4	15.50	2.50	3.33	3.41	3.45	3.50	3.59
6 x 6	1	37.4	22.00	1.80	2.59	2.68	2.72	2.77	2.87
	$1\frac{1}{8}$	26.5	15.56	1.83	2.54	2.63	2.67	2.71	2.81
	$\frac{5}{8}$	14.9	8.72	1.88	2.49	2.58	2.62	2.66	2.75
5 x 5	1	30.6	18.00	1.48	2.19	2.28	2.33	2.38	2.47
	$1\frac{1}{8}$	21.8	12.80	1.51	2.13	2.22	2.26	2.31	2.40
	$\frac{5}{8}$	12.3	7.22	1.56	2.09	2.17	2.21	2.26	2.35
4 x 4	$1\frac{3}{16}$	19.9	11.68	1.18	1.75	1.85	1.89	1.94	2.04
	$\frac{1}{4}$	6.6	3.88	1.25	1.66	1.75	1.79	1.84	1.93
$3\frac{1}{2}$ x $3\frac{1}{2}$	$1\frac{3}{16}$	17.1	10.06	1.02	1.55	1.65	1.70	1.75	1.85
	$\frac{1}{4}$	5.8	3.38	1.09	1.46	1.55	1.59	1.64	1.73
3 x 3	$\frac{5}{8}$	11.5	6.72	0.88	1.32	1.41	1.46	1.51	1.61
	$\frac{1}{4}$	4.9	2.88	0.93	1.25	1.34	1.38	1.43	1.53
$2\frac{1}{2}$ x $2\frac{1}{2}$	$\frac{1}{2}$	7.7	4.50	0.74	1.09	1.19	1.24	1.29	1.39
	$\frac{3}{4}$	4.1	2.38	0.77	1.05	1.14	1.19	1.24	1.34
2 x 2	$\frac{7}{16}$	5.3	3.12	0.59	0.88	0.98	1.03	1.08	1.19
	$\frac{1}{4}$	3.19	1.88	0.61	0.85	0.94	0.99	1.04	1.14

This table and the two preceding are employed in computing the safe resistance to compressive stress of two angles, back to back, used as a strut or as the compression chord of a roof truss, etc., as follows:

Obtain from the compression formula in use the allowed stress per square inch corresponding to the ratio of slenderness of the section, and multiply that value by the area. The result will be the allowable compressive stress.

Example 1. Section given. Required the safe load in compression on a strut composed of two angles $4'' \times 4'' \times \frac{1}{4}''$, back to back, with an unsupported length of 9 feet.

Area of Section, $A=3.88$ square inches; Least Radius, $r=1.25$.

Ratio of Slenderness, $l/r = 9 \times 12 \div 1.25=86.4$.

Safe Load, $Af=3.88 \times$ allowed unit stress for l/r according to formula.

Example 2. Stress given. Required a section for a member in compression $12'-3''$ long, made of two angles separated by $\frac{1}{2}$ inch gusset plates, to resist a total stress of 48,000 pounds; ratio of slenderness not to exceed 120.

Assume 2 angles, $5'' \times 3'' \times \frac{5}{16}''$, $5''$ -legs, back to back.

Area of Section, $A=4.80$ square inches; Least Radius, $r=1.26$ inches.

Ratio of Slenderness, $l/r = 12.25 \times 12 \div 1.26=116.7$.

Allowed Unit Stress f , by A. I. S. C. formula = 10,250 lb./sq. in.

Safe Stress, $Af=4.80 \times 10,250=49,200$ pounds.

In the first case the least radius is that about axis 1-1; in the second case about axis 2-2; in all cases the least radius determines the ratio of slenderness and therewith the allowed safe compressive stress. In all cases also the two angles are to be secured together by stay rivets so spaced as to insure that the section acts as a unit. The ratio of slenderness of any single angle between rivets must always be less than that of the strut or compression chord.

ELEMENTS OF SECTIONS

HOLLOW ROUND SECTIONS

AREAS AND RADII OF GYRATION



$$\text{Area} = \frac{\pi(D^2-d^2)}{4} = 0.7854 (D^2-d^2) \text{ sq. in.}$$

$$\text{Radius of gyration} = \frac{\sqrt{D^2+d^2}}{4} \text{ in.}$$

Dia. D, Inches	Elements	Thickness in Inches																	
		¼	⅜	½	⅝	¾	⅞	1	1⅛	1¼	1⅝	1½	1⅞	2	2¼	2½	2⅞	3	
2	A	1.37	1.66																
	r	0.63	0.61																
3	A	2.16	2.64																
	r	0.98	0.96																
4	A	2.95	3.62	4.27	5.50														
	r	1.33	1.31	1.29	1.25														
5	A	3.73	4.60	5.45	7.07	8.59	10.01												
	r	1.68	1.66	1.64	1.60	1.56	1.53												
6	A	4.52	5.58	6.63	8.64	10.55	12.37	14.09	15.71										
	r	2.03	2.01	1.99	1.95	1.91	1.88	1.84	1.80										
7	A	5.30	6.57	7.80	10.21	12.52	14.73	16.84	18.85	20.76	22.58								
	r	2.39	2.37	2.35	2.30	2.27	2.23	2.19	2.15	2.12	2.08								
8	A	6.09	7.55	8.98	11.78	14.48	17.08	19.59	21.99	24.30	26.51	28.62	30.63						
	r	2.74	2.72	2.70	2.66	2.62	2.58	2.54	2.50	2.46	2.43	2.39	2.36						
9	A	6.87	8.53	10.16	13.35	16.44	19.44	22.33	25.13	27.83	30.43	32.94	35.34	37.65	39.86				
	r	3.09	3.07	3.05	3.01	2.97	2.93	2.89	2.85	2.81	2.78	2.74	2.70	2.67	2.64				
10	A	7.66	9.51	11.34	14.92	18.41	21.79	25.08	28.27	31.37	34.36	37.26	40.06	42.76	45.36	47.86	50.27		
	r	3.45	3.43	3.41	3.36	3.32	3.28	3.24	3.20	3.16	3.13	3.09	3.05	3.02	2.98	2.95	2.92		
11	A	8.44	10.49	12.52	16.49	20.37	24.15	27.83	31.42	34.90	38.29	41.58	44.77	47.86	50.85	53.75	56.55		
	r	3.80	3.78	3.76	3.72	3.67	3.63	3.59	3.55	3.51	3.48	3.44	3.40	3.36	3.33	3.29	3.26		
12	A	9.23	11.47	13.70	18.06	22.33	26.51	30.58	34.56	38.44	42.22	45.90	49.48	52.97	56.35	59.64	62.83		
	r	4.16	4.13	4.11	4.07	4.03	3.99	3.95	3.91	3.87	3.83	3.79	3.75	3.71	3.68	3.64	3.61		
13	A	10.01	12.46	14.87	19.63	24.30	28.86	33.33	37.70	41.97	46.14	50.22	54.19	58.07	61.85	65.53	69.12		
	r	4.51	4.49	4.47	4.42	4.38	4.34	4.30	4.26	4.22	4.18	4.14	4.10	4.06	4.03	3.99	3.95		
14	A	10.80	13.44	16.05	21.21	26.26	31.22	36.08	40.84	45.50	50.07	54.54	58.91	63.18	67.35	71.42	75.40		
	r	4.86	4.84	4.82	4.78	4.73	4.69	4.65	4.61	4.57	4.53	4.49	4.45	4.41	4.38	4.34	4.30		
15	A	11.58	14.42	17.23	22.78	28.23	33.58	38.83	43.98	49.04	54.00	58.86	63.62	68.28	72.85	77.31	81.68		
	r	5.22	5.19	5.17	5.13	5.09	5.05	5.00	4.96	4.92	4.88	4.84	4.80	4.76	4.73	4.69	4.65		
16	A	12.37	15.40	18.41	24.35	30.19	35.93	41.58	47.12	52.57	57.92	63.18	68.33	73.39	78.34	83.20	87.97		
	r	5.57	5.55	5.53	5.48	5.44	5.40	5.36	5.32	5.27	5.23	5.19	5.15	5.11	5.08	5.04	5.00		
17	A	13.16	16.38	19.59	25.92	32.15	38.29	44.33	50.27	56.11	61.85	67.50	73.04	78.49	83.84	89.09	94.25		
	r	5.92	5.90	5.88	5.84	5.79	5.75	5.71	5.67	5.63	5.59	5.55	5.51	5.47	5.43	5.39	5.35		
18	A	13.94	17.36	20.76	27.49	34.12	40.64	47.07	53.41	59.64	65.78	71.82	77.75	83.60	89.34	94.98	100.53		
	r	6.28	6.25	6.23	6.19	6.15	6.10	6.06	6.02	5.98	5.94	5.90	5.86	5.82	5.78	5.74	5.70		
19	A	14.73	18.35	21.94	29.06	36.08	43.00	49.82	56.55	63.18	69.70	76.13	82.47	88.70	94.84	100.87	106.82		
	r	6.63	6.61	6.59	6.54	6.50	6.46	6.42	6.37	6.33	6.29	6.25	6.21	6.17	6.13	6.09	6.05		
20	A	15.51	19.33	23.12	30.63	38.04	45.36	52.57	59.69	66.71	73.63	80.45	87.18	93.81	100.33	106.77	113.10		
	r	6.98	6.96	6.94	6.90	6.85	6.81	6.77	6.73	6.69	6.64	6.60	6.56	6.52	6.48	6.44	6.40		

STRESSES IN BEAMS

In the application of the principles of structural mechanics to determine what sections should be used safely to sustain superimposed loads under specified conditions of loading, it is necessary to ascertain, first, the effects produced on the structure by the loads under those conditions; second, to decide what unit strength the material, the use of which is contemplated, has to resist the stresses produced within the structure by the loading; and, third, to select a section whose section modulus is equivalent to the ratio found to exist between the stresses tending to cause deformation within the structure and the unit strength of the material to resist them.

Reactions. In the simple case of a beam supported at both ends, each support reacts with an upward pressure called the reaction of the support. The sum of these two reactions is equal to the total load on the beam.

Shear. The loads and the reactions of the supports are vertical forces tending to shear or cut the beam across and the stresses they produce within the beam are, therefore, called shearing stresses. The shear at each support is equal to the reaction of the support; the shear at any point between the supports is equal to the reaction of a support less the total load between that support and the point; or, if the reaction acting upward is considered as positive and the loads, acting downwards, as negative, the shear at any point is the algebraic sum of the vertical forces acting on the beam between that point and either support.

If such a simple beam supported at both ends carries a load uniformly distributed over its entire length, the reaction and the shear at each support is equal to one-half the total load on the beam, but the shear decreases uniformly to zero at the center of the span; if the load is concentrated at the center of the span, the reaction and the shear at each support are also equal to one-half the total load, but the shear is uniform throughout the entire length of the beam.

Bending Moment. The loads on the beam and the reactions of the supports constitute external forces which produce bending stress in the beam. The summation of the moments of the external forces about any point is called the bending moment and varies from point to point. It attains a maximum value at a point where the shear is either zero or changes from positive to negative or vice versa. If the loads are concentrated at several points, the maximum bending moment always occurs at the point of application of

one of the loads so located that the sum of all the loads on the beam between one support up to and including that load is equal to or greater than the reaction of the support.

Vertical Deflection. Bending stress within a beam produces flexure, and the deflection, or the amount of its departure from a straight line, is the measure of the deformation which the beam has undergone in its resistance to bending stress. So long as the stress is within the safe limits allowed for the material, the deflection is negligible so far as concerns the beam itself; it may, however, be of sufficient magnitude to cause the disruption of other materials in contact with or supported by the beam but of less strength, such as plaster. In such cases the limit of allowable deflection may determine or at least influence the choice of a section.

Lateral Deflection. The stresses within a beam under transverse loading are compressive on one side of the neutral axis and tensile on the other. The tensile stresses tend to hold the beam in a straight line between the supports, while the compressive stresses tend to deflect it in a lateral direction, just as the bending stresses as a whole tend to deflect it in a vertical plane. On long spans unsupported against sidewise deflection, this consideration may influence the choice of sections.

Method of Computation. A complete investigation of the strength of beams under transverse loading must take into account all the elements, the bending moment, the vertical deflection, the lateral deflection and the shearing stress; though under the usual loading conditions the first alone determines the size and weight of section.

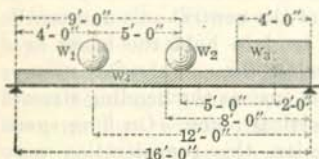
In the calculation of bending stresses, the loads are usually expressed in pounds, the span length and the distance between the loads in feet; the resulting bending moments are in terms of foot pounds, which necessitates conversion to inch pounds before the section can be selected from the tables. The section modulus of the required section is obtained by dividing the maximum bending moment in inch pounds by the allowed fiber stress in pounds per square inch. In such calculations it is assumed that the neutral axis of the section is normal to the line of action of the load. When this is not the case, correction must be made for the eccentricity of the loading.

In the pages which immediately follow are given general formulas for the bending moments and vertical deflections of beams under the usual conditions of loading, and also diagrams illustrative of those conditions. The general method for the computation of the maximum bending moment of a beam supported at its ends and loaded at various points is as follows:—

First. Find the reaction at the left (right) support by multiplying each load by its distance from the right (left) support and dividing the sum of these products by the length of the span.

Second. Starting from the left (right) end of the beam, add the successive loads until a point is reached where the sum of the loads equals or exceeds the reaction of the left (right) support; the point of maximum bending moment is located at this point.

Third. Multiply the reaction at the left (right) support by its distance from the point of maximum bending moment and subtract the sum of the products of all loads to the left (right) of this point by the corresponding distance from this point; the difference between these moments is then the maximum bending moment.



Example: Required the size of a steel beam to support the following quiescent loads over a clear span of 16 feet between supports, at a maximum fiber stress not to exceed 16000 pounds per square inch.

- $W_1 = 18000$ pounds, 4 feet from left support.
- $W_2 = 18000$ " 9 " " " "
- $W_3 = 2000$ " per foot, uniform up to 4 feet from right support.
- $W_4 = 60$ " " " assumed weight of beam uniformly distributed over entire span.

$$\text{Left Reaction, } \frac{18000 \times 12 + (60 \times 16)8 + 18000 \times 7 + (2000 \times 4) \times 2}{16} = 21355 \text{ lbs.}$$

$$\text{Right Reaction, } \frac{18000 \times 4 + (60 \times 16)8 + 18000 \times 9 + (2000 \times 4) \times 14}{16} = 21605 \text{ lbs.}$$

$$\text{Sum of reactions} = \text{sum of loads} = W_1 + W_2 + W_3 + W_4 = 42960 \text{ lbs.}$$

$$\text{Points of maximum moment } (60 \times 4) + 18000 = 16240 < 21355$$

$$(60 \times 9) + 18000 + 18000 = 34540 > 21355$$

therefore the point of maximum bending moment is at point of load W_2 .

$$\text{Maximum bending moment, } 21355 \times 9 - 18000 \times 5 - (60 \times 9) \times 4.5 = 109765 \text{ ft. lbs.}$$

$$\text{or, } 21605 \times 7 - (2000 \times 4) \times 5 - (60 \times 7) \times 3.5 = 109765 \text{ ft. lbs.}$$

$$\text{Required section modulus} = \frac{109765 \times 12}{16000} = \frac{1317180}{16000} = 82.4 \text{ in.}^3$$

As the section modulus of the 15 inch 65 pound or the 18 inch 54.7 pound beam is greater than this, either of these sections may be used.

If the allowed fiber stress were 18000 pounds per square inch, the required section modulus would be $\frac{109765 \times 12}{18000} = \frac{1317180}{18000} = 73.2 \text{ in.}^3$

COMPARISON OF VARIOUS LOADING CONDITIONS

The formulas and diagrams on the pages which follow give the various stresses in sections used as beams, resulting from usual conditions of loading.

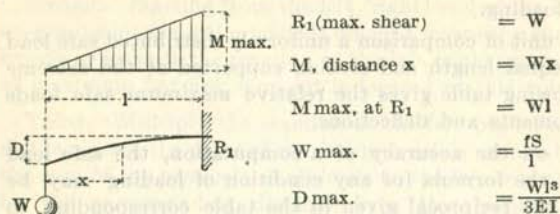
Taking as a unit of comparison a uniformly distributed safe load on beams of equal length and section, supported at the extreme ends, the following table gives the relative maximum safe loads or bending moments and deflections.

As a check on the accuracy of a computation, the safe load obtained from the formula for any condition of loading may be multiplied by the reciprocal given in the table corresponding to such loading condition; the result should be the maximum allowable uniform load as taken from beam safe load tables.

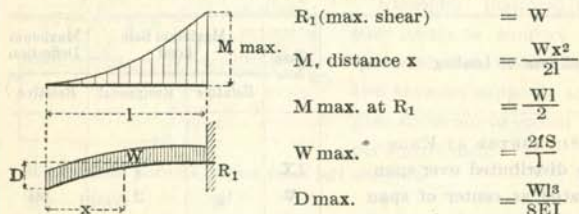
Conditions of Loading	Case No.	Maximum Safe Load		Maximum Deflection
		Relative	Reciprocal	Relative
BEAM SUPPORTED AT ENDS *				
Load uniformly distributed over span	IX	1	1	1
Load concentrated at center of span	V	$\frac{1}{2}$	2	.80
Two equal loads symmetrically concentrated	VII	$\frac{1}{4a}$	$\frac{4a}{1}$	
Load increasing uniformly to one end	X	.9743	1.0264	.976
Load increasing uniformly to center	XII	$\frac{3}{4}$	$1\frac{1}{3}$.96
Load decreasing uniformly to center	XI	$\frac{3}{2}$	$\frac{2}{3}$	1.08
BEAM FIXED AT ONE END, CANTILEVER				
Load uniformly distributed over span	II	$\frac{1}{4}$	4	2.40
Load concentrated at end	I	$\frac{1}{8}$	8	3.20
Load increasing uniformly to fixed end	III	$\frac{3}{8}$	$2\frac{2}{3}$	1.92
BEAM CONTINUOUS OVER TWO SUPPORTS EQUIDISTANT FROM ENDS				
Load uniformly distributed over span	XVI			
1. If distance $a > 0.2071 l$		$\frac{l^2}{4a^2}$	$\frac{4a^2}{l^2}$	
2. If distance $a < 0.2071 l$		$\frac{1}{1-4a}$	$\frac{1-4a}{1}$	
3. If distance $a = 0.2071 l$		5.8285	.1716	
Two equal loads concentrated at ends	XV	$\frac{1}{4a}$	$\frac{4a}{1}$	

BEAMS UNDER VARIOUS LOADING CONDITIONS
BENDING MOMENTS AND DEFLECTIONS

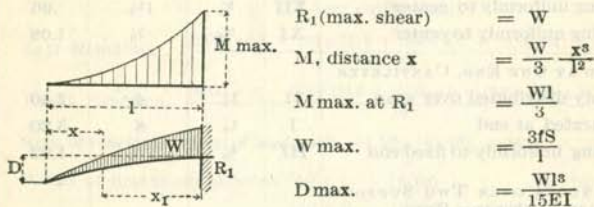
I. CANTILEVER BEAM—Concentrated load at free end



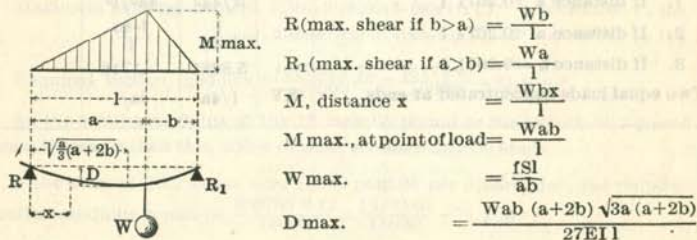
II. CANTILEVER BEAM—Uniformly distributed load



III. CANTILEVER BEAM—Load increasing uniformly to fixed end

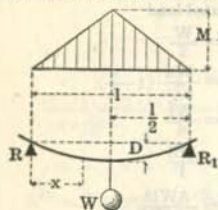


IV. BEAM SUPPORTED AT ENDS—Concentrated load near one end



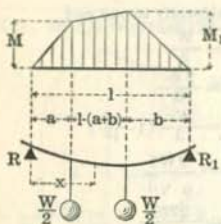
BEAMS UNDER VARIOUS LOADING CONDITIONS
BENDING MOMENTS AND DEFLECTIONS

V. BEAM SUPPORTED AT ENDS—Concentrated load at center



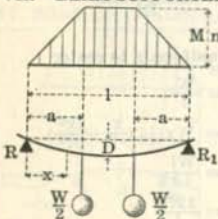
$$\begin{aligned}
 M \text{ max. } R \text{ (max. shear)} &= R_1 = \frac{W}{2} \\
 M, \text{ distance } x &= \frac{Wx}{2} \\
 M \text{ max., at point of load} &= \frac{Wl}{4} \\
 W \text{ max.} &= \frac{4fS}{l} \\
 D \text{ max.} &= \frac{Wl^3}{48EI}
 \end{aligned}$$

VI. BEAM SUPPORTED AT ENDS—Two unsymmetrical concentrated loads



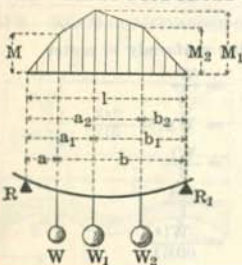
$$\begin{aligned}
 R \text{ (max. shear if } a < b) &= \frac{W}{2l}(l-a+b) \\
 R_1 &= \frac{W}{2l}(l+a-b) \\
 M, \text{ distance } a &= Ra = \frac{Wa}{2l}(l-a+b) \\
 M_1 \text{ max., distance } b \text{ (} b > a) &= R_1b = \frac{Wb}{2l}(l+a-b) \\
 M_2, \text{ distance } x &= Rx - \frac{W}{2}(x-a) \\
 W \text{ max. (} b > a) &= \frac{2fS}{b(l+a-b)}
 \end{aligned}$$

VII. BEAM SUPPORTED AT ENDS—Two symmetrical concentrated loads



$$\begin{aligned}
 M \text{ max. } R \text{ (max. shear)} &= R_1 = \frac{W}{2} \\
 M, \text{ distance } x &= \frac{Wx}{2} \\
 M \text{ max. at and between loads} &= \frac{Wa}{2} \\
 W \text{ max.} &= \frac{2fS}{a} \\
 D \text{ max.} &= \frac{Wa}{12EI}(4l^2-a^2)
 \end{aligned}$$

VIII. BEAM SUPPORTED AT ENDS—Three concentrated loads

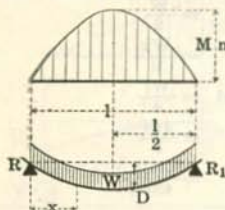


$$\begin{aligned}
 R &= \frac{Wb+W_1b_1+W_2b_2}{l} \\
 R_1 &= \frac{Wa+W_1a_1+W_2a_2}{l} \\
 M \text{ at } W &= Ra \\
 M \text{ max. if } W &= \text{or } > R \\
 M \text{ at } W_1 &= Ra_1 - W(a_1 - a) \\
 M \text{ max. if } W_1 + W &= R \text{ or } > R \\
 M \text{ max. if } W_1 + W_2 &= R_1 \text{ or } > R_1 \\
 M \text{ at } W_2 &= Ra_2 - W(a_2 - a) - W_1(a_2 - a_1) \\
 M \text{ max. if } W_2 &= R_1 \text{ or } > R_1
 \end{aligned}$$

BEAMS UNDER VARIOUS LOADING CONDITIONS

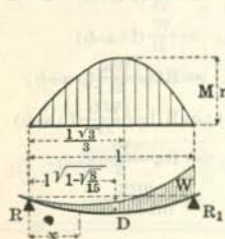
BENDING MOMENTS AND DEFLECTIONS

IX. BEAM SUPPORTED AT ENDS—Uniformly distributed load



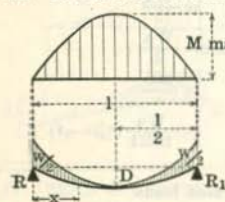
$$\begin{aligned}
 R(\text{max. shear}) &= R_1 = \frac{W}{2} \\
 M, \text{ distance } x &= \frac{Wx}{2} \left(1 - \frac{x}{l}\right) \\
 M \text{ max. at center} &= \frac{Wl}{8} \\
 W \text{ max.} &= \frac{8fs}{l} \\
 D \text{ max.} &= \frac{5Wl^3}{384EI}
 \end{aligned}$$

X BEAM SUPPORTED AT ENDS—Load increasing uniformly to one end



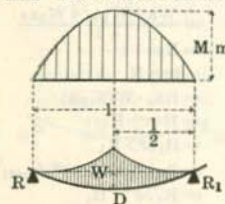
$$\begin{aligned}
 R &= \frac{W}{3} \\
 R_1(\text{max. shear}) &= \frac{2W}{3} \\
 M, \text{ distance } x &= \frac{Wx}{3} \left(1 - \frac{x^2}{l^2}\right) \\
 M \text{ max., distance } \frac{l\sqrt{3}}{3} &= \frac{2Wl}{9\sqrt{3}} \\
 W \text{ max.} &= \frac{27fs}{2l\sqrt{3}} \\
 D \text{ max.} &= \frac{.013044 Wl^3}{EI}
 \end{aligned}$$

XI. BEAM SUPPORTED AT ENDS—Load decreasing uniformly to center



$$\begin{aligned}
 R(\text{max. shear}) &= R_1 = \frac{W}{2} \\
 M, \text{ distance } x &= Wx \left(\frac{1}{2} \cdot \frac{x}{l} + \frac{2x^2}{3l^2}\right) \\
 M \text{ max., distance } \frac{l}{2} &= \frac{Wl}{12} \\
 W \text{ max.} &= \frac{12fs}{l} \\
 D \text{ max.} &= \frac{3Wl^3}{320EI}
 \end{aligned}$$

XII. BEAM SUPPORTED AT ENDS—Load increasing uniformly to center



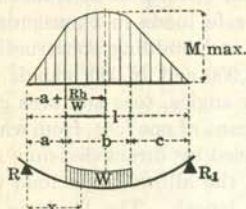
$$\begin{aligned}
 R(\text{max. shear}) &= R_1 = \frac{W}{2} \\
 M, \text{ distance } x &= Wx \left(\frac{1}{2} - \frac{2x^2}{3l^2}\right) \\
 M \text{ max., distance } \frac{l}{2} &= \frac{Wl}{6} \\
 W \text{ max.} &= \frac{6fs}{l} \\
 D \text{ max.} &= \frac{Wl^3}{60EI}
 \end{aligned}$$

FLEXURE FORMULAS

BEAMS UNDER VARIOUS LOADING CONDITIONS

BENDING MOMENTS AND DEFLECTIONS—Concluded

XIII. BEAM SUPPORTED AT ENDS—Uniform load partially distributed



$$R \text{ (max. shear if } a < c) = \frac{W(2c+b)}{2l}$$

$$R_1 = \frac{W(2a+b)}{2l}$$

$$M, \text{ dist. } x = a \text{ or } < a, = Rx$$

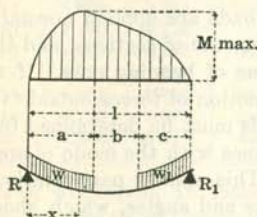
$$M_1 \text{ dist. } x > a, = Rx - \frac{W(x-a)^2}{2b}$$

$$M_2, \text{ dist. } x > (a+b), = Rx - \frac{W(2x-2a-b)}{2}$$

$$M \text{ max., dist. } a + \frac{Rb}{W} = \frac{W(2c+b)[4al+b(2c+b)]}{8l^2}$$

$$W \text{ max.} = \frac{8l^2 f_s}{(2c+b)[4al+b(2c+b)]}$$

XIV. BEAM SUPPORTED AT ENDS—Uniform load partially discontinuous



$$R \text{ (max. shear if } W > W_1) = \frac{W(2l-a) + W_1c}{2l}$$

$$R_1 = \frac{W_1(2l-c) + Wa}{2l}$$

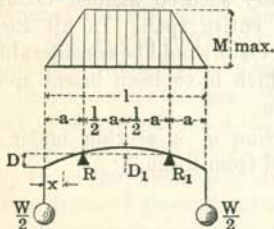
$$M, \text{ distance } x < a, = Rx - \frac{Wx^2}{2a}$$

$$M_1 \text{ distance } x > a, = Rx - \frac{W(2x-a)}{2}$$

$$M \text{ max. dist. } x = \frac{2Wal - Wa^2 + W_1ca}{2Wl} \quad \& \quad Wa > W_1c$$

$$W \text{ max.} = \frac{R^2 a}{2f_s}$$

XV. BEAM CONTINUOUS OVER TWO SUPPORTS—Two exterior symmetrical loads



$$R \text{ (max. shear)} = R_1 = \frac{W}{2}$$

$$M, \text{ distance } x = \frac{Wx}{2}$$

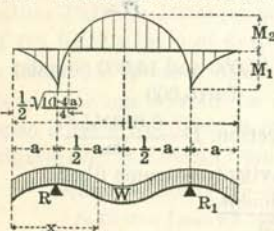
$$M \text{ max., from } R \text{ to } R_1 = \frac{Wa}{2}$$

$$W \text{ max.} = \frac{2f_s}{a}$$

$$D, \text{ distance } a = \frac{Wa(3al-4a^2)}{12EI}$$

$$D_1, \text{ distance } \frac{l}{2} - a = \frac{Wa(l-2a)^2}{16EI}$$

XVI. BEAM CONTINUOUS OVER TWO SUPPORTS—Uniformly distributed load



$$R = R_1 = \frac{W}{2}, \text{ max. shear } \frac{Wa}{l} \text{ or } \frac{W}{l} \left(\frac{l}{2} - a \right)$$

$$M, \text{ distance } x = \frac{W(x^2 - lx + al)}{2l}, \text{ o, if } x = \frac{l}{2} + \sqrt{\frac{l(l-4a)}{4}}$$

$$M_1 \text{ at } R \text{ and } R_1 = \frac{Wa^2}{2l} \quad \text{max. if } a > l(\sqrt{\frac{1}{2}} - \frac{1}{2})$$

$$M_2 \text{ at center} = \frac{W(l-4a)}{8} \quad \text{max. if } a < l(\sqrt{\frac{1}{2}} - \frac{1}{2})$$

$$W_1 \text{ max.} = \frac{2l f_s}{a^2} \quad \text{max. if } a > l(\sqrt{\frac{1}{2}} - \frac{1}{2})$$

$$W_2 \text{ max.} = \frac{8f_s}{l-4a} \quad \text{max. if } a < l(\sqrt{\frac{1}{2}} - \frac{1}{2})$$

SAFE LOADS FOR SECTIONS USED AS BEAMS

EXPLANATION OF TABLES

The tables of safe loads for structural beams, channels, H-beams and cross tie sections, used as beams under conditions of transverse loading, give the uniformly distributed safe loads in thousands of pounds for spans customary in bridge and building construction, based upon an extreme fiber stress of 18,000 and 16,000 pounds per square inch. The tables of safe loads for angles, tees and zees give the values at the same fiber stresses on spans of one foot, from which the safe load for any length may be obtained by direct division, and also the values for those spans at which the allowed safe load will produce a deflection of $\frac{1}{800}$ of the span length. The loads in all cases include the weight of the section, which should be deducted in order to arrive at the net load which the section will support.

It is assumed in all cases that the loads are applied normal to the axis 1-1 as shown in the tables of elements of sections, and that the beam deflects vertically in the plane of bending only. If the conditions of loading involve the introduction of forces outside this plane of loading, the allowable safe loads must be determined from the general theory of flexure, in accordance with the mode of application of the load and its character. This applies particularly to unsymmetrical sections, such as zee bars and angles, which should be used only under those conditions of loading where the section can deflect vertically only, being rigidly secured against lateral deflection or twisting throughout the entire span. In all such cases of eccentric loading, the actual safe loads would be considerably lower than the tabulated safe loads which have been based upon the most favorable conditions of loading.

Vertical Deflection. The vertical deflection of a section under a uniformly distributed load is determined from formula:

$$\text{Deflection, } D = \frac{5}{384} \frac{Wl^3}{EI} ; Wl = 8 f \frac{I}{n}$$

$$\text{" } D = \frac{40}{384} \frac{fl^2}{En} ; \text{ for span length in feet, } \frac{1}{2} l = L = L_1 + L_2$$

$$\text{" } D = \frac{15 fl^2}{En} \text{ inches}$$

Steel, $E=29,000,000$; for fiber stresses of 18,000 and 16,000 pounds:

$$f=18,000$$

$$f=16,000$$

$$\text{Deflection, } D = \frac{0.01862L^2}{2n}$$

$$\text{Deflection, } D = \frac{0.01655L^2}{2n}$$

n =distance from center line of gravity to extreme fiber.

$$\text{Deflection} = \frac{\text{Coefficient}}{2n}$$

BEAM SAFE LOADS

Deflection Coefficients for Fiber Stresses of 18,000 and 16,000 Pounds

Span, Feet	Fiber Stress		Span, Feet	Fiber Stress		Span, Feet	Fiber Stress	
	18,000	16,000		18,000	16,000		18,000	16,000
1	0.019	0.017	21	8.212	7.299	41	31.301	27.823
2	0.074	0.066	22	9.012	8.011	42	32.847	29.197
3	0.168	0.149	23	9.850	8.756	43	34.430	30.604
4	0.298	0.265	24	10.726	9.534	44	36.050	32.044
5	0.466	0.414	25	11.638	10.345	45	37.707	33.517
6	0.670	0.596	26	12.588	11.189	46	39.401	35.023
7	0.912	0.811	27	13.574	12.066	47	41.133	36.563
8	1.192	1.059	28	14.599	12.977	48	42.902	38.135
9	1.508	1.341	29	15.660	13.920	49	44.708	39.741
10	1.862	1.655	30	16.759	14.897	50	46.552	41.379
11	2.253	2.003	31	17.894	15.906	51	48.432	43.051
12	2.681	2.383	32	19.068	16.949	52	50.350	44.756
13	3.147	2.797	33	20.278	18.025	53	52.306	46.494
14	3.650	3.244	34	21.526	19.134	54	54.298	48.265
15	4.190	3.724	35	22.810	20.276	55	56.328	50.069
16	4.767	4.237	36	24.132	21.451	56	58.395	51.906
17	5.381	4.783	37	25.492	22.659	57	60.499	53.777
18	6.033	5.363	38	26.888	23.901	58	62.640	55.680
19	6.722	5.975	39	28.322	25.175	59	64.819	57.617
20	7.448	6.621	40	29.793	26.483	60	67.035	59.586

The deflection, in inches, of sections subjected to transverse stresses due to uniformly distributed loads are obtained as follows:

Symmetrical Sections. To find the deflection in inches of a section symmetrical about the neutral axis, such as beams, channels, zees, etc., divide the coefficient in the table corresponding to given span and fiber stress by the depth of the section in inches.

Unsymmetrical Sections. To find the deflection in inches of a section not symmetrical about the neutral axis, such as angles, tees, etc., divide the coefficient corresponding to given span and fiber stress by twice the distance of extreme fiber from neutral axis obtained from table of elements of sections.

Other Fiber Stresses. To find the deflection of any section for other fiber stresses than those given, multiply the coefficient for either 18000 or 16000 pounds fiber stress corresponding to the span given by the ratio of desired fiber stress and 18000 or 16000.

Limits of Deflection. The deflection of floor beams carrying plastered ceilings should be limited to not more than $\frac{1}{360}$ of the span length; this limit is indicated in the safe load tables by lower zigzag line, is derived from the following formulas:

$$\text{Deflection, } D_{\max} = \frac{12L}{360} = \frac{15fL^2}{En} \quad \text{Limiting Span, } L_{\max} = \frac{En}{450f}$$

$f = 18,000, L_{\max} = 3.580n$
 $f = 16,000, L_{\max} = 4.027n$

CARNEGIE STEEL COMPANY

Lateral Deflection of Beams. The tabular safe loads are based on the assumption that the compression flanges of the various sections are secured against lateral deflection by the use of tie rods or by other means at proper intervals.

Full tabular safe loads may be used up to a span length equal to fifteen times the flange width, but when the unbraced length exceeds fifteen times the width, the tabular safe loads must be reduced in accordance with the ratios given in the following table in order to insure that the stresses in the compression flanges do not exceed the safe unit stress. The lateral unbraced length of beams and girders should not exceed forty times the width of the compression flanges.

Reduction of Safe Loads for Ratio of Span Length to Flange Width, l/b .

Ratio, l/b	Tabular Load, Per Cent.	Ratio, l/b	Tabular Load, Per Cent.	Ratio, l/b	Tabular Load, Per Cent.
15	99.9	23.5	87.1	32	73.5
15.5	99.2	24	86.3	32.5	72.7
16	98.5	24.5	85.5	33	71.9
16.5	97.8	25	84.7	33.5	71.2
17	97.1	25.5	83.9	34	70.4
17.5	96.4	26	83.0	34.5	69.7
18	95.6	26.5	82.2	35	68.9
18.5	94.9	27	81.4	35.5	68.2
19	94.1	27.5	80.6	36	67.4
19.5	93.4	28	79.8	36.5	66.7
20	92.6	28.5	79.0	37	66.0
20.5	91.8	29	78.2	37.5	65.2
21	91.0	29.5	77.4	38	64.5
21.5	90.3	30	76.6	38.5	63.8
22	89.5	30.5	75.8	39	63.1
22.5	88.7	31	75.1	39.5	62.4
23	87.9	31.5	74.3	40	61.7

In addition to this lateral deflection which is induced within the beam by the action of pure bending stresses, lateral deflection may be induced by the thrust of floor arches or other loading acting on an axis perpendicular to the line of principal bending stress. The thrust of these arches should either be neutralized by tie rods, or the safe carrying capacity of the beam should be computed in accordance with the general formulas of flexure to provide for the combined stresses due to the action of both vertical and horizontal forces; that is to say, the safe loads should be figured around both the axes 1-1 and 2-2, and the unit stress computed so as not to exceed the allowable fiber stress.

Effect of Impact on Stresses. The formulas upon which the tables of safe loads are based assume all loads to be quiescent or static. The effect of moving loads may be taken care of either by reducing the allowable unit stresses, or else by increasing the theoretical loads.

When a load is suddenly applied, the resultant stresses are twice as great as those due to an equal quiescent load.

When an instantaneously applied load produces impact or percussion, the resultant stresses are dynamic and are measured by the laws governing the energy of bodies in motion. The following formulas give the fiber stress and deflection due to a load falling on center of a beam rigidly supported at both ends when the weight of beam is negligible as compared with that of falling load, and when no account is taken of the local distortion due to impact or percussion at point of application of load; but when the weight of the beam is a real factor, theoretical formulas do not agree with observed results and practical tests give values which are far less than those indicated by theoretical formulas; this is notably true in drop-tests of axles:

W =Weight of falling load, in pounds.

h =Height of fall, in inches.

f =Extreme fiber stress due to static effect of load, W,
in pounds per square inch.

f_d =Extreme fiber stress due to impact of load, W,
in pounds per square inch.

D =Deflection due to static effect of load, W, in inches.

D_d =Deflection due to impact of load, W, in inches.

$$f_d = f \left(1 + \sqrt{\frac{2h}{D} + 1} \right) \qquad D_d = D \left(1 + \sqrt{\frac{2h}{D} + 1} \right)$$

Shearing Stresses. The safe load tables for beams and channels are computed solely with reference to safe unit stresses due to flexure, and the safe loads uniformly distributed on the spans given will not produce excessive shearing stresses in the web.

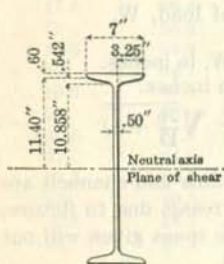
When, however, beams must support heavy loads which are concentrated near the supports, or when beams of short span are loaded with uniformly distributed loads to their full carrying capacity as regards flexure, the bending moments may be small in comparison with the reactions at the supports, and the beams may fail along the neutral plane as a result of longitudinal shearing stresses, or may buckle as a result of the combined longitudinal and vertical web stresses. On such spans the safe shearing or buckling strength of the web may limit the carrying capacity of the beam, so that the deciding factor will often be the resistance of the web to shearing stresses, rather than the resistance of the flanges to bending stresses.

Longitudinal Shear. At any point in any section of a beam, the horizontal and vertical components of the web stress are equal to each other and proportional to the vertical shear; their intensities are dependent upon the distance of the point from the neutral axis. In order to determine the intensity of the vertical shearing stress at a given point in a vertical section of the beam, therefore, it is sufficient to find the equal intensity of the horizontal shearing stress at the same point in the horizontal plane.

The longitudinal unit shear is zero at the upper and lower flanges of the beam and a maximum at the neutral plane. It is greatest at the supports and zero where there is no vertical shear.

The intensity of the longitudinal shear at any point in any section is the product of the vertical shear, V , for that section and the static moment, M_s , of the section included between the horizontal plane of shear through that point and the extreme fibers on the same side of the neutral plane divided by the product of the moment of inertia of the beam around the proper axis and the thickness at the plane of shear; or

$$\text{Longitudinal shear per square inch} = \frac{V M_s}{t I}$$



Example—Required the maximum longitudinal shear per square inch in a 24" 79.9 lb. beam loaded with two symmetrical loads of 100,000 pounds each, disregarding the weight of the beam.

$$M_s \text{ of Flange Rectangle} = 7 \times .60 \times 11.7 = 49.14$$

$$M_s \text{ of Flange Triangles} = 3.25 \times .542 \times 11.219 = 19.76$$

$$M_s \text{ of Web} = 11.40 \times .50 \times 5.70 = 32.49$$

$$\text{Total Static Moment} = 101.39$$

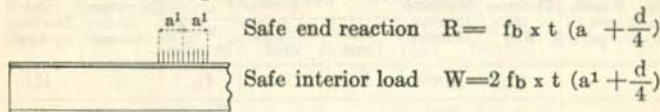
$$\text{Moment of Inertia of Beam, } I = 2087.2$$

$$\text{Longitudinal Shear} = \frac{100000 \times 101.39}{2087.2 \times .50} = 9715 \text{ pounds per square inch.}$$

Under usual conditions of loading, the longitudinal shear need not be taken into consideration.

Buckling Values of Beam Webs. The vertical shearing stresses or the vertical compressive components of the web stress may, under some conditions, exceed the safe resistance of the beam to buckling, and there remains the possibility that a web which is amply secure as against the safe allowed shear will not be of sufficient strength when considered as a column. In such cases provision must be made for security against buckling either by web stiffeners or by increasing the thickness of the web.

A series of experiments have been carried out on beams of various depths and web thicknesses to arrive at a basis for a simpler method of computation to use in the investigation of the safe buckling resistance of beams with unsupported webs, and from these experiments the following formulas have been deduced:



Safe end reaction $R = f_b \times t \left(a + \frac{d}{4} \right)$

Safe interior load $W = 2 f_b \times t \left(a^1 + \frac{d}{4} \right)$

In the formulas R is the end reaction, W the concentrated load, t the web thickness, d the depth of the beam, a^1 half the distance over which the concentrated load is applied and a the whole distance over which the end reaction is applied, while f_b is the safe resistance of the web to buckling in pounds per square inch, obtained from formula: $19000 - 100 \frac{d}{2r} = 19000 - 173 \frac{d}{t}$.

The first formula is general and applies to any condition of loading. The second formula is for a single load concentrated at the center of a span; it can be extended for a system of concentrated loads, provided the sum of the distances a^1 is not less than a .

The tables give for beams and channels with unsupported webs:

1. Allowed web resistance f_b , in pounds per square inch, computed from this compression formula.
2. The distance a , or the distance over which the end reaction must be distributed when the shearing stress, V , in the web is the maximum allowable of 12,000 and 10,000 pounds per square inch.
3. The allowable end reaction (R) when a is taken at $3\frac{1}{2}$ "', which is the usual length of beam actually resting on the 4" angles ordinarily used in building construction for beam seats.
4. The allowable shear V , on the gross area of beam or channel webs at 12,000 and 10,000 pounds per square inch.

Maximum Bending Moments. In addition to data referring to maximum loads on beams and channels as computed from the web resistance, these tables also give the maximum bending moments in foot pounds, obtained by multiplying the section modulus of each section by the allowed fiber stress of 18,000 and 16,000 pounds and dividing the product by 12 in order to reduce to a foot-pound basis.

These maximum bending moments may be used on inspection instead of the table of properties to ascertain the proper size section to be used in any particular instance.

CARNEGIE STEEL COMPANY

BEAMS

MAXIMUM BENDING MOMENTS AND WEB RESISTANCES

Bending Stress 18,000 Pounds—Shearing Stress 12,000 Pounds

Depth of Beam	Weight per Foot	Thickness of Web	Maximum Bending Moment	Web Resistance			Minimum End Bearing	End Reaction, $a=3\frac{1}{2}''$
				Web Shear	Minimum Span	Web Buckling		
d		t	M_{max}	V		f_b	a	R
Inches	Pounds	Inches	Foot Pounds	Pounds	Feet	Pounds per Sq. In.	Inches	Pounds
24	120.0	.798	376350	229820	6.55	13790	14.88	104560
	115.0	.737	367570	212260	6.93	13360	15.56	93550
	110.0	.675	358640	194400	7.38	12840	16.43	82350
	105.9	.625	351440	180000	7.81	12350	17.32	73320
24	100.0	.747	296470	215140	5.51	13430	15.44	95330
	95.0	.686	287690	197570	5.82	12940	16.26	84320
	90.0	.624	278760	179710	6.20	12340	17.35	73130
	85.0	.563	269980	162140	6.66	11620	18.80	62120
	79.9	.500	260900	144000	7.25	10680	20.96	50750
20	100.0	.873	247250	209520	4.72	15030	10.97	111550
	95.0	.800	239950	192000	5.00	14670	11.36	99750
	90.0	.726	232550	174240	5.34	14230	11.87	87810
	85.0	.653	225250	156720	5.75	13700	12.52	76010
	81.4	.600	219950	144000	6.11	13230	13.15	67450
20	75.0	.641	189520	153840	4.93	13590	12.65	74070
	70.0	.567	182120	136080	5.35	12890	13.62	62130
	65.4	.500	175420	120000	5.85	12070	14.88	51300
18	90.0	.796	209410	171940	4.87	15090	9.82	96080
	85.0	.714	202770	154220	5.26	14630	10.26	83580
	80.0	.632	196130	136510	5.75	14070	10.85	71140
	75.6	.560	190300	120960	6.29	13440	11.57	60210
18	70.0	.711	152920	153580	3.98	14620	10.27	83160
	65.0	.629	146280	135860	4.31	14050	10.87	70690
	60.0	.547	139640	118150	4.73	13310	11.73	58230
	54.7	.460	132590	99360	5.34	12230	13.16	45010
15	75.0	.868	137440	156240	3.52	16010	7.49	100730
	70.0	.770	131920	138600	3.81	15630	7.77	87230
	65.0	.672	126410	120960	4.18	15130	8.14	73730
	60.8	.590	121800	106200	4.59	14600	8.58	62430
15	55.0	.648	101740	116640	3.49	14990	8.26	70430
	50.0	.550	96230	99000	3.89	14280	8.86	56930
	45.0	.452	90690	81360	4.46	13250	9.83	43430
	42.9	.410	88350	73800	4.79	12670	10.46	37650

BEAM SAFE LOADS—FIBER STRESS 18,000

BEAMS

MAXIMUM BENDING MOMENTS AND WEB RESISTANCES

Bending Stress 18,000 Pounds—Shearing Stress 12,000 Pounds

Depth of Beam	Weight per Foot	Thickness of Web	Maximum Bending Moment	Web Resistance			Minimum End Bearing	End Reaction, $a=3\frac{1}{2}''$
				Web Shear	Minimum Span	Web Buckling		
d		t	M _{max}	V		f _b	a	R
Inches	Pounds	Inches	Foot Pounds	Pounds	Feet	Pounds per Sq. In.	Inches	Pounds
12	55.0	.810	79840	116640	2.74	16430	5.76	86530
	50.0	.687	75410	98930	3.05	15970	6.01	71330
	45.0	.565	71020	81360	3.49	15320	6.40	56270
	40.8	.460	67240	66240	4.06	14480	6.94	43300
12	35.0	.428	56760	61630	3.68	14150	7.18	39350
	31.8	.350	53950	50400	4.28	13060	8.03	29710
10	40.0	.741	47400	88920	2.13	16660	4.70	74080
	35.0	.594	43730	71280	2.45	16090	4.96	57330
	30.0	.447	40050	53640	2.99	15120	5.43	40560
	25.4	.310	36630	37200	3.94	13410	6.45	24950
9	35.0	.724	37100	78190	1.90	16850	4.16	70130
	30.0	.561	33800	60590	2.23	16220	4.41	52320
	25.0	.397	30470	42880	2.84	15070	4.92	34410
	21.8	.290	28310	31320	3.62	13620	5.68	22720
8	25.5	.532	25520	51070	2.00	16400	3.85	47970
	23.0	.441	24060	42340	2.27	15860	4.05	38460
	20.5	.349	22590	33500	2.70	15030	4.39	28850
	18.4	.270	21330	25920	3.29	13870	4.92	20590
7	20.0	.450	17980	37800	1.90	16310	3.40	38520
	17.5	.345	16690	28980	2.30	15490	3.67	28050
	15.3	.250	15530	21000	2.96	14150	4.19	18570
6	17.25	.465	13010	33480	1.55	16770	2.79	38980
	14.75	.343	11910	24700	1.93	15970	3.01	27390
	12.50	.230	10890	16560	2.63	14480	3.47	16660
5	14.75	.494	9030	29640	1.22	17250	2.23	40470
	12.25	.347	8110	20820	1.56	16510	2.39	27200
	10.0	.210	7250	12600	2.30	14880	2.78	14840
4	10.5	.400	5310	19200	1.11	17270	1.78	31080
	9.5	.326	5020	15650	1.28	16870	1.84	24750
	8.5	.253	4730	12140	1.56	16260	1.95	18510
	7.7	.190	4470	9120	1.96	15350	2.13	13120
3	7.5	.349	2880	12560	0.92	17510	1.31	25970
	6.5	.251	2660	9040	1.18	16930	1.38	18060
	5.7	.170	2480	6120	1.62	15950	1.51	11520

CARNEGIE STEEL COMPANY

CHANNELS

MAXIMUM BENDING MOMENTS AND WEB RESISTANCES

Bending Stress 18,000 Pounds—Shearing Stress 12,000 Pounds

Depth of Channel	Weight per Foot	Thickness of Web	Maximum Bending Moment	Web Resistance			Minimum End Bearing	End Reaction, $a=3\frac{1}{2}d$	
				Web Shear	Minimum Span	Web Buckling			
d		t	M _{max}	V		fb	a	R	
Inches	Pounds	Inches	Foot Pounds	Pounds	Feet	Pounds per Sq. In.	Inches	Pounds	
15	55.0	.814	85800	146520	2.34	15810	7.64	93290	
	50.0	.716	80340	128880	2.49	15370	7.96	79790	
	45.0	.618	74770	111240	2.69	14800	8.42	66290	
	40.0	.520	69260	93600	2.96	14000	9.10	52790	
	35.0	.422	63750	75960	3.36	12840	10.27	39290	
	33.9	.400	62510	72000	3.47	12510	10.64	36270	
13	50.0	.787	72210	122770	2.35	16140	6.42	85730	
	45.0	.673	67400	104990	2.57	15660	6.71	71120	
	40.0	.560	62620	87360	2.87	14980	7.16	56620	
	37.0	.492	59750	76750	3.11	14420	7.57	47900	
	35.0	.447	57850	69730	3.32	13960	7.92	42120	
	31.8	.375	54800	58500	3.75	13000	8.75	32900	
12	40.0	.755	49130	108720	1.81	16250	5.86	79740	
	35.0	.632	44700	91010	1.96	15710	6.17	64540	
	30.0	.510	40310	73440	2.20	14920	6.65	49470	
	25.0	.387	35880	55730	2.58	13630	7.57	34280	
	20.7	.280	32030	40320	3.18	11570	9.44	21060	
	10	35.0	.820	34560	98400	1.41	16890	4.61	83090
30.0		.673	30890	80760	1.53	16430	4.81	66330	
25.0		.526	27210	63120	1.72	15710	5.14	49570	
20.0		.379	23540	45480	2.07	14430	5.82	32810	
15.3		.240	20060	28800	2.79	11790	7.68	16970	
9		25.0	.612	23510	66100	1.42	16450	4.31	57900
	20.0	.448	20190	48380	1.67	15520	4.71	39980	
	15.0	.285	16890	30780	2.19	13530	5.73	22180	
	13.4	.230	15770	24840	2.54	12220	6.59	16160	
	8	21.25	.579	17860	55580	1.29	16610	3.78	52880
		18.75	.487	16390	46750	1.40	16160	3.94	43270
16.25		.395	14910	37920	1.57	15490	4.20	33650	
13.75		.303	13440	29090	1.85	14430	4.65	24040	
11.5		.220	12110	21120	2.29	12700	5.56	15370	
7		19.75	.629	14170	52840	1.07	17070	3.17	56380
	17.25	.524	12880	44020	1.17	16690	3.28	45910	
	14.75	.419	11600	35200	1.32	16110	3.47	35430	
	12.25	.314	10310	26380	1.56	15140	3.80	24950	
	9.8	.210	9040	17640	2.05	13220	4.60	14580	
	6	15.5	.559	9730	40250	0.97	17140	2.70	47910
13.0		.437	8630	31460	1.10	16620	2.83	36320	
10.5		.314	7530	22610	1.33	15690	3.09	24630	
8.2		.200	6500	14400	1.81	13800	3.72	13800	
5		11.5	.472	6210	28320	0.88	17170	2.25	38490
		9.0	.325	5290	19500	1.09	16340	2.42	25220
	6.7	.190	4450	11400	1.56	14440	2.91	13030	
	4	7.25	.320	3410	15360	0.89	16840	1.85	24240
		6.25	.247	3110	11860	1.05	16200	1.96	18000
		5.4	.180	2840	8640	1.32	15150	2.17	12270
3		6.0	.356	2060	12820	0.64	17540	1.30	26540
		5.0	.258	1830	9290	0.79	16990	1.37	18630
		4.1	.170	1640	6120	1.07	15950	1.51	11520

BEAM SAFE LOADS—FIBER STRESS 18,000

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 18,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections									Coefficient of Deflection
	24 Inch									
	120 lbs.	115 lbs.	110 lbs.	105.9 lbs.	100 lbs.	95 lbs.	90 lbs.	85 lbs.	79.9 lbs.	
6	495.6	424.5			430.3	395.1				0.67
7	430.1	420.1			395.3	383.6				0.91
8	376.4	367.6	388.8	360.0	338.8	328.8	318.6	308.6	288.0	1.19
9	334.5	326.7	318.8	312.4	296.5	287.7	278.8	270.0	260.9	1.51
10	301.1	294.1	286.9	281.2	263.5	255.7	247.8	240.0	231.9	1.86
11	273.7	267.3	260.8	255.6	215.6	209.2	202.7	196.4	189.7	2.25
12	250.9	245.0	239.1	234.3	197.7	191.8	185.8	180.0	173.9	2.68
13	231.6	226.2	220.7	216.3	182.4	177.0	171.5	166.1	160.6	3.15
14	215.1	210.0	204.9	200.8	169.4	164.4	159.3	154.3	149.1	3.65
15	200.7	196.0	191.3	187.4	158.1	153.4	148.7	144.0	139.1	4.19
16	188.2	183.8	179.3	175.7	148.2	143.8	139.4	135.0	130.5	4.77
17	177.1	173.0	168.8	165.4	139.5	135.4	131.2	127.0	122.8	5.38
18	167.3	163.4	159.4	156.2	131.8	127.9	123.9	120.0	116.0	6.03
19	158.5	154.8	151.0	148.0	124.8	121.1	117.4	113.7	109.9	6.72
20	150.5	147.0	143.5	140.6	118.6	115.1	111.5	108.0	104.4	7.45
21	143.4	140.0	136.6	133.9	112.9	109.6	106.2	102.8	99.4	8.21
22	136.9	133.7	130.4	127.8	107.8	104.6	101.4	98.2	94.9	9.01
23	130.9	127.8	124.7	122.2	103.1	100.1	97.0	93.9	90.7	9.85
24	125.5	122.5	119.5	117.1	98.8	95.9	92.9	90.0	87.0	10.73
25	120.4	117.6	114.8	112.5	94.9	92.1	89.2	86.4	83.5	11.64
26	115.8	113.1	110.4	108.1	91.2	88.5	85.8	83.1	80.3	12.59
27	111.5	108.9	106.3	104.1	87.8	85.2	82.6	80.0	77.3	13.57
28	107.5	105.0	102.5	100.4	84.7	82.2	79.6	77.1	74.5	14.60
29	103.8	101.4	98.9	96.9	81.8	79.4	76.9	74.5	72.0	15.66
30	100.4	98.0	95.6	93.7	79.1	76.7	74.3	72.0	69.6	16.76
31	97.1	94.9	92.6	90.7	76.5	74.2	71.9	69.7	67.3	17.89
32	94.1	91.9	89.7	87.9	74.1	71.9	69.7	67.5	65.2	19.07
33	91.2	89.1	86.9	85.2	71.9	69.7	67.6	65.4	63.3	20.28
34	88.6	86.5	84.4	82.7	69.8	67.7	65.6	63.5	61.4	21.53
35	86.0	84.0	82.0	80.3	67.8	65.8	63.7	61.7	59.6	22.81
36	83.6	81.7	79.7	78.1	65.9	63.9	61.9	60.0	58.0	24.13
37	81.4	79.5	77.5	76.0	64.1	62.2	60.3	58.4	56.4	25.49
38	79.2	77.4	75.5	74.0	62.4	60.6	58.7	56.8	54.9	26.89
39	77.2	75.4	73.6	72.1	60.8	59.0	57.2	55.4	53.5	28.32
40	75.3	73.5	71.7	70.3	59.3	57.5	55.8	54.0	52.2	29.79
41	73.4	71.7	70.0	68.6	57.8	56.1	54.4	52.7	50.9	31.30
42	71.7	70.0	68.3	66.9	56.5	54.8	53.1	51.4	49.7	32.85
43	70.0	68.4	66.7	65.4	55.2	53.5	51.9	50.2	48.5	34.43
44	68.4	66.8	65.2	63.9	53.9	52.3	50.7	49.1	47.4	36.05
45	66.9	65.3	63.8	62.5	52.7	51.1	49.6	48.0	46.4	37.71
46	65.5	63.9	62.4	61.1	51.6	50.0	48.5	47.0	45.4	39.40
47	64.1	62.6	61.0	59.8	50.5	49.0	47.5	46.0	44.4	41.13
48	62.7	61.3	59.8	58.6	49.4	47.9	46.5	45.0	43.5	42.90
49	61.5	60.0	58.6	57.4	48.4	47.0	45.5	44.1	42.6	44.71
50	60.2	58.9	57.4	56.2	47.4	46.0	44.6	43.2	41.7	46.55

Loads above upper horizontal lines will produce maximum allowable shear in webs.
Loads below lower horizontal lines will produce excessive deflections.
For maximum safe loads, see tables of Maximum Bending Moments and Web Resistances.

CARNEGIE STEEL COMPANY

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 18,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection		
	20 Inch								18 Inch						
	100 lbs.	95 lbs.	90 lbs.	85 lbs.	81.4 lbs.	75 lbs.	70 lbs.	65.4 lbs.	90 lbs.	85 lbs.	80 lbs.	75.6 lbs.			
5	419.0	384.0				307.7			343.9						0.47
6	329.7	319.9	310.1	300.3	288.0	252.7	242.8	233.9	279.2	270.4	261.5	241.9			0.67
7	282.6	274.2	265.8	257.4	251.4	216.6	208.1	200.5	239.3	231.7	224.1	217.5			0.91
8	247.2	239.9	232.5	225.2	219.9	189.5	182.1	175.4	209.4	202.8	196.1	190.3			1.19
9	219.8	213.3	206.7	200.2	195.5	168.5	161.9	155.9	186.1	180.2	174.3	169.2			1.51
10	197.8	192.0	186.0	180.2	176.0	151.6	145.7	140.3	167.5	162.2	156.9	152.2			1.86
11	179.8	174.5	169.1	163.8	160.0	137.8	132.5	127.6	152.3	147.5	142.6	138.4			2.25
12	164.8	160.0	155.0	150.2	146.6	126.3	121.4	116.9	139.6	135.2	130.8	126.9			2.68
13	152.2	147.7	143.1	138.6	135.3	116.6	112.1	108.0	128.9	124.8	120.7	117.1			3.15
14	141.3	137.1	132.9	128.7	125.7	108.3	104.1	100.2	119.7	115.9	112.1	108.7			3.65
15	131.9	128.0	124.0	120.1	117.3	101.1	97.1	93.6	111.7	108.1	104.6	101.5			4.19
16	123.6	120.0	116.3	112.6	110.0	94.8	91.1	87.7	104.7	101.4	98.1	95.1			4.77
17	116.4	112.9	109.4	106.0	103.5	89.2	85.7	82.6	98.6	95.4	92.3	89.6			5.38
18	109.9	106.6	103.4	100.1	97.8	84.2	80.9	78.0	93.1	90.1	87.2	84.6			6.03
19	104.1	101.0	97.9	94.8	92.6	79.8	76.7	73.9	88.2	85.4	82.6	80.1			6.72
20	98.9	96.0	93.0	90.1	88.0	75.8	72.8	70.2	83.8	81.1	78.5	76.1			7.45
21	94.2	91.4	88.6	85.8	83.8	72.2	69.4	66.8	79.8	77.3	74.7	72.5			8.21
22	89.9	87.2	84.6	81.9	80.0	68.9	66.2	63.8	76.2	73.7	71.3	69.2			9.01
23	86.0	83.5	80.9	78.3	76.5	65.9	63.3	61.0	72.8	70.5	68.2	66.2			9.85
24	82.4	80.0	77.5	75.1	73.3	63.2	60.7	58.5	69.8	67.6	65.4	63.4			10.73
25	79.1	76.8	74.4	72.1	70.4	60.6	58.3	56.1	67.0	64.9	62.8	60.9			11.64
26	76.1	73.8	71.6	69.3	67.7	58.3	56.0	54.0	64.4	62.4	60.4	58.6			12.59
27	73.3	71.1	68.9	66.7	65.2	56.2	54.0	52.0	62.1	60.1	58.1	56.4			13.57
28	70.6	68.6	66.4	64.4	62.8	54.2	52.0	50.1	59.8	57.9	56.0	54.4			14.60
29	68.2	66.2	64.2	62.1	60.7	52.3	50.2	48.4	57.8	55.9	54.1	52.5			15.66
30	65.9	64.0	62.0	60.1	58.7	50.5	48.6	46.8	55.8	54.1	52.3	50.7			16.76
31	63.8	61.9	60.0	58.1	56.8	48.9	47.0	45.3	54.0	52.3	50.6	49.1			17.89
32	61.8	60.0	58.1	56.3	55.0	47.4	45.5	43.9	52.4	50.7	49.0	47.6			19.07
33	59.9	58.2	56.4	54.6	53.3	45.9	44.2	42.5	50.8	49.2	47.5	46.1			20.28
34	58.2	56.5	54.7	53.0	51.8	44.6	42.9	41.3	49.3	47.7	46.2	44.8			21.53
35	56.5	54.8	53.2	51.5	50.3	43.3	41.6	40.1	47.9	46.4	44.8	43.5			22.81
36	54.9	53.3	51.7	50.1	48.9	42.1	40.5	39.0	46.5	45.1	43.6	42.3			24.13
37	53.5	51.9	50.3	48.7	47.6	41.0	39.4	37.9	45.3	43.9	42.4	41.2			25.49
38	52.1	50.5	49.0	47.4	46.3	39.9	38.3	36.9	44.1	42.7	41.3	40.1			26.89
39	50.7	49.2	47.7	46.2	45.1	38.9	37.4	36.0							28.32
40	49.4	48.0	46.5	45.0	44.0	37.9	36.4	35.1							29.79
41	48.2	46.8	45.4	44.0	42.9	37.0	35.5	34.2							31.30
42	47.1	45.7	44.3	42.9	41.9	36.1	34.7	33.4							32.85

Loads above upper horizontal lines will produce maximum allowable shear in webs.

Loads below horizontal lines will produce excessive deflections.

For maximum safe loads, see tables of Maximum Bending Moments and Web Resistances.

BEAM SAFE LOADS—FIBER STRESS 18,000

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS
 Applicable only when sections are braced against lateral deflection.
 For unbraced sections safe loads must be reduced, see page 101.
 Maximum Bending Stress, 18,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection			
	18 Inch				15 Inch											
	70 lbs.	65 lbs.	60 lbs.	54.7 lbs.	75 lbs.	70 lbs.	65 lbs.	60.8 lbs.	55 lbs.	50 lbs.	45 lbs.	42.9 lbs.				
4	307.2				312.5	277.3			233.3	198.0						0.30
5	305.8	271.7	236.3		274.9	263.8	241.9	212.4	203.5	192.5	162.7	147.6				0.47
	244.7	234.0	223.4		219.9	211.1	202.3	194.9	162.8	154.0	145.1	141.4				
6	203.9	195.0	186.2	198.7	183.3	175.9	168.5	162.4	135.7	128.3	120.9	117.8				0.67
7	174.8	167.2	159.6	151.5	157.1	150.8	144.5	139.2	116.3	110.0	103.6	101.0				0.91
8	152.9	146.3	139.6	132.6	137.4	131.9	126.4	121.8	101.7	96.2	90.7	88.4				1.19
9	135.9	130.0	124.1	117.9	122.2	117.3	112.4	108.3	90.4	85.5	80.6	78.5				1.51
10	122.3	117.0	111.7	106.1	110.0	105.5	101.1	97.4	81.4	77.0	72.6	70.7				1.86
11	111.2	106.4	101.6	96.4	100.0	95.9	91.9	88.6	74.0	70.0	66.0	64.3				2.25
12	101.9	97.5	93.1	88.4	91.6	88.0	84.3	81.2	67.8	64.2	60.5	58.9				2.68
13	94.1	90.0	85.9	81.6	84.6	81.2	77.8	75.0	62.6	59.2	55.8	54.4				3.15
14	87.4	83.6	79.8	75.8	78.5	75.4	72.2	69.6	58.1	55.0	51.8	50.5				3.65
15	81.6	78.0	74.5	70.7	73.3	70.4	67.4	65.0	54.3	51.3	48.4	47.1				4.19
16	76.5	73.1	69.8	66.3	68.7	66.0	63.2	60.9	50.9	48.1	45.3	44.2				4.77
17	72.0	68.8	65.7	62.4	64.7	62.1	59.5	57.3	47.9	45.3	42.7	41.6				5.38
18	68.0	65.0	62.1	58.9	61.1	58.6	56.2	54.1	45.2	42.8	40.3	39.3				6.03
19	64.4	61.6	58.8	55.8	57.9	55.5	53.2	51.3	42.8	40.5	38.2	37.2				6.72
20	61.2	58.5	55.9	53.0	55.0	52.8	50.6	48.7	40.7	38.5	36.3	35.3				7.45
21	58.3	55.7	53.2	50.5	52.4	50.3	48.2	46.4	38.8	36.7	34.5	33.7				8.21
22	55.6	53.2	50.8	48.2	50.0	48.0	46.0	44.3	37.0	35.0	33.0	32.1				9.01
23	53.2	50.9	48.6	46.1	47.8	45.9	44.0	42.4	35.4	33.5	31.5	30.7				9.85
24	51.0	48.8	46.5	44.2	45.8	44.0	42.1	40.6	33.9	32.1	30.2	29.5				10.73
25	48.9	46.8	44.7	42.4	44.0	42.2	40.5	39.0	32.6	30.8	29.0	28.3				11.64
26	47.1	45.0	43.0	40.8	42.3	40.6	38.9	37.5	31.3	29.6	27.9	27.2				12.59
27	45.3	43.3	41.4	39.3	40.7	39.1	37.5	36.1	30.1	28.5	26.9	26.2				13.57
28	43.7	41.8	39.9	37.9	39.3	37.7	36.1	34.8	29.1	27.5	25.9	25.2				14.60
29	42.2	40.4	38.5	36.6	37.9	36.4	34.9	33.6	28.1	26.5	25.0	24.4				15.66
30	40.8	39.0	37.2	35.4	36.7	35.2	33.7	32.5	27.1	25.7	24.2	23.6				16.76
31	39.5	37.8	36.0	34.2	35.5	34.0	32.6	31.4	26.3	24.8	23.4	22.8				17.89
32	38.2	36.6	34.9	33.1	34.4	33.0	31.6	30.4	25.4	24.1	22.7	22.1				19.07
33	37.1	35.5	33.8	32.1												20.28
34	36.0	34.4	32.9	31.2												21.53
35	35.0	33.4	31.9	30.3												22.81
36	34.0	32.5	31.0	29.5												24.13
37	33.1	31.6	30.2	28.7												25.49
38	32.2	30.8	29.4	27.9												26.89

Loads above upper horizontal lines will produce maximum allowable shear in webs.
 Loads below lower horizontal lines will produce excessive deflections.
 For maximum safe loads, see tables of Maximum Bending Moments and Web Resistances.

CARNEGIE STEEL COMPANY

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 18,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections										Coefficient of Deflection	
	12 Inch					10 Inch						
	55 lbs.	50 lbs.	45 lbs.	40.8 lbs.	35 lbs.	31.8 lbs.	40 lbs.	35 lbs.	30 lbs.	25.4 lbs.		
3	233.3						177.8	142.6	107.3			
4	212.9	197.9	162.7		153.3		126.4	116.6	106.8	74.4		0.17
5	159.7	150.8	142.0	132.5	113.5	100.8	94.8	87.5	80.1	73.3		0.30
6	127.7	120.7	113.6	107.6	90.8	86.3	75.8	70.0	64.1	58.6		0.47
7	106.5	100.5	94.7	89.7	75.7	71.9	63.2	58.3	53.4	48.8		0.67
8	91.3	86.2	81.2	76.8	64.9	61.7	54.2	50.0	45.8	41.9		0.91
9	79.8	75.4	71.0	67.2	56.8	54.0	47.4	43.7	40.1	36.6		1.19
10	71.0	67.0	63.1	59.8	50.5	48.0	42.1	38.9	35.6	32.6		1.51
11	63.9	60.3	56.8	53.8	45.4	43.2	37.9	35.0	32.0	29.3		1.86
12	58.1	54.8	51.6	48.9	41.3	39.2	34.5	31.8	29.1	26.6		2.25
13	53.2	50.3	47.3	44.8	37.8	36.0	31.6	29.2	26.7	24.4		2.68
14	49.1	46.4	43.7	41.4	34.9	33.2	29.2	26.9	24.6	22.5		3.15
15	45.6	43.1	40.6	38.4	32.4	30.8	27.1	25.0	22.9	20.9		3.65
16	42.6	40.2	37.9	35.9	30.3	28.8	25.3	23.3	21.4	19.5		4.19
17	39.9	37.7	35.5	33.6	28.4	27.0	23.7	21.9	20.0	18.3		4.77
18	37.6	35.5	33.4	31.6	26.7	25.4	22.3	20.6	18.8	17.2		5.38
19	35.5	33.5	31.6	29.9	25.2	24.0	21.1	19.4	17.8	16.3		6.03
20	33.6	31.8	29.9	28.3	23.9	22.7	20.0	18.4	16.9	15.4		6.72
21	31.9	30.2	28.4	26.9	22.7	21.6	19.0	17.5	16.0	14.7		7.45
22	30.4	28.7	27.1	25.6	21.6	20.6	18.1	16.7	15.3	14.0		8.21
23	29.0	27.4	25.8	24.4	20.6	19.6	17.2	15.9	14.6	13.3		9.01
24	27.8	26.2	24.7	23.4	19.7	18.8						9.85
25	26.6	25.1	23.7	22.4	18.9	18.0						10.73
26	25.5	24.1	22.7	21.5	18.2	17.3						11.64
26	24.6	23.2	21.8	20.7	17.5	16.6						12.59

Loads above upper horizontal lines will produce maximum allowable shear in webs.
Loads below lower horizontal lines will produce excessive deflections.
For maximum safe loads, see tables of Maximum Bending Moments and Web Resistances.

BEAM SAFE LOADS—FIBER STRESS 18,000

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS
 Applicable only when sections are braced against lateral deflection.
 For unbraced sections safe loads must be reduced, see page 101.
 Maximum Bending Stress, **18,000** Pounds per Square Inch

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection	
	9 Inch				8 Inch				7 Inch					
	35 lbs.	30 lbs.	25 lbs.	21.8 lbs.	25.5 lbs.	23 lbs.	20.5 lbs.	18.4 lbs.	20 lbs.	17.5 lbs.	15.3 lbs.			
2	156.4				102.1				75.6					
3	148.4	121.2	85.8		102.1	84.7	67.0		71.9	58.0	42.0			0.07
4	98.9	90.1	81.3	62.6	68.1	64.2	60.2	51.8	47.9	44.5	41.4			0.17
5	74.2	67.6	60.9	56.6	51.0	48.1	45.2	42.7	36.0	33.4	31.1			0.30
6	59.4	54.1	48.8	45.3	40.8	38.5	36.1	34.1	28.8	26.7	24.8			0.47
7	49.5	45.1	40.6	37.7	34.0	32.1	30.1	28.4	24.0	22.3	20.7			0.67
8	42.4	38.6	34.8	32.4	29.2	27.5	25.8	24.4	20.5	19.1	17.7			0.91
9	37.1	33.8	30.5	28.3	25.5	24.1	22.6	21.3	18.0	16.7	15.5			1.19
10	33.0	30.0	27.1	25.2	22.7	21.4	20.1	19.0	16.0	14.8	13.8			1.51
11	29.7	27.0	24.4	22.6	20.4	19.3	18.1	17.1	14.4	13.4	12.4			1.86
12	27.0	24.6	22.2	20.6	18.6	17.5	16.4	15.5	13.1	12.1	11.3			2.25
13	24.7	22.5	20.3	18.9	17.0	16.0	15.1	14.2	12.0	11.1	10.4			2.68
14	22.8	20.8	18.8	17.4	15.7	14.8	13.9	13.1	11.1	10.3	9.6			3.15
15	21.2	19.3	17.4	16.2	14.6	13.7	12.9	12.2	10.3	9.5	8.9			3.65
16	19.8	18.0	16.3	15.1	13.6	12.8	12.0	11.4	9.6	8.9	8.3			4.19
17	18.5	16.9	15.2	14.2	12.8	12.0	11.3	10.7	9.0	8.3	7.8			4.77
18	17.5	15.9	14.3	13.3	12.0	11.3	10.6	10.0						5.38
19	16.5	15.0	13.5	12.6	11.3	10.7	10.0	9.5						6.03
20	15.6	14.2	12.8	11.9										6.72
20	14.8	13.5	12.2	11.3										7.45

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection	
	6 Inch			5 Inch			4 Inch			3 Inch				
	17.25 lbs.	14.75 lbs.	12.5 lbs.	14.75 lbs.	12.25 lbs.	10 lbs.	10.5 lbs.	9.5 lbs.	8.5 lbs.	7.7 lbs.	7.5 lbs.	6.5 lbs.		5.7 lbs.
1	67.0	49.4		59.3	41.6		38.4	31.3	24.3	18.2	23.1	18.1	12.2	0.02
2	52.0	47.6	33.1	36.1	32.4	25.2	21.3	20.1	18.9	17.9	11.5	10.7	9.9	0.07
3	34.7	31.8	29.0	24.1	21.6	19.3	14.2	13.4	12.6	11.9	7.7	7.1	6.6	0.17
4	26.0	23.8	21.8	18.1	16.2	14.5	10.6	10.0	9.5	8.9	5.8	5.3	5.0	0.30
5	20.8	19.1	17.4	14.4	13.0	11.6	8.5	8.0	7.6	7.2	4.6	4.3	4.0	0.47
6	17.3	15.9	14.5	12.0	10.8	9.7	7.1	6.7	6.3	6.0	3.9	3.6	3.3	0.67
7	14.9	13.6	12.4	10.3	9.3	8.3	6.1	5.7	5.4	5.1	3.3	3.0	2.8	0.91
8	13.0	11.9	10.9	9.0	8.1	7.3	5.3	5.0	4.7	4.5	2.9	2.7	2.5	1.19
9	11.6	10.6	9.7	8.0	7.2	6.4	4.7	4.5	4.2	4.0				1.51
10	10.4	9.5	8.7	7.2	6.5	5.8	4.3	4.0	3.8	3.6				1.86
11	9.5	8.7	7.9	6.6	5.9	5.3								2.25
12	8.7	7.9	7.3	6.0	5.4	4.8								2.68
13	8.0	7.3	6.7											3.15
14	7.4	6.8	6.2											3.65

Loads above upper horizontal lines will produce maximum allowable shear in webs.
 Loads below lower horizontal lines will produce excessive deflections.
 For maximum safe loads, see tables of Maximum Bending Moments and Web Resistances.

CHANNELS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS
 Applicable only when sections are braced against lateral deflection.
 For unbraced sections safe loads must be reduced, see page 101.
 Maximum Bending Stress, 18,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection	
	15 Inch						13 Inch							
	55 lbs.	50 lbs.	45 lbs.	40 lbs.	35 lbs.	33.9 lbs.	50 lbs.	45 lbs.	40 lbs.	37 lbs.	35 lbs.	31.8 lbs.		
	293.0	257.8	222.5	187.2			245.5	210.0	174.7					
3	228.8	214.2	199.4	184.7		151.9	144.0	192.6	179.7	167.0	153.5	139.5	117.9	0.17
4	171.6	160.7	149.5	138.5	127.5	125.0	144.4	134.8	125.2	119.5	115.7	109.6	103.6	0.30
5	137.3	128.5	119.6	110.8	102.0	100.0	115.5	107.8	100.2	95.6	92.6	87.7	82.7	0.47
6	114.4	107.1	99.7	92.4	85.0	83.4	96.3	89.9	83.5	79.7	77.1	73.1	69.1	0.67
7	98.1	91.8	85.5	79.2	72.9	71.4	82.5	77.0	71.6	68.3	66.1	62.6	58.8	0.91
8	85.8	80.3	74.8	69.3	63.7	62.5	72.2	67.4	62.6	59.7	57.8	54.8	51.8	1.19
9	76.3	71.4	66.5	61.6	56.7	55.6	64.2	59.9	55.7	53.1	51.4	48.7	46.1	1.51
10	68.6	64.3	59.8	55.4	51.0	50.0	57.8	53.9	50.1	47.8	46.3	43.8	41.8	1.86
11	62.4	58.4	54.4	50.4	46.4	45.5	52.5	49.0	45.5	43.5	42.1	39.9	38.1	2.25
12	57.2	53.6	49.8	46.2	42.5	41.7	48.1	44.9	41.7	39.8	38.6	36.5	34.8	2.68
13	52.8	49.4	46.0	42.6	39.2	38.5	44.4	41.5	38.5	36.8	35.6	33.7	32.1	3.15
14	49.0	45.9	42.7	39.6	36.4	35.7	41.3	38.5	35.8	34.1	33.1	31.3	29.8	3.65
15	45.8	42.8	39.9	36.9	34.0	33.3	38.5	35.9	33.4	31.9	30.9	29.2	27.7	4.19
16	42.9	40.2	37.4	34.6	31.9	31.3	36.1	33.7	31.3	29.9	28.9	27.4	26.0	4.77
17	40.4	37.8	35.2	32.6	30.0	29.4	34.0	31.7	29.5	28.1	27.2	25.8	24.5	5.38
18	38.1	35.7	33.2	30.8	28.3	27.8	32.1	30.0	27.8	26.6	25.7	24.4	23.2	6.03
19	36.1	33.8	31.5	29.2	26.8	26.3	30.4	28.4	26.4	25.2	24.4	23.1	22.0	6.72
20	34.3	32.1	29.9	27.7	25.5	25.0	28.9	27.0	25.0	23.9	23.1	21.9	20.9	7.45
21	32.7	30.6	28.5	26.4	24.3	23.8	27.5	25.7	23.9	22.8	22.0	20.9	20.0	8.21
22	31.2	29.2	27.2	25.2	23.2	22.7	26.3	24.5	22.8	21.7	21.0	19.9	19.0	9.01
23	29.8	27.9	26.0	24.1	22.2	21.7	25.1	23.4	21.8	20.8	20.1	19.1	18.2	9.85
24	28.6	26.8	24.9	23.1	21.2	20.8	24.1	22.5	20.9	19.9	19.3	18.3	17.4	10.73
25	27.5	25.7	23.9	22.2	20.4	20.0	23.1	21.6	20.0	19.1	18.5	17.5	16.7	11.64
26	26.4	24.7	23.0	21.3	19.6	19.2	22.2	20.7	19.3	18.4	17.8	16.9	16.1	12.59
27	25.4	23.8	22.2	20.5	18.9	18.5	21.4	20.0	18.6	17.7	17.1	16.2	15.4	13.57
28	24.5	23.0	21.4	19.8	18.2	17.9	20.6	19.3	17.9	17.1	16.5	15.7	14.9	14.60
29	23.7	22.2	20.6	19.1	17.6	17.2								15.66
30	22.9	21.4	19.9	18.5	17.0	16.7								16.76
31	22.1	20.7	19.3	17.9	16.5	16.1								17.89
32	21.4	20.1	18.7	17.3	15.9	15.6								19.07

Loads above upper horizontal lines will produce maximum allowable shear in webs.
 Loads below lower horizontal lines will produce excessive deflections.
 For maximum safe loads, see tables of Maximum Bending Moments and Web Resistances.

BEAM SAFE LOADS—FIBER STRESS 18,000

CHANNELS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS
 Applicable only when sections are braced against lateral deflection.
 For unbraced sections safe loads must be reduced, see page 101.
 Maximum Bending Stress, **18,000** Pounds per Square Inch

Span in Feet	Depth and Weight of Sections										Coefficient of Deflection	
	12 Inch					10 Inch						
	40 lbs.	35 lbs.	30 lbs.	25 lbs.	20 lbs.	35 lbs.	30 lbs.	25 lbs.	20 lbs.	15 lbs.		
	217.4	182.0				196.8	161.5	126.2				
2	196.5	178.8	146.9	111.5		138.3	123.6	108.9	91.0	57.6		0.07
3	131.0	119.2	107.5	95.7	80.6	92.2	82.4	72.6	62.8	53.5		0.17
4	98.3	89.4	80.6	71.8	64.1	69.1	61.8	54.4	47.1	40.1		0.30
5	78.6	71.5	64.5	57.4	51.2	55.3	49.4	43.5	37.7	32.1		0.47
6	65.5	59.6	53.7	47.8	42.7	46.1	41.2	36.3	31.4	26.8		0.67
7	56.2	51.1	46.1	41.0	36.6	39.5	35.3	31.1	26.9	22.9		0.91
8	49.1	44.7	40.3	35.9	32.0	34.6	30.9	27.2	23.5	20.1		1.19
9	43.7	39.7	35.8	31.9	28.5	30.7	27.5	24.2	20.9	17.8		1.51
10	39.3	35.8	32.2	28.7	25.6	27.7	24.7	21.8	18.8	16.1		1.86
11	35.7	32.5	29.3	26.1	23.3	25.1	22.5	19.8	17.1	14.6		2.25
12	32.8	29.8	26.9	23.9	21.4	23.0	20.6	18.1	15.7	13.4		2.68
13	30.2	27.5	24.8	22.1	19.7	21.3	19.0	16.7	14.5	12.3		3.15
14	28.1	25.5	23.0	20.5	18.3	19.8	17.7	15.6	13.5	11.5		3.65
15	26.2	23.8	21.5	19.1	17.1	18.4	16.5	14.5	12.6	10.7		4.19
16	24.6	22.3	20.2	17.9	16.0	17.3	15.4	13.6	11.8	10.0		4.77
17	23.1	21.0	19.0	16.9	15.1	16.3	14.5	12.8	11.1	9.4		5.38
18	21.8	19.9	17.9	15.9	14.2	15.4	13.7	12.1	10.5	8.9		6.03
19	20.7	18.8	17.0	15.1	13.5	14.6	13.0	11.5	9.9	8.4		6.72
20	19.7	17.9	16.1	14.4	12.8	13.8	12.4	10.9	9.4	8.0		7.45
21	18.7	17.0	15.4	13.7	12.2	13.2	11.8	10.4	9.0	7.6		8.21
22	17.9	16.3	14.7	13.0	11.6	12.6	11.2	9.9	8.6	7.3		9.01
23	17.1	15.5	14.0	12.5	11.1							9.85
24	16.4	14.9	13.4	12.0	10.7							10.73
25	15.7	14.3	12.9	11.5	10.2							11.64
26	15.1	13.8	12.4	11.0	9.9							12.59

Loads above upper horizontal lines will produce maximum allowable shear in webs.
 Loads below lower horizontal lines will produce excessive deflections.
 For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

CHANNELS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS
 Applicable only when sections are braced against lateral deflection.
 For unbraced sections safe loads must be reduced, see page 101.
 Maximum Bending Stress, 18,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections														Coefficient of Deflection	
	9 Inch				8 Inch				7 Inch							
	25 lbs.	20 lbs.	15 lbs.	13.4 lbs.	21.25 lbs.	18.75 lbs.	16.25 lbs.	13.75 lbs.	11.5 lbs.	19.75 lbs.	17.25 lbs.	14.75 lbs.	12.25 lbs.	9.8 lbs.		
	132.2	96.8			111.2	93.5	75.8	58.2		105.7	88.0	70.4	52.8			
2	94.0	80.8	61.6	49.7	71.4	65.5	59.7	53.8	42.2	56.7	51.5	46.4	41.2	35.3		0.07
3	62.7	53.8	45.0	42.1	47.6	43.7	39.8	35.8	32.3	37.8	34.3	30.9	27.5	24.1		0.17
4	47.0	40.4	33.8	31.5	35.7	32.8	29.8	26.9	24.2	28.3	25.8	23.2	20.6	18.1		0.30
5	37.6	32.3	27.0	25.2	28.6	26.2	23.9	21.5	19.4	22.7	20.6	18.6	16.5	14.5		0.47
6	31.4	26.9	22.5	21.0	23.8	21.9	19.9	17.9	16.2	18.9	17.2	15.5	13.7	12.0		0.67
7	26.9	23.1	19.3	18.0	20.4	18.7	17.0	15.4	13.8	16.2	14.7	13.3	11.8	10.3		0.91
8	23.5	20.2	16.9	15.8	17.9	16.4	14.9	13.4	12.1	14.2	12.9	11.6	10.3	9.0		1.19
9	20.9	17.9	15.0	14.0	15.9	14.6	13.3	11.9	10.8	12.6	11.4	10.3	9.2	8.0		1.51
10	18.8	16.2	13.5	12.6	14.3	13.1	11.9	10.8	9.7	11.3	10.3	9.3	8.2	7.2		1.86
11	17.1	14.7	12.3	11.5	13.0	11.9	10.8	9.8	8.8	10.3	9.4	8.4	7.5	6.6		2.25
12	15.7	13.5	11.3	10.5	11.9	10.9	9.9	9.0	8.1	9.4	8.6	7.7	6.9	6.0		2.68
13	14.5	12.4	10.4	9.7	11.0	10.1	9.2	8.3	7.5	8.7	7.9	7.1	6.3	5.6		3.15
14	13.4	11.5	9.7	9.0	10.2	9.4	8.5	7.7	6.9	8.1	7.4	6.6	5.9	5.2		3.65
15	12.5	10.8	9.0	8.4	9.5	8.7	8.0	7.2	6.5	7.6	6.9	6.2	5.5	4.8		4.19
16	11.8	10.1	8.4	7.9	8.9	8.2	7.5	6.7	6.1	7.1	6.4	5.8	5.2	4.5		4.77
17	11.1	9.5	8.0	7.4	8.4	7.7	7.0	6.3	5.7							5.38
18	10.4	9.0	7.5	7.0	7.9	7.3	6.6	6.0	5.4							6.03
19	9.9	8.5	7.1	6.6												6.72
20	9.4	8.1	6.8	6.3												7.45

Span in Feet	Depth and Weight of Sections											Coefficient of Deflection		
	6 Inch				5 Inch			4 Inch			3 Inch			
	15.5 lbs.	13 lbs.	10.5 lbs.	8.2 lbs.	11.5 lbs.	9 lbs.	6.7 lbs.	7.25 lbs.	6.25 lbs.	5.4 lbs.	6 lbs.		5 lbs.	4.1 lbs.
	80.5				66.6			30.7			25.6	18.6		
1	77.8	62.9	45.2	28.8	49.7	39.0	22.8	27.3	23.7	17.3	16.4	14.7	12.2	
2	38.9	34.5	30.1	26.0	24.8	21.2	17.8	13.6	12.5	11.4	8.2	7.3	6.5	
3	25.9	23.0	20.1	17.3	16.6	14.1	11.9	9.1	8.3	7.6	5.5	4.9	4.4	
4	19.5	17.3	15.1	13.0	12.4	10.6	8.9	6.8	6.2	5.7	4.1	3.7	3.3	
5	15.6	13.8	12.0	10.4	9.9	8.5	7.1	5.5	5.0	4.6	3.3	2.9	2.6	
6	13.0	11.5	10.0	8.7	8.3	7.1	5.9	4.5	4.2	3.8	2.7	2.4	2.2	
7	11.1	9.9	8.6	7.4	7.1	6.0	5.1	3.9	3.6	3.3	2.3	2.1	1.9	
8	9.7	8.6	7.5	6.5	6.2	5.3	4.4	3.4	3.1	2.8	2.1	1.8	1.6	
9	8.6	7.7	6.7	5.8	5.5	4.7	4.0	3.0	2.8	2.5				
10	7.8	6.9	6.0	5.2	5.0	4.2	3.6	2.7	2.5	2.3				
11	7.1	6.3	5.5	4.7	4.5	3.8	3.2							
12	6.5	5.8	5.0	4.3	4.1	3.5	3.0							
13	6.0	5.3	4.6	4.0										
14	5.6	4.9	4.3	3.7										

Loads above upper horizontal lines will produce maximum allowable shear in webs.
 Loads below lower horizontal lines will produce excessive deflections.
 For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

BEAM SAFE LOADS—FIBER STRESS 18,000

MISCELLANEOUS BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS
 Applicable only when sections are braced against lateral deflection
 For unbraced sections safe loads must be reduced, see page 101.
 Maximum Bending Stress, 18,000 Pounds per Square Inch

H BEAMS

Span in Feet	Depth and Weight of Sections								Coefficients of Deflection
	8 Inch			6 Inch			5 Inch	4 Inch	
	37.7 lb.	34.3 lb.	32.6 lb.	26.7 lb.	24.1 lb.	22.8 lb.	18.9 lb.	13.8 lb.	
3	96.0			63.0				30.6	
4	90.6	72.0		47.4	45.0	36.0	37.5	21.4	0.17
5	72.5	69.3		37.9	36.1	35.2	28.5	16.0	0.30
6	60.4	57.7	60.0	31.6	30.1	29.3	22.8	12.8	0.47
7	51.8	49.5	56.4	27.1	25.8	25.1	19.0	10.7	0.67
8	45.3	43.3	48.3	23.7	23.7	22.6	16.3	9.2	0.91
9	40.3	38.5	42.3	21.1	20.1	20.1	14.3	8.0	1.19
10	36.2	34.6	37.6	21.1	20.1	19.6	12.7	7.1	1.51
11	32.9	31.5	33.8	19.0	18.1	17.6	11.4	6.4	1.86
12	30.2	28.9	30.8	17.2	16.4	16.0	10.4		2.25
13	27.9	26.6	28.2	15.8	15.0	14.7	9.5		2.68
14	25.9	24.7	26.0	14.6	13.9	13.5			3.15
15	24.2	23.1	24.2	13.5	12.9	12.6			3.65
16	22.6	21.6	22.6						4.77
17	21.3	20.4	21.2						5.38
18	20.1	19.2	19.9						6.03

CROSS TIE SECTIONS

Span in Feet	Depth and Weight of Sections				*Coefficients of Deflection
	5.5 Inch 24.0 Pounds	5.5 Inch 20.0 Pounds	4.25 Inch 14.5 Pounds	3 Inch 9.4 Pounds	
3	49.5		25.5	14.6	
4	45.4	33.0	22.0	10.0	0.17
5	34.0	29.2	16.5	7.5	0.30
6	27.2	23.4	13.2	6.0	0.47
7	22.7	19.5	11.0	5.0	0.67
8	19.4	16.7	9.4	4.3	0.91
9	17.0	14.6	8.3	3.8	1.19
10	15.1	13.0	7.3	3.3	1.51
11	13.6	11.7	6.6	3.0	1.86
12	12.4	10.6	6.0		2.25
13	11.3	9.7			2.68
14	10.5	9.0			3.15
15	9.7	8.3			3.65

Loads above upper horizontal lines will produce maximum allowable shear in webs.

Loads below lower horizontal lines will produce excessive deflections.

*To obtain deflections divide Coefficient by twice the distance of neutral axis from extreme fiber obtained from table from Elements of Cross Tie Sections.

CARNEGIE STEEL COMPANY

UNEQUAL ANGLES

Neutral Axis Parallel to Shorter Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 18,000 Pounds per Square Inch

Size, Inches	Thick- ness, Inches	1 Foot Span			Size, Inches	Thick- ness, Inches	1 Foot Span			
		Safe Load	Safe Load	Length, Feet			Safe Load	Safe Load	Length, Feet	
8 x 6	1	181.32	9.47	19.2	6 x 3 1/2	1	93.96	7.02	13.4	
	15/16	171.24	8.91	19.2		15/16	88.92	6.61	13.5	
	7/8	160.92	8.34	19.3		7/8	83.76	6.19	13.5	
	13/16	150.60	7.77	19.4		13/16	78.60	5.78	13.6	
	3/4	140.04	7.19	19.5		3/4	73.20	5.35	13.7	
	11/16	129.24	6.61	19.5		11/16	67.80	4.92	13.8	
	5/8	118.44	6.03	19.6		5/8	62.28	4.49	13.9	
	9/16	107.40	5.45	19.7		9/16	56.64	4.07	13.9	
	1/2	96.24	4.86	19.8		1/2	50.88	3.63	14.0	
	7/16	84.84	4.27	19.9		7/16	45.00	3.19	14.1	
8 x 3 1/2	1	164.28	9.50	17.3	5 x 4	3/8	39.00	2.75	14.2	
	15/16	155.28	8.93	17.4		5/16	32.88	2.30	14.3	
	7/8	146.16	8.37	17.5		5 x 3 1/2	7/8	59.88	5.08	11.8
	13/16	136.80	7.80	17.5			13/16	56.28	4.74	11.9
	3/4	127.32	7.22	17.7			3/4	52.44	4.39	12.0
	11/16	117.60	6.64	17.7			11/16	48.60	4.04	12.0
	5/8	107.76	6.06	17.8			5/8	44.76	3.70	12.1
	9/16	97.80	5.47	17.9			9/16	40.68	3.34	12.2
	1/2	87.72	4.88	18.0			1/2	36.60	2.98	12.3
	7/16	77.40	4.28	18.1			7/16	32.40	2.62	12.4
7 x 3 1/2	1	126.96	8.25	15.4	3/8		28.08	2.26	12.4	
	15/16	120.00	7.78	15.4	5 x 3		7/8	58.56	5.10	11.5
	7/8	113.04	7.28	15.5		13/16	54.96	4.75	11.6	
	13/16	105.84	6.78	15.6		3/4	51.36	4.41	11.6	
	3/4	98.64	6.29	15.7		11/16	47.64	4.06	11.7	
	11/16	91.20	5.79	15.8		5/8	43.80	3.71	11.8	
	5/8	83.64	5.28	15.9		9/16	39.84	3.35	11.9	
	9/16	75.96	4.77	15.9		1/2	35.88	3.00	12.0	
	1/2	68.16	4.26	16.0		1/2	31.68	2.63	12.1	
	7/16	60.12	3.73	16.1		5/8	27.48	2.26	12.1	
3/8	51.96	3.21	16.2	9/16		23.28	1.91	12.2		
6 x 4	1	96.24	7.02	13.7	5 x 3	13/16	53.40	4.75	11.2	
	15/16	91.08	6.59	13.8		3/4	49.92	4.41	11.3	
	7/8	85.80	6.18	13.9		11/16	46.32	4.07	11.4	
	13/16	80.40	5.76	14.0		5/8	42.60	3.72	11.5	
	3/4	75.00	5.35	14.0		9/16	38.76	3.35	11.6	
	11/16	69.36	4.92	14.1		1/2	34.92	3.00	11.6	
	5/8	63.72	4.49	14.2		7/16	30.96	2.64	11.7	
	9/16	57.96	4.06	14.3		3/8	26.88	2.28	11.8	
	1/2	52.08	3.63	14.4		9/16	22.68	1.91	11.9	
	7/16	45.96	3.18	14.5						
3/8	39.84	2.74	14.5							

BEAM SAFE LOADS—FIBER STRESS 18,000

UNEQUAL ANGLES

Neutral Axis Parallel to Longer Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 18,000 Pounds per Square Inch

Size, Inches	Thick-ness, Inches	1 Foot Span		Maximum Span 360 x Deflection		Size, Inches	Thick-ness, Inches	1 Foot Span		Maximum Span 360 x Deflection	
		Safe Load	Safe Load	Length, Feet	Safe Load			Safe Load	Length, Feet		
8	x6	1	107.04	6.87	15.6	6	x3½	1	34.80	3.91	8.9
		1½	101.16	6.47	15.6			1½	32.88	3.66	9.0
		¾	95.28	6.06	15.7			¾	31.08	3.43	9.1
		1¾	89.28	5.65	15.8			1¾	29.16	3.19	9.1
		¾	83.16	5.23	15.9			¾	27.24	2.96	9.2
		1½	76.92	4.82	16.0			1½	25.32	2.72	9.3
		5/8	70.56	4.40	16.0			5/8	23.28	2.48	9.4
		9/16	64.08	3.98	16.1			9/16	21.24	2.25	9.5
		½	57.48	3.55	16.2			½	19.08	2.00	9.6
		7/16	50.76	3.12	16.3			7/16	16.92	1.76	9.6
8	x3½	1	36.24	3.92	9.2	5	x4	¾	14.76	1.51	9.7
		1½	34.20	3.67	9.3			5/16	12.48	1.27	9.8
		¾	32.28	3.43	9.4			¾	39.72	3.98	10.0
		1¾	30.24	3.19	9.5			1¾	37.32	3.70	10.1
		¾	28.20	2.94	9.6			¾	34.80	3.42	10.2
		1½	26.04	2.69	9.7			1½	32.28	3.15	10.2
		5/8	24.00	2.46	9.7			5/8	29.76	2.89	10.3
		9/16	21.84	2.22	9.9			9/16	27.12	2.61	10.4
		½	19.68	1.98	9.9			½	24.48	2.33	10.5
		7/16	17.52	1.75	10.0			7/16	21.72	2.06	10.6
7	x3½	1	35.52	3.91	9.1	5	x3½	¾	18.84	1.77	10.6
		1½	33.60	3.67	9.2			¾	30.24	3.43	8.8
		¾	31.68	3.42	9.3			1¾	28.44	3.20	8.9
		1¾	29.76	3.19	9.3			¾	26.64	2.98	9.0
		¾	27.72	2.94	9.4			1½	24.72	2.73	9.1
		1½	25.68	2.71	9.5			5/8	22.80	2.50	9.1
		5/8	23.64	2.47	9.6			9/16	20.76	2.26	9.2
		9/16	21.60	2.23	9.7			9/16	18.72	2.02	9.3
		½	19.44	2.00	9.7			½	16.68	1.78	9.4
		7/16	17.28	1.75	9.9			¾	14.52	1.54	9.5
6	x4	¾	15.12	1.52	9.9	5	x3	5/16	12.24	1.29	9.5
		1	45.48	4.49	10.1			1¾	20.88	2.73	7.7
		1½	43.08	4.21	10.2			¾	19.56	2.53	7.7
		¾	40.68	3.95	10.3			1½	18.12	2.32	7.8
		1¾	38.16	3.68	10.4			5/8	16.68	2.12	7.9
		¾	35.64	3.41	10.5			9/16	15.24	1.91	8.0
		1½	33.12	3.14	10.5			½	13.80	1.71	8.1
		5/8	30.48	2.87	10.6			7/16	12.24	1.51	8.1
		9/16	27.72	2.59	10.7			¾	10.68	1.30	8.2
		½	24.96	2.32	10.8			5/16	9.00	1.08	8.3
7/16	22.20	2.04	10.9								
5/8	19.20	1.75	11.0								

UNEQUAL ANGLES

Neutral Axis Parallel to Shorter Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 18,000 Pounds per Square Inch

Size, Inches	Thick- ness, Inches	1 Foot Span			Maximum Span 360 x Deflection			Size, Inches	Thick- ness, Inches	1 Foot Span			Maximum Span 360 x Deflection			
		Safe Load	Safe Load	Length, Feet	Safe Load	Safe Load	Length, Feet			Safe Load	Safe Load	Length, Feet	Safe Load	Safe Load	Length, Feet	
4 1/2 x 3	1 3/16	43.44	4.26	10.2	3	x 2 1/2	9/16	13.80	1.95	7.1	3	x 2	1 1/2	12.00	1.75	6.9
	3/4	40.56	3.95	10.3			1 1/2	12.48	1.74	7.2						
	1 1/16	37.68	3.63	10.4			7/16	11.16	1.54	7.2						
	5/8	34.68	3.32	10.5			5/8	9.72	1.33	7.3						
	9/16	31.68	3.01	10.5			5/16	8.28	1.12	7.4						
	1/2	28.44	2.68	10.6			1/4	6.72	0.90	7.5						
	7/16	25.20	2.36	10.7			3	x 2	1 1/2	12.00			1.75	6.9		
	5/8	21.96	2.04	10.8					7/16	10.68			1.54	6.9		
	9/16	18.48	1.70	10.9					5/8	9.36			1.33	7.0		
	4 x 3 1/2	1 3/16	35.04	3.71					9.5	5/8			7.92	1.12	7.1	
3/4		33.00	3.47	9.5	7/16	7.92			1.12	7.1						
1 1/16		30.72	3.20	9.6	1/4	6.48			0.90	7.2						
5/8		28.20	2.91	9.7	2 1/2 x 2	1 1/2	8.40	1.45	5.8							
9/16		25.80	2.64	9.8		7/16	7.56	1.28	5.9							
1/2		23.16	2.35	9.9		5/8	6.60	1.10	6.0							
7/16		20.64	2.08	9.9		5/16	5.64	0.93	6.1							
3/8		18.00	1.80	10.0		1/4	4.56	0.75	6.1							
9/16		15.12	1.50	10.1		5/16	3.48	0.56	6.2							
4 x 3		1 3/16	34.44	3.76		9.2	1/8	2.40	0.38	6.3						
	3/4	32.16	3.48	9.2		2 1/2 x 1 1/2	5/16	5.28	0.92	5.7						
	1 1/16	29.88	3.20	9.3			1/4	4.32	0.75	5.8						
	5/8	27.60	2.93	9.4			5/16	3.36	0.57	5.9						
	9/16	25.20	2.66	9.5	2 1/4 x 1 1/2		1 1/2	6.48	1.30	5.0						
	1/2	22.68	2.37	9.6			7/16	5.76	1.13	5.1						
	7/16	20.16	2.08	9.7			5/8	5.04	0.98	5.2						
	5/8	17.52	1.80	9.7			5/16	4.32	0.83	5.2						
	9/16	14.76	1.51	9.8			1/4	3.60	0.68	5.3						
	1/4	12.00	1.22	9.9			5/16	2.76	0.51	5.4						
3 1/2 x 3	1 3/16	26.40	3.25	8.1			2	x 1 1/2	5/8	4.08	0.88	4.6				
	3/4	24.60	3.00	8.2		5/16			3.48	0.74	4.7					
	1 1/16	22.92	2.77	8.3		1/4			2.88	0.60	4.8					
	5/8	21.12	2.53	8.3		5/16			2.16	0.44	4.9					
	9/16	19.32	2.30	8.4	1/8	1.50			0.30	4.9						
	1/2	17.40	2.05	8.5	2	x 1 1/4			1/4	2.75	0.60	4.6				
	7/16	15.48	1.80	8.6					5/16	2.12	0.45	4.7				
	5/8	13.56	1.57	8.7					3 1/2 x 2 1/2	1 1/4	2.11	0.51	4.1			
	9/16	11.52	1.32	8.7						5/16	1.64	0.39	4.2			
	1/4	9.36	1.06	8.8						1/8	1.13	0.27	4.3			
3 1/2 x 2 1/2	1 1/16	22.20	2.78	8.0			1 1/2 x 1 1/4	5/16		1.92	0.55	3.5				
	5/8	20.52	2.55	8.1				1/4		1.56	0.44	3.6				
	9/16	18.72	2.30	8.1				5/16		1.21	0.33	3.7				
	1/2	16.92	2.06	8.2				1 1/2 x 1 1/4		5/16	1.92	0.55	3.5			
	7/16	15.12	1.82	8.3						1/4	1.56	0.44	3.6			
	5/8	13.20	1.57	8.4	5/16	1.21				0.33	3.7					
	9/16	11.16	1.32	8.4	1 1/2 x 1 1/4	5/16				1.92	0.55	3.5				
	1/4	9.00	1.05	8.6		1/4			1.56	0.44	3.6					

BEAM SAFE LOADS—FIBER STRESS 18,000

UNEQUAL ANGLES

Neutral Axis Parallel to Longer Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection.

Maximum Bending Stress, 18,000 Pounds per Square Inch

Size, Inches	Thick-ness, Inches	1 Foot Span			Maximum Span 360 x Deflection					
		Safe Load	Safe Load	Length, Feet	Safe Load	Safe Load	Length, Feet			
4 1/2 x 3	1 9/16	20.52	2.73	7.5	3 x 2 1/2	9/16	9.84	1.59	6.2	
	8/16	19.20	2.53	7.6		1/2	8.88	1.42	6.3	
	1 1/16	17.88	2.32	7.7		7/16	7.92	1.25	6.3	
	5/8	16.44	2.11	7.8		3/8	6.96	1.08	6.4	
	9/16	15.00	1.90	7.8		5/16	5.88	0.90	6.5	
	1/2	13.56	1.71	7.9		1/4	4.80	0.73	6.6	
	7/16	12.12	1.51	8.0		3 x 2	1/2	5.64	1.11	5.1
	3/8	10.56	1.31	8.1			7/16	5.04	0.98	5.2
	9/16	9.00	1.10	8.2			3/8	4.44	0.85	5.2
	4 x 3 1/2	1 9/16	27.60	3.22			8.6	5/16	3.84	0.72
8/16		25.80	2.99	8.6	1/4		3.00	0.58	5.4	
1 1/16		24.00	2.76	8.7	2 1/2 x 2	1/2	5.52	1.12	4.9	
5/8		22.08	2.51	8.8		7/16	4.92	0.98	5.0	
9/16		20.16	2.27	8.9		3/8	4.32	0.85	5.1	
1/2		18.24	2.04	9.0		5/16	3.72	0.72	5.2	
7/16		16.20	1.80	9.0		1/4	3.00	0.58	5.2	
4 x 3		3/8	14.16	1.56	9.1	3/8	2.40	0.45	5.3	
		9/16	12.00	1.31	9.2	1/2	1.62	0.30	5.4	
		1 9/16	20.16	2.74	7.4	2 1/2 x 1 1/2	5/16	2.04	0.52	3.9
	8/16	18.84	2.53	7.4	1/4		1.68	0.41	4.1	
	1 1/16	17.52	2.32	7.6	3/16		1.32	0.32	4.1	
	5/8	16.20	2.12	7.6	2 1/4 x 1 1/2		1/2	3.12	0.85	3.7
	9/16	14.76	1.92	7.7			7/16	2.76	0.74	3.7
	1/2	13.32	1.71	7.8		3/8	2.40	0.63	3.8	
	7/16	11.88	1.51	7.9		5/16	2.04	0.53	3.9	
	3/8	10.44	1.31	7.9		1/4	1.68	0.42	4.0	
3 1/2 x 3	9/16	8.88	1.11	8.0	3/16	1.32	0.33	4.0		
	1/4	7.20	.89	8.1	2 x 1 1/2	3/8	2.40	0.65	3.7	
	1 9/16	19.80	2.74	7.2		5/16	2.04	0.54	3.8	
	8/16	18.48	2.53	7.3		1/4	1.68	0.43	3.9	
	1 1/16	17.28	2.34	7.4		3/16	1.32	0.33	4.0	
	5/8	15.96	2.14	7.4		1/2	0.90	0.22	4.0	
	3 1/2 x 2 1/2	9/16	14.52	1.93	7.5	2 x 1 1/4	3/8	1.68	0.43	3.9
		1/2	13.20	1.74	7.6		5/16	1.32	0.33	4.0
		7/16	11.76	1.53	7.7		1/4	0.90	0.22	4.0
		3/8	10.20	1.31	7.8		2 x 1 1/2	1/2	1.16	0.35
9/16		8.64	1.10	7.8	3/8			0.90	0.27	3.4
1/4		6.96	0.88	7.9	1 3/4 x 1 1/4	1/4		1.14	0.35	3.2
1 1/16		11.88	1.92	6.2		3/16		0.90	0.27	3.3
5/8		11.04	1.76	6.3		1/8		0.62	0.19	3.4
9/16		10.08	1.59	6.3		1 1/2 x 1 1/4	3/16	1.36	0.45	3.0
1/2		9.12	1.42	6.4			1/4	1.12	0.36	3.1
7/16	8.16	1.25	6.5	3/8	0.88		0.27	3.2		
3/8	7.08	1.07	6.6	1 1/2 x 1 1/4	5/16		1.36	0.45	3.0	
9/16	6.00	0.90	6.7		1/4		1.12	0.36	3.1	
1/4	4.92	0.73	6.8	3/8	0.88	0.27	3.2			

CARNEGIE STEEL COMPANY

EQUAL ANGLES

Neutral Axis Parallel to Either Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 18,000 Pounds per Square Inch

Size, Inches	Thick- ness, Inches	1 Foot Span		Maximum Span 360 x Deflection		Size, Inches	Thick- ness, Inches	1 Foot Span		Maximum Span 360 x Deflection			
		Safe Load	Safe Load	Length, Feet	Safe Load			Safe Load	Length, Feet				
8	x8	1 1/8	210.36	10.51	20.0	3 1/2 x 3 1/2	1 1/16	27.00	3.24	8.3			
		1 1/16	200.04	9.96	20.1		3/4	25.32	3.01	8.4			
		1	189.60	9.40	20.2		1 1/16	23.52	2.76	8.5			
		15/16	179.04	8.84	20.2		5/8	21.72	2.53	8.6			
		7/8	168.24	8.28	20.3		9/16	19.80	2.29	8.7			
		15/16	157.32	7.71	20.4		1/2	17.88	2.05	8.7			
		3/4	146.28	7.14	20.5		7/16	15.84	1.80	8.8			
		11/16	135.00	6.56	20.6		5/8	13.80	1.55	8.9			
		5/8	123.60	5.98	20.7		5/16	11.76	1.31	9.0			
		9/16	112.08	5.41	20.7		1/4	9.48	1.05	9.1			
		1/2	100.44	4.83	20.8		5/16	15.60	2.16	7.2			
		6	x6	1	102.84		6.94	14.8	3 x 3	1/16	14.28	1.95	7.3
				15/16	97.32		6.54	14.9		1/2	12.84	1.73	7.4
				7/8	91.56		6.12	15.0		7/16	11.40	1.52	7.5
15/16	85.80			5.70	15.0	5/8	9.96	1.32		7.6			
3/4	79.92			5.29	15.1	9/16	8.52	1.12		7.6			
11/16	74.04			4.87	15.2	1/4	6.96	0.90		7.7			
5/8	67.92			4.44	15.3	1/2	8.76	1.45		6.1			
9/16	61.68			4.02	15.4	7/16	7.80	1.27		6.2			
1/2	55.32			3.58	15.5	5/8	6.84	1.10		6.2			
7/16	48.84			3.14	15.5	9/16	5.76	0.92		6.3			
5/8	42.36			2.71	15.6	1/4	4.68	0.74		6.4			
5	x5			1	69.60	5.73	12.1	2 1/2 x 2 1/2		5/16	3.60	0.55	6.5
				15/16	65.88	5.39	12.2			1/8	2.40	0.37	6.6
				7/8	62.04	5.05	12.3			7/16	4.80	1.00	4.8
		15/16	58.20	4.71	12.4	5/8	4.20		.86	4.9			
		3/4	54.36	4.36	12.5	9/16	3.60		.72	5.0			
		11/16	50.40	4.02	12.5	1/4	3.00		.59	5.1			
		5/8	46.32	3.68	12.6	7/16	2.28		.45	5.1			
		9/16	42.12	3.32	12.7	1/8	1.56		.30	5.2			
		1/2	37.80	2.96	12.8	7/16	3.60		.87	4.2			
		7/16	33.48	2.61	12.9	5/8	3.12		.74	4.2			
		5/8	29.04	2.25	12.9	9/16	2.76		.64	4.3			
		4	x4	15/16	36.12	3.72	9.7		1 3/4 x 1 3/4	1/4	2.28	.52	4.4
				3/4	33.72	3.45	9.8			5/16	1.68	.38	4.4
				11/16	31.32	3.18	9.8			1/8	1.20	.26	4.6
5/8	28.80			2.91	9.9	5/8	2.28	.64		3.5			
9/16	26.28			2.63	10.0	9/16	1.94	.54		3.6			
1/2	23.64			2.34	10.1	1/4	1.61	.44		3.7			
7/16	21.00			2.06	10.2	5/16	1.25	.33		3.8			
5/8	18.24			1.78	10.2	1/8	0.86	.22		3.9			
9/16	15.48			1.50	10.3	5/16	1.31	.44		3.0			
1/4	12.60			1.21	10.4	1/4	1.09	.36		3.0			
1	x1			15/16	15.48	1.50	10.3	1 1/2 x 1 1/2		5/16	0.85	.27	3.1
				3/4	12.60	1.21	10.4			1/8	0.59	.18	3.2
				5/8	18.24	1.78	10.2			1/4	0.67	.28	2.4
				9/16	15.48	1.50	10.3			5/16	0.53	.22	2.4
1/4	12.60	1.21	10.4	1/8	0.37	.15	2.5						

BEAM SAFE LOADS—FIBER STRESS 18,000

TEES AND ZEES

Neutral Axis Parallel to Flanges

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection.

Maximum Bending Stress, 18,000 Pounds per Square Inch

TEES

Size Flange x Stem, Inches	Weight per Foot, Pounds	1 Foot Span			Maximum Span 360 x Deflection				
		Safe Load	Safe Load	Length, Feet	Size Flange x Stem, Inches	Weight per Foot, Pounds	1 Foot Span	Maximum Span 360 x Deflection	
6 1/2 x 6 1/2	19.8	59.40	3.50	17.0	5 x 3	11.5	12.72	1.59	8.0
4 x 4	13.5	24.24	2.40	10.1	4 x 5	15.3	37.56	3.05	12.3
4 x 4	10.5	18.96	1.85	10.3	4 x 5	11.9	29.16	2.33	12.5
3 x 3	7.8	10.32	1.36	7.6	4 x 4 1/2	14.4	30.48	2.72	11.2
3 x 3	6.7	8.88	1.16	7.7	4 x 4 1/2	11.2	23.76	2.08	11.4
2 1/2 x 2 1/2	6.4	7.08	1.14	6.2	4 x 3	9.2	10.80	1.36	7.9
2 1/2 x 2 1/2	5.5	6.00	0.95	6.3	4 x 3	7.8	9.24	1.15	8.1
2 1/4 x 2 1/4	4.9	4.92	0.88	5.6	4 x 2 1/2	8.5	7.44	1.11	6.7
2 1/4 x 2 1/4	4.1	3.84	0.67	5.7	4 x 2 1/2	7.2	6.36	0.94	6.8
2 x 2	4.3	3.72	0.75	5.0	3 x 2 1/2	6.1	6.24	0.96	6.5
2 x 2	3.56	3.12	0.62	5.0	2 1/2 x 3	6.1	8.64	1.16	7.4
					1 1/2 x 2	2.45	2.34	0.48	4.9
					1 1/2 x 1 1/4	1.25	0.64	0.19	3.3
					1 1/4 x 3/4	0.88	0.17	0.10	1.7

ZEES

Size Depth x Flange, Inches	Thick-ness, Inches	1 Foot Span			Maximum Span 360 x Deflection				
		Safe Load	Safe Load	Length, Feet	Size Depth x Flange, Inches	Thick-ness, Inches	1 Foot Span	Maximum Span 360 x Deflection	
6 1/8 x 3 3/8	7/8	196.80	17.95	11.0	4 1/8 x 3 3/8	3/4	87.12	11.80	7.4
6 1/16 x 3 3/16	1 1/16	182.64	16.83	10.9	4 1/16 x 3 1/16	1 1/16	79.80	10.97	7.3
6 x 3 1/2	3/4	168.48	15.68	10.7	4 x 3 1/2	3/8	72.60	10.14	7.2
6 1/8 x 3 3/8	1 1/16	169.20	15.43	11.0	4 1/8 x 3 3/8	3/8	74.16	10.04	7.4
6 1/16 x 3 3/16	3/8	153.84	14.18	10.9	4 1/16 x 3 1/16	1/2	66.00	9.08	7.3
6 x 3 1/2	3/8	138.60	12.91	10.7	4 x 3 1/2	3/8	57.96	8.10	7.2
6 1/8 x 3 3/8	1/2	134.64	12.28	11.0	4 1/8 x 3 3/8	3/8	56.04	7.59	7.4
6 1/16 x 3 3/16	7/16	117.96	10.87	10.9	4 1/16 x 3 1/16	3/8	46.92	6.45	7.3
6 x 3 1/2	3/8	101.28	9.43	10.7	4 x 3 1/2	1/4	37.68	5.26	7.2
5 1/8 x 3 3/8	1 1/16	134.40	14.65	9.2					
5 1/16 x 3 3/16	3/4	124.08	13.69	9.1					
5 x 3 1/4	1 1/16	113.64	12.70	9.0	3 1/16 x 2 3/16	3/8	41.16	7.51	5.5
5 1/8 x 3 3/8	3/8	114.84	12.52	9.2	3 x 2 1/16	1/2	36.72	6.84	5.4
5 1/16 x 3 3/16	3/8	103.44	11.42	9.1	3 1/16 x 2 3/16	3/8	35.76	6.52	5.5
5 x 3 1/4	1/2	92.16	10.30	9.0	3 x 2 1/16	3/8	30.84	5.74	5.4
5 1/8 x 3 3/8	7/16	89.28	9.73	9.2	3 1/16 x 2 3/16	3/8	28.56	5.21	5.5
5 1/16 x 3 3/16	3/8	76.68	8.46	9.1	3 x 2 1/16	1/4	23.04	4.29	5.4
5 x 3 1/4	3/8	64.08	7.16	9.0					

CARNEGIE STEEL COMPANY

BEAMS

MAXIMUM BENDING MOMENTS AND WEB RESISTANCES

Bending Stress 16,000 Pounds—Shearing Stress 10,000 Pounds

Depth of Beam	Weight per Foot	Thickness of Web	Maximum Bending Moment	Web Resistance			Minimum End Bearing	End Reaction, $a=3\frac{1}{2}'$
				Web Shear	Minimum Span	Web Buckling		
d		t	M _{max}	V		f _b	a	R
Inches	Pounds	Inches	Foot Pounds	Pounds	Feet	Pounds per Sq. In.	Inches	Pounds
24	120.0	.798	334530	191520	6.99	13790	11.40	104560
	115.0	.737	326730	176880	7.39	13360	11.96	93550
	110.0	.675	318790	162000	7.87	12840	12.69	82350
	105.9	.625	312390	150000	8.33	12350	13.44	73320
24	100.0	.747	263530	179280	5.88	13430	11.87	95330
	95.0	.686	255720	164640	6.21	12940	12.55	84320
	90.0	.624	247790	149760	6.62	12340	13.45	73130
	85.0	.563	239980	135120	7.10	11620	14.66	62120
	79.9	.500	231920	120000	7.73	10680	16.46	50750
20	100.0	.873	219780	174600	5.04	15030	8.31	111550
	95.0	.800	213290	160000	5.33	14670	8.63	99750
	90.0	.726	206710	145200	5.69	14230	9.06	87810
	85.0	.653	200220	130600	6.13	13700	9.60	76010
	81.4	.600	195510	120000	6.52	13230	10.12	67450
20	75.0	.641	168470	128200	5.26	13590	9.71	74070
	70.0	.567	161890	113400	5.71	12890	10.52	62130
	65.4	.500	155930	100000	6.24	12070	11.57	51300
18	90.0	.796	186140	143280	5.20	15090	7.43	96080
	85.0	.714	180240	128520	5.61	14630	7.80	83580
	80.0	.632	174340	113760	6.13	14070	8.29	71140
	75.6	.560	169150	100800	6.71	13440	8.89	60210
18	70.0	.711	135930	127980	4.25	14620	7.81	83160
	65.0	.629	130030	113220	4.59	14050	8.31	70690
	60.0	.547	124120	98460	5.04	13310	9.03	58230
	54.7	.460	117860	82800	5.69	12230	10.22	45010
15	75.0	.868	122170	130200	3.75	16010	5.62	100730
	70.0	.770	117270	115500	4.06	15630	5.85	87230
	65.0	.672	112370	100800	4.46	15130	6.16	73730
	60.8	.590	108270	88500	4.89	14600	6.53	62430
15	55.0	.648	90430	97200	3.72	14990	6.26	70430
	50.0	.550	85530	82500	4.15	14280	6.76	56930
	45.0	.452	80620	67800	4.76	13250	7.57	43430
	42.9	.410	78530	61500	5.11	12670	8.09	37650

BEAM SAFE LOADS—FIBER STRESS 16,000

BEAMS

MAXIMUM BENDING MOMENTS AND WEB RESISTANCES

Bending Stress 16,000 Pounds—Shearing Stress 10,000 Pounds

Depth of Beam	Weight per Foot	Thickness of Web	Maximum Bending Moment	Web Resistance			Minimum End Bearing	End Reaction, $a=3\frac{1}{2}''$
				Web Shear	Minimum Span	Web Buckling		
d		t	M _{max}	V		f _b	a	R
Inches	Pounds	Inches	Foot Pounds	Pounds	Feet	Pounds per Sq. In.	Inches	Pounds
12	55.0	.810	70970	97200	2.92	16430	4.30	86530
	50.0	.687	67030	82440	3.25	15970	4.51	71330
	45.0	.565	63130	67800	3.72	15320	4.83	56270
	40.8	.460	59770	55200	4.33	14480	5.29	43300
12	35.0	.428	50460	51360	3.93	14150	5.48	39350
	31.8	.350	47960	42000	4.57	13060	6.19	29710
10	40.0	.741	42140	74100	2.27	16660	3.50	74080
	35.0	.594	38870	59400	2.62	16090	3.72	57330
	30.0	.447	35600	44700	3.19	15120	4.11	40560
	25.4	.310	32560	31000	4.20	13410	4.96	24950
9	35.0	.724	32970	65160	2.02	16850	3.09	70130
	30.0	.561	30040	50490	2.38	16220	3.30	52320
	25.0	.397	27090	35730	3.03	15070	3.72	34410
	21.8	.290	25160	26100	3.86	13620	4.36	22720
8	25.5	.532	22680	42560	2.13	16400	2.88	47970
	23.0	.441	21390	35280	2.43	15860	3.05	38460
	20.5	.349	20080	27920	2.88	15030	3.32	28850
	18.4	.270	18960	21600	3.51	13870	3.77	20590
7	20.0	.450	15980	31500	2.03	16310	2.54	38520
	17.5	.345	14840	24150	2.46	15490	2.77	28050
	15.3	.250	13800	17500	3.15	14150	3.20	18570
6	17.25	.465	11560	27900	1.66	16770	2.08	38980
	14.75	.343	10590	20580	2.06	15970	2.26	27390
	12.50	.230	9680	13800	2.81	14480	2.64	16650
5	14.75	.494	8030	24700	1.30	17250	1.65	40470
	12.25	.347	7210	17350	1.66	16510	1.78	27200
	10.0	.210	6450	10500	2.46	14880	2.11	14840
4	10.5	.400	4720	16000	1.18	17270	1.32	31080
	9.5	.326	4460	13040	1.37	16870	1.37	24750
	8.5	.253	4200	10120	1.66	16260	1.46	18510
	7.7	.190	3980	7600	2.09	15350	1.61	13120
3	7.5	.349	2560	10470	0.98	17510	0.96	25970
	6.5	.251	2370	7530	1.26	16930	1.02	18060
	5.7	.170	2210	5100	1.73	15950	1.13	11520

CARNEGIE STEEL COMPANY

CHANNELS

MAXIMUM BENDING MOMENTS AND WEB RESISTANCES

Bending Stress 16,000 Pounds—Shearing Stress 10,000 Pounds

Depth of Channel	Weight per Foot	Thickness of Web	Maximum Bending Moment	Web Resistance			Minimum End Bearing	End Reaction, $a = 3\frac{1}{2}''$	
				Web Shear	Minimum Span	Web Buckling			
d		t	M _{max}	V		f _b	a	R	
Inches	Pounds	Inches	Foot Pounds	Pounds	Feet	Pounds per Sq. In.	Inches	Pounds	
15	55.0	.814	76270	122100	2.50	15810	5.74	93290	
	50.0	.716	71420	107400	2.66	15370	6.01	79790	
	45.0	.618	66470	92700	2.87	14800	6.39	66290	
	40.0	.520	61570	78000	3.16	14000	6.96	52790	
	35.0	.422	56670	63300	3.58	12840	7.93	39290	
	33.9	.400	55570	60000	3.70	12510	8.24	36270	
13	50.0	.787	64190	102310	2.51	16140	4.81	85730	
	45.0	.673	59910	87490	2.74	15660	5.05	71120	
	40.0	.560	55660	72800	3.06	14980	5.43	56620	
	37.0	.492	53110	63960	3.32	14420	5.76	47900	
	35.0	.447	51420	58110	3.54	13960	6.06	42120	
	31.8	.375	48720	48750	4.00	13000	6.75	32900	
12	40.0	.755	43670	90600	1.93	16250	4.39	79740	
	35.0	.632	39730	75840	2.10	15710	4.64	64540	
	30.0	.510	35830	61200	2.34	14920	5.04	49470	
	25.0	.387	31890	46440	2.75	13630	5.81	34280	
	20.7	.280	28470	33600	3.39	11570	7.37	21060	
	10	35.0	.820	30720	82000	1.50	16890	3.42	83090
30.0		.673	27460	67300	1.63	16430	3.59	66330	
25.0		.526	24190	52600	1.84	15710	3.87	49570	
20.0		.379	20920	37900	2.21	14430	4.43	32810	
15.3		.240	17830	24000	2.97	11790	5.98	16970	
9		25.0	.612	20900	55080	1.52	16450	3.22	57900
	20.0	.448	17950	40320	1.78	15520	3.55	39980	
	15.0	.285	15010	25650	2.34	13530	4.40	22180	
	13.4	.230	14020	20700	2.71	12220	5.11	16160	
	8	21.25	.579	15870	46320	1.37	16610	2.82	52880
		18.75	.487	14570	38960	1.50	16160	2.95	43270
16.25		.395	13260	31600	1.68	15490	3.16	33650	
13.75		.303	11950	24240	1.97	14430	3.55	24040	
11.5		.220	10770	17600	2.45	12700	4.30	15370	
7		19.75	.629	12590	44030	1.14	17070	2.35	56380
	17.25	.524	11450	36680	1.25	16690	2.44	45910	
	14.75	.419	10310	29330	1.41	16110	2.60	35430	
	12.25	.314	9170	21980	1.67	15140	2.87	24950	
	9.8	.210	8030	14700	2.19	13220	3.54	14580	
	6	15.5	.559	8650	33540	1.03	17140	2.00	47910
13.0		.437	7670	26220	1.17	16620	2.11	36320	
10.5		.314	6690	18840	1.42	15690	2.32	24630	
8.2		.200	5780	12000	1.93	13800	2.85	13800	
5		11.5	.472	5520	23600	0.94	17170	1.66	38490
		9.0	.325	4710	16250	1.16	16340	1.81	25220
	6.7	.190	3950	9500	1.67	14440	2.21	13030	
	4	7.25	.320	3030	12800	0.95	16840	1.38	24240
		6.25	.247	2770	9880	1.12	16200	1.47	18000
		5.4	.180	2530	7200	1.40	15150	1.64	12270
3		6.0	.356	1830	10680	0.68	17540	0.96	26540
		5.0	.258	1630	7740	0.84	16990	1.02	18630
		4.1	.170	1450	5100	1.14	15950	1.13	11520

BEAM SAFE LOADS—FIBER STRESS 16,000

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 16,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections									Coefficient of Deflection	
	24 Inch										
	120 lbs.	115 lbs.	110 lbs.	105.9 lbs.	100 lbs.	95 lbs.	90 lbs.	85 lbs.	79.9 lbs.		
6	383.0				358.6						0.60
7	382.3				351.4						0.81
8	334.5	353.8	324.0		301.2	329.2	299.5				1.06
9	297.4	290.4	283.4		263.5	255.7	247.8	270.2	240.0		1.34
10	267.6	261.4	255.0	300.0	277.7	234.2	227.3	220.3	213.3	206.1	1.66
11	243.3	237.6	231.8		227.2	191.7	186.0	180.2	174.5	168.7	2.00
12	223.0	217.8	212.5		208.3	175.7	170.5	165.2	160.0	154.6	2.38
13	205.9	201.1	196.2		192.2	162.2	157.4	152.5	147.7	142.7	2.80
14	191.2	186.7	182.2		178.5	150.6	146.1	141.6	137.1	132.5	3.24
15	178.4	174.3	170.0		166.6	140.5	136.4	132.2	128.0	123.7	3.72
16	167.3	163.4	159.4		156.2	131.8	127.9	123.9	120.0	116.0	4.24
17	157.4	153.8	150.0		147.0	124.0	120.3	116.6	112.9	109.1	4.78
18	148.7	145.2	141.7		138.8	117.1	113.7	110.1	106.7	103.1	5.36
19	140.9	137.6	134.2		131.5	111.0	107.7	104.3	101.0	97.6	5.98
20	133.8	130.7	127.5		125.0	105.4	102.3	99.1	96.0	92.8	6.62
21	127.4	124.5	121.4		119.0	100.4	97.4	94.4	91.4	88.3	7.30
22	121.6	118.8	115.9		113.6	95.8	93.0	90.1	87.3	84.3	8.01
23	116.4	113.6	110.9		108.7	91.7	88.9	86.2	83.5	80.7	8.76
24	111.5	108.9	106.3		104.1	87.8	85.2	82.6	80.0	77.3	9.53
25	107.1	104.6	102.0		100.0	84.3	81.8	79.3	76.8	74.2	10.35
26	102.9	100.5	98.1		96.1	81.1	78.7	76.2	73.8	71.4	11.19
27	99.1	96.8	94.5		92.6	78.1	75.8	73.4	71.1	68.7	12.07
28	95.6	93.4	91.1		89.2	75.3	73.1	70.8	68.6	66.3	12.98
29	92.3	90.1	87.9		86.2	72.7	70.5	68.4	66.2	64.0	13.92
30	89.2	87.1	85.0		83.3	70.3	68.2	66.1	64.0	61.8	14.90
31	86.3	84.3	82.3		80.6	68.0	66.0	63.9	61.9	59.8	15.91
32	83.6	81.7	79.7		78.1	65.9	63.9	62.0	60.0	58.0	16.95
33	81.1	79.2	77.3		75.7	63.9	62.0	60.1	58.2	56.2	18.03
34	78.7	76.9	75.0		73.5	62.0	60.2	58.3	56.5	54.6	19.13
35	76.5	74.7	72.9		71.4	60.2	58.5	56.6	54.9	53.0	20.28
36	74.3	72.6	70.8		69.4	58.6	56.8	55.1	53.3	51.5	21.45
37	72.3	70.6	68.9		67.5	57.0	55.3	53.6	51.9	50.1	22.66
38	70.4	68.8	67.1		65.8	55.5	53.8	52.2	50.5	48.8	23.90
39	68.6	67.0	65.4		64.1	54.1	52.5	50.8	49.2	47.6	25.18
40	66.9	65.3	63.8		62.5	52.7	51.1	49.6	48.0	46.4	26.48
41	65.3	63.8	62.2		61.0	51.4	49.9	48.4	46.8	45.3	27.82
42	63.7	62.2	60.7		59.5	50.2	48.7	47.2	45.7	44.2	29.20
43	62.2	60.8	59.3		58.1	49.0	47.6	46.1	44.6	43.1	30.60
44	60.8	59.4	58.0		56.8	47.9	46.5	45.1	43.6	42.2	32.04
45	59.5	58.1	56.7		55.5	46.8	45.5	44.1	42.7	41.2	33.52
46	58.2	56.8	55.4		54.3	45.8	44.5	43.1	41.7	40.3	35.02
47	56.9	55.6	54.3		53.2	44.9	43.5	42.2	40.8	39.5	36.56
48	55.8	54.5	53.1		52.1	43.9	42.6	41.3	40.0	38.7	38.14
49	54.6	53.3	52.1		51.0	43.0	41.8	40.5	39.2	37.9	39.74
50	53.5	52.3	51.0		50.0	42.2	40.9	39.6	38.4	37.1	41.38

Loads above upper horizontal lines will produce maximum allowable shear in webs.

Loads below lower horizontal lines will produce excessive deflections.

For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 16,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection
	20 Inch						18 Inch						
	100 lbs.	95 lbs.	90 lbs.	85 lbs.	81.4 lbs.	75 lbs.	70 lbs.	65.4 lbs.	90 lbs.	85 lbs.	80 lbs.	75.6 lbs.	
	849.2	820.0	290.4			256.4	236.8		286.6	287.0			
6	293.0	284.4	275.6	261.2	240.0	224.6	215.9	200.0	248.2	224.0	227.5	201.6	0.60
7	251.2	243.8	236.2	228.8	223.4	192.5	185.0	178.2	212.7	206.0	199.2	193.3	0.81
8	219.8	213.3	206.7	200.2	195.5	168.5	161.9	155.9	186.1	180.2	174.3	169.2	1.06
9	195.4	189.6	183.7	178.0	173.8	149.7	143.9	138.6	165.5	160.2	155.0	150.4	1.34
10	175.8	170.6	165.4	160.2	156.4	134.8	129.5	124.7	148.9	144.2	139.5	135.3	1.66
11	159.8	155.1	150.3	145.6	142.2	122.5	117.7	113.4	135.4	131.1	126.8	123.0	2.00
12	146.5	142.2	137.8	133.5	130.3	112.3	107.9	104.0	124.1	120.2	116.2	112.8	2.38
13	135.2	131.2	127.2	123.2	120.3	103.7	99.6	96.0	114.5	110.9	107.3	104.1	2.80
14	125.6	121.9	118.1	114.4	111.7	96.3	92.5	89.1	106.4	103.0	99.6	96.7	3.24
15	117.2	113.8	110.2	106.8	104.3	89.9	86.3	83.2	99.3	96.1	93.0	90.2	3.72
16	109.9	106.6	103.4	100.1	97.8	84.2	80.9	78.0	93.1	90.1	87.2	84.6	4.24
17	103.4	100.4	97.3	94.2	92.0	79.3	76.2	73.4	87.6	84.8	82.0	79.6	4.78
18	97.7	94.8	91.9	89.0	86.9	74.9	72.0	69.3	82.7	80.1	77.5	75.2	5.36
19	92.5	89.8	87.0	84.3	82.3	70.9	68.2	65.7	78.4	75.9	73.4	71.2	5.98
20	87.9	85.3	82.7	80.1	78.2	67.4	64.8	62.4	74.5	72.1	69.7	67.7	6.62
21	83.7	81.3	78.7	76.3	74.5	64.2	61.7	59.4	70.9	68.7	66.4	64.4	7.30
22	79.9	77.6	75.2	72.8	71.1	61.3	58.9	56.7	67.7	65.5	63.4	61.5	8.01
23	76.4	74.2	71.9	69.6	68.0	58.6	56.3	54.2	64.7	62.7	60.6	58.8	8.76
24	73.3	71.1	68.9	66.7	65.2	56.2	54.0	52.0	62.1	60.1	58.1	56.4	9.53
25	70.3	68.3	66.2	64.1	62.6	53.9	51.8	49.9	59.6	57.7	55.8	54.1	10.35
26	67.6	65.6	63.6	61.6	60.2	51.8	49.8	48.0	57.3	55.5	53.6	52.0	11.19
27	65.1	63.2	61.2	59.3	57.9	49.9	48.0	46.2	55.2	53.4	51.6	50.1	12.07
28	62.8	60.9	59.1	57.2	55.9	48.1	46.3	44.6	53.2	51.5	49.8	48.3	12.98
29	60.6	58.8	57.0	55.2	53.9	46.5	44.7	43.0	51.4	49.7	48.1	46.7	13.92
30	58.6	56.9	55.1	53.4	52.1	44.9	43.2	41.6	49.6	48.1	46.5	45.1	14.90
31	56.7	55.0	53.4	51.7	50.5	43.5	41.8	40.2	48.0	46.5	45.0	43.7	15.91
32	54.9	53.3	51.7	50.1	48.9	42.1	40.5	39.0	46.5	45.1	43.6	42.3	16.95
33	53.3	51.7	50.1	48.5	47.4	40.8	39.2	37.8	45.1	43.7	42.3	41.0	18.03
34	51.7	50.2	48.6	47.1	46.0	39.6	38.1	36.7	43.8	42.4	41.0	39.8	19.13
35	50.2	48.8	47.2	45.8	44.7	38.5	37.0	35.6	42.5	41.2	39.9	38.7	20.28
36	48.8	47.4	45.9	44.5	43.4	37.4	36.0	34.7	41.4	40.0	38.7	37.6	21.45
37	47.5	46.1	44.7	43.3	42.3	36.4	35.0	33.7	40.2	38.9	37.7	36.6	22.66
38	46.3	44.9	43.5	42.1	41.2	35.5	34.1	32.8	39.2	37.9	36.7	35.6	23.90
39	45.1	43.8	42.4	41.1	40.1	34.6	33.2	32.0					25.18
40	44.0	42.7	41.3	40.0	39.1	33.7	32.4	31.2					26.48
41	42.9	41.6	40.3	39.1	38.1	32.9	31.6	30.4					27.82
42	41.9	40.6	39.4	38.1	37.2	32.1	30.8	29.7					29.20

Loads above upper horizontal lines will produce maximum allowable shear in webs.
Loads below horizontal lines will produce excessive deflections.
For maximum safe loads see tables of Maximum Bending Moments and Web Resistance

BEAM SAFE LOADS—FIBER STRESS 16,000

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 16,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection
	18 Inch				15 Inch								
	70 lbs.	65 lbs.	60 lbs.	54.7 lbs.	75 lbs.	70 lbs.	65 lbs.	60.8 lbs.	55 lbs.	50 lbs.	45 lbs.	42.9 lbs.	
4					260.4				194.4				
5	256.0	226.4			244.3	231.0	201.6	177.0	180.9	165.0	135.6		0.27
	217.5	208.0			195.5	187.6	179.8	173.2	144.7	136.9	129.0		0.41
6			196.9	165.6								123.0	
7	181.2	173.4	165.5	157.1	162.9	156.4	149.8	144.4	120.6	114.0	107.5	104.7	0.60
8	155.3	148.6	141.9	134.7	139.8	134.0	128.4	123.7	103.4	97.8	92.1	89.7	0.81
9	135.9	130.0	124.1	117.9	122.2	117.3	112.4	108.3	90.4	85.5	80.6	78.5	1.06
10	120.8	115.6	110.3	104.8	108.6	104.2	99.9	96.2	80.4	76.0	71.7	69.8	1.34
	108.7	104.0	99.3	94.3	97.7	93.8	89.9	86.6	72.3	68.4	64.5	62.8	1.66
11	98.9	94.6	90.3	85.7	88.9	85.3	81.7	78.7	65.8	62.2	58.6	57.1	2.00
12	90.6	86.7	82.7	78.6	81.5	78.2	74.9	72.2	60.3	57.0	53.7	52.4	2.38
13	83.7	80.0	76.4	72.5	75.2	72.2	69.1	66.6	55.6	52.6	49.6	48.3	2.80
14	77.7	74.3	70.9	67.3	69.8	67.0	64.2	61.9	51.7	48.9	46.1	44.9	3.24
15	72.5	69.4	66.2	62.9	65.2	62.5	59.9	57.7	48.2	45.6	43.0	41.9	3.72
16	68.0	65.0	62.1	58.9	61.1	58.6	56.2	54.1	45.2	42.8	40.3	39.3	4.24
17	64.0	61.2	58.4	55.5	57.5	55.2	52.9	50.9	42.6	40.3	37.9	37.0	4.78
18	60.4	57.8	55.2	52.4	54.3	52.1	49.9	48.1	40.2	38.0	35.8	34.9	5.36
19	57.2	54.8	52.3	49.6	51.4	49.4	47.3	45.6	38.1	36.0	33.9	33.1	5.98
20	54.4	52.0	49.7	47.1	48.9	46.9	44.9	43.3	36.2	34.2	32.2	31.4	6.62
21	51.8	49.5	47.3	44.9	46.5	44.7	42.8	41.2	34.5	32.6	30.7	29.9	7.30
22	49.4	47.3	45.1	42.9	44.4	42.6	40.9	39.4	32.9	31.1	29.3	28.6	8.01
23	47.3	45.2	43.2	41.0	42.5	40.8	39.1	37.7	31.5	29.8	28.0	27.3	8.76
24	45.3	43.3	41.4	39.3	40.7	39.1	37.5	36.1	30.1	28.5	26.9	26.2	9.53
25	43.5	41.6	39.7	37.7	39.1	37.5	36.0	34.6	28.9	27.4	25.8	25.1	10.35
26	41.8	40.0	38.2	36.3	37.6	36.1	34.6	33.3	27.8	26.3	24.8	24.2	11.19
27	40.3	38.5	36.8	34.9	36.2	34.7	33.3	32.1	26.8	25.3	23.9	23.3	12.07
28	38.8	37.1	35.5	33.7	34.9	33.5	32.1	30.9	25.8	24.4	23.0	22.4	12.98
29	37.5	35.9	34.2	32.5	33.7	32.3	31.0	29.9	24.9	23.6	22.2	21.7	13.92
30	36.2	34.7	33.1	31.4	32.6	31.3	30.0	28.9	24.1	22.8	21.5	20.9	14.90
31	35.1	33.6	32.0	30.4	31.5	30.3	29.0	27.9	23.3	22.1	20.8	20.3	15.91
32	34.0	32.5	31.0	29.5	30.5	29.3	28.1	27.1	22.6	21.4	20.2	19.6	16.95
33	33.0	31.5	30.1	28.6									18.03
34	32.0	30.6	29.2	27.7									19.13
35	31.1	29.7	28.4	26.9									20.28
36	30.2	28.9	27.6	26.2									21.45
37	29.4	28.1	26.8	25.5									22.66
38	28.6	27.4	26.1	24.8									23.90

Loads above upper horizontal lines will produce maximum allowable shear in webs.
Loads below lower horizontal lines will produce excessive deflections.
For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 16,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections										Coefficient of Deflection.	
	12 Inch					10 Inch						
	55 lbs.	50 lbs.	45 lbs.	40 lbs.	35 lbs.	31.8 lbs.	40 lbs.	35 lbs.	30 lbs.	25.4 lbs.		
3	194.4						148.2	118.8				0.15
4	189.2	164.9	135.6		102.7		112.4	103.7	89.4			0.27
5	141.9	134.1	126.3	110.4	100.9	84.0	84.3	77.7	71.2	62.0		0.41
6	113.5	107.2	101.0	95.6	80.7	76.7	67.4	62.2	57.0	52.1		0.60
7	94.6	89.4	84.2	79.7	67.3	63.9	56.2	51.8	47.5	43.4		0.81
8	81.1	76.6	72.1	68.3	57.6	54.8	48.2	44.4	40.7	37.2		1.06
9	71.0	67.0	63.1	59.8	50.5	48.0	42.1	38.9	35.6	32.6		1.34
10	63.1	59.6	56.1	53.1	44.9	42.6	37.5	34.6	31.6	28.9		1.66
11	56.8	53.6	50.5	47.8	40.4	38.4	33.7	31.1	28.5	26.0		2.00
12	51.6	48.7	45.9	43.5	36.7	34.9	30.6	28.3	25.9	23.7		2.38
13	47.3	44.7	42.1	39.8	33.6	32.0	28.1	25.9	23.7	21.7		2.80
14	43.7	41.2	38.8	36.8	31.0	29.5	25.9	23.9	21.9	20.0		3.24
15	40.6	38.3	36.1	34.2	28.8	27.4	24.1	22.2	20.3	18.6		3.72
16	37.8	35.7	33.7	31.9	26.9	25.6	22.5	20.7	19.0	17.4		4.24
17	35.5	33.5	31.6	29.9	25.2	24.0	21.1	19.4	17.8	16.3		4.78
18	33.4	31.5	29.7	28.1	23.7	22.6	19.8	18.3	16.8	15.3		5.36
19	31.5	29.8	28.1	26.6	22.4	21.3	18.7	17.3	15.8	14.5		5.98
20	29.9	28.2	26.6	25.2	21.2	20.2	17.7	16.4	15.0	13.7		6.62
21	28.4	26.8	25.3	23.9	20.2	19.2	16.9	15.5	14.2	13.0		7.30
22	27.0	25.5	24.0	22.8	19.2	18.3	16.1	14.8	13.6	12.4		8.01
23	25.8	24.4	23.0	21.7	18.3	17.4	15.3	14.1	12.9	11.8		8.76
24	24.7	23.3	22.0	20.8	17.5	16.7						9.53
25	23.7	22.3	21.0	19.9	16.8	16.0						10.35
26	22.7	21.4	20.2	19.1	16.1	15.3						11.19
27	21.8	20.6	19.4	18.4	15.5	14.8						

Loads above upper horizontal lines will produce maximum allowable shear in webs.
Loads below horizontal lines will produce excessive deflections.
For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

BEAM SAFE LOADS—FIBER STRESS 16,000

BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.

For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 16,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections											Coefficient of Deflection
	9 Inch				8 Inch				7 Inch			
	35 lbs.	30 lbs.	25 lbs.	21.8 lbs.	25.5 lbs.	23 lbs.	20.5 lbs.	18.4 lbs.	20 lbs.	17.5 lbs.	15.3 lbs.	
3	130.3	101.0			85.1	70.6	55.8		63.0	48.3		0.15
4	87.9	80.1	71.5	52.2	60.5	57.0	53.5	43.2	42.6	39.6	35.0	0.27
5	66.0	60.1	54.2	50.3	45.4	42.8	40.2	37.9	32.0	29.7	27.6	0.41
6	52.8	48.1	43.3	40.3	36.3	34.2	32.1	30.3	25.6	23.7	22.1	0.60
7	44.0	40.1	36.1	33.6	30.2	28.5	26.8	25.3	21.3	19.8	18.4	0.81
8	37.7	34.3	31.0	28.8	25.9	24.4	22.9	21.7	18.3	17.0	15.8	1.06
9	33.0	30.0	27.1	25.2	22.7	21.4	20.1	19.0	16.0	14.8	13.8	1.34
10	29.3	26.7	24.1	22.4	20.2	19.0	17.9	16.9	14.2	13.2	12.3	1.66
11	26.4	24.0	21.7	20.1	18.1	17.1	16.1	15.2	12.8	11.9	11.0	2.00
12	24.0	21.8	19.7	18.3	16.5	15.6	14.6	13.8	11.6	10.8	10.0	2.38
13	22.0	20.0	18.1	16.8	15.1	14.3	13.4	12.6	10.7	9.9	9.2	2.80
14	20.3	18.5	16.7	15.5	14.0	13.2	12.4	11.7	9.8	9.1	8.5	3.24
15	18.8	17.2	15.5	14.4	13.0	12.2	11.5	10.8	9.1	8.5	7.9	3.72
16	17.6	16.0	14.4	13.4	12.1	11.4	10.7	10.1	8.5	7.9	7.4	4.24
17	16.5	15.0	13.5	12.6	11.3	10.7	10.0	9.5	8.0	7.4	6.9	4.78
18	15.5	14.1	12.7	11.8	10.7	10.1	9.5	8.9				5.36
19	14.7	13.3	12.0	11.2	10.1	9.5	8.9	8.4				5.98
20	13.9	12.6	11.4	10.6								6.62
	13.2	12.0	10.8	10.1								

Span in Feet	Depth and Weight of Sections											Coefficient of Deflection		
	6 Inch			5 Inch			4 Inch				3 Inch			
	17.25 lbs.	14.75 lbs.	12.5 lbs.	14.75 lbs.	12.25 lbs.	10 lbs.	10.5 lbs.	9.5 lbs.	8.5 lbs.	7.7 lbs.	7.5 lbs.		6.5 lbs.	5.7 lbs.
1	55.8			49.4	34.7		32.0	26.1	20.2		20.9	15.1	10.2	0.02
2	46.3	41.2	37.5	32.1	28.8	21.0	18.9	17.8	16.8	15.2	10.3	9.5	8.8	0.07
3	30.8	28.2	25.8	21.4	19.2	17.2	12.6	11.9	11.2	10.6	6.8	6.3	5.9	0.15
4	23.1	21.2	19.4	16.1	14.4	12.9	9.4	8.9	8.4	8.0	5.1	4.7	4.4	0.27
5	18.5	16.9	15.5	12.8	11.5	10.3	7.6	7.1	6.7	6.4	4.1	3.8	3.5	0.41
6	15.4	14.1	12.9	10.7	9.6	8.6	6.3	5.9	5.6	5.3	3.4	3.2	2.9	0.60
7	13.2	12.1	11.1	9.2	8.2	7.4	5.4	5.1	4.8	4.5	2.9	2.7	2.5	0.81
8	11.6	10.6	9.7	8.0	7.2	6.4	4.7	4.5	4.2	4.0	2.6	2.4	2.2	1.06
9	10.3	9.4	8.6	7.1	6.4	5.7	4.2	4.0	3.7	3.5				1.34
10	9.3	8.5	7.7	6.4	5.8	5.2	3.8	3.6	3.4	3.2				1.66
11	8.4	7.7	7.0	5.8	5.2	4.7								2.00
12	7.7	7.1	6.5	5.4	4.8	4.3								2.38
13	7.1	6.5	6.0											2.80
14	6.6	6.1	5.5											3.24

Loads above upper horizontal lines will produce maximum allowable shear in webs.

Loads below lower horizontal lines will produce excessive deflections.

For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

CARNEGIE STEEL COMPANY

CHANNELS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 16,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection
	15 Inch						13 Inch						
	55 lbs.	50 lbs.	45 lbs.	40 lbs.	35 lbs.	33.9 lbs.	50 lbs.	45 lbs.	40 lbs.	37 lbs.	35 lbs.	31.8 lbs.	
	244.2	214.8	185.4				204.6	175.0					
3	203.4	190.4	177.2	156.0	126.6	120.0	171.2	159.8	146.6	127.9	116.2	97.5	0.15
4	152.5	142.8	132.9	123.1	113.3	111.1	128.4	119.8	111.3	106.2	102.8	97.4	0.27
5	122.0	114.3	106.3	98.5	90.7	88.9	102.7	95.9	89.1	85.0	82.3	77.9	0.41
6	101.7	95.2	88.6	82.1	75.6	74.1	85.6	79.9	74.2	70.8	68.6	65.0	0.60
7	87.1	81.6	76.0	70.6	64.8	63.5	73.3	68.5	63.6	60.7	58.8	55.7	0.81
8	76.3	71.4	66.5	61.6	56.7	55.6	64.2	59.9	55.7	53.1	51.4	48.7	1.06
9	67.8	63.5	59.1	54.7	50.4	49.4	57.1	53.2	49.5	47.2	45.7	43.3	1.34
10	61.0	57.1	53.2	49.3	45.3	44.5	51.3	47.9	44.5	42.5	41.1	39.0	1.66
11	55.5	51.9	48.3	44.8	41.2	40.4	46.7	43.6	40.5	38.6	37.4	35.4	2.00
12	50.8	47.6	44.3	41.1	37.8	37.0	42.8	39.9	37.1	35.4	34.3	32.5	2.38
13	46.9	44.0	40.9	37.9	34.9	34.2	39.5	36.9	34.3	32.7	31.6	30.0	2.80
14	43.6	40.8	38.0	35.2	32.4	31.8	36.7	34.2	31.8	30.3	29.4	27.8	3.24
15	40.7	38.1	35.5	32.8	30.2	29.6	34.2	31.9	29.7	28.3	27.4	26.0	3.72
16	38.1	35.7	33.2	30.8	28.3	27.8	32.1	30.0	27.8	26.6	25.7	24.4	4.24
17	35.9	33.6	31.3	29.0	26.7	26.1	30.2	28.2	26.2	25.0	24.2	22.9	4.78
18	33.9	31.7	29.5	27.4	25.2	24.7	28.5	26.6	24.7	23.6	22.9	21.7	5.36
19	32.1	30.1	28.0	25.9	23.9	23.4	27.0	25.2	23.4	22.4	21.6	20.5	5.98
20	30.5	28.6	26.6	24.6	22.7	22.2	25.7	24.0	22.3	21.2	20.6	19.5	6.62
21	29.1	27.2	25.3	23.5	21.6	21.2	24.5	22.8	21.2	20.2	19.6	18.6	7.30
22	27.7	26.0	24.2	22.4	20.6	20.2	23.3	21.8	20.2	19.3	18.7	17.7	8.01
23	26.5	24.8	23.1	21.4	19.7	19.3	22.3	20.8	19.4	18.5	17.9	16.9	8.76
24	25.4	23.8	22.2	20.5	18.9	18.5	21.4	20.0	18.6	17.7	17.1	16.2	9.53
25	24.4	22.9	21.3	19.7	18.1	17.8	20.5	19.2	17.8	17.0	16.5	15.6	10.35
26	23.5	22.0	20.5	18.9	17.4	17.1	19.8	18.4	17.1	16.3	15.8	15.0	11.19
27	22.6	21.2	19.7	18.2	16.8	16.5	19.0	17.7	16.5	15.7	15.2	14.4	12.07
28	21.8	20.4	19.0	17.6	16.2	15.9	18.3	17.1	15.9	15.2	14.7	13.9	12.98
29	21.0	19.7	18.3	17.0	15.6	15.3							13.92
30	20.3	19.0	17.7	16.4	15.1	14.8							14.90
31	19.7	18.4	17.2	15.9	14.6	14.3							15.91
32	19.1	17.9	16.6	15.4	14.2	13.9							16.95

Loads above upper horizontal lines will produce maximum allowable shear in webs.

Loads below lower horizontal lines will produce excessive deflections.

For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

BEAM SAFE LOADS—FIBER STRESS 16,000

CHANNELS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS
 Applicable only when sections are braced against lateral deflection.
 For unbraced sections safe loads must be reduced, see page 101.
 Maximum Bending Stress, 16,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections										Coefficient of Deflection
	12 Inch					10 Inch					
	40 lbs.	35 lbs.	30 lbs.	25 lbs.	20.7 lbs.	35 lbs.	30 lbs.	25 lbs.	20 lbs.	15.3 lbs.	
	181.2					164.0	134.6	106.2			
2	174.7	151.7	122.4	92.9		122.9	109.8	96.8	75.9	48.0	0.07
3	116.4	105.9	95.5	85.0	67.2	81.9	73.2	64.5	55.8	47.6	0.15
4	87.3	79.5	71.7	63.8	56.9	61.4	54.9	48.4	41.8	35.7	0.27
5	69.9	63.6	57.3	51.0	45.5	49.2	43.9	38.7	33.5	28.5	0.41
6	58.2	53.0	47.8	42.5	38.0	41.0	36.6	32.3	27.9	23.8	0.60
7	49.9	45.4	40.9	36.4	32.5	35.1	31.4	27.6	23.9	20.4	0.81
8	43.7	39.7	35.8	31.9	28.5	30.7	27.5	24.2	20.9	17.8	1.06
9	38.8	35.3	31.8	28.3	25.3	27.3	24.4	21.5	18.6	15.9	1.34
10	34.9	31.8	28.7	25.5	22.8	24.6	22.0	19.4	16.7	14.3	1.66
11	31.8	28.9	26.1	23.2	20.7	22.3	20.0	17.6	15.2	13.0	2.00
12	29.1	26.5	23.9	21.3	19.0	20.5	18.3	16.1	13.9	11.9	2.38
13	26.9	24.5	22.0	19.6	17.5	18.9	16.9	14.9	12.9	11.0	2.80
14	25.0	22.7	20.5	18.2	16.3	17.6	15.7	13.8	12.0	10.2	3.24
15	23.3	21.2	19.1	17.0	15.2	16.4	14.6	12.9	11.2	9.5	3.72
16	21.8	19.9	17.9	15.9	14.2	15.4	13.7	12.1	10.5	8.9	4.24
17	20.6	18.7	16.9	15.0	13.4	14.5	12.9	11.4	9.8	8.4	4.78
18	19.4	17.7	15.9	14.2	12.7	13.7	12.2	10.7	9.3	7.9	5.36
19	18.4	16.7	15.1	13.4	12.0	12.9	11.6	10.2	8.8	7.5	5.98
20	17.5	15.9	14.3	12.8	11.4	12.3	11.0	9.7	8.4	7.1	6.62
21	16.6	15.1	13.6	12.1	10.8	11.7	10.5	9.2	8.0	6.8	7.30
22	15.9	14.4	13.0	11.6	10.4	11.2	10.0	8.8	7.6	6.5	8.01
23	15.2	13.8	12.5	11.1	9.9						8.76
24	14.6	13.2	11.9	10.6	9.5						9.53
25	14.0	12.7	11.5	10.2	9.1						10.35
26	13.4	12.2	11.0	9.8	8.8						11.19

Loads above upper horizontal lines will produce maximum allowable shear in webs.
 Loads below lower horizontal lines will produce excessive deflections.
 For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

CARNEGIE STEEL COMPANY

CHANNELS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

Maximum Bending Stress, 16,000 Pounds per Square Inch

Span in Feet	Depth and Weight of Sections															Coefficient of Deflection
	9 Inch				8 Inch					7 Inch						
	25 lbs.	20 lbs.	15 lbs.	13.4 lbs.	21.25 lbs.	18.75 lbs.	16.25 lbs.	13.75 lbs.	11.5 lbs.	19.75 lbs.	17.25 lbs.	14.75 lbs.	12.25 lbs.	9.8 lbs.		
	110.2	80.6			92.6	77.9	63.2	48.5		88.1	73.4	58.7	44.0			
2	83.6	71.8	61.3	41.4	63.5	58.3	53.0	47.8	35.2	50.4	45.8	41.2	36.7	29.4	0.07	
3	55.7	47.8	40.0	37.4	42.3	38.8	35.3	31.9	28.7	33.6	30.5	27.5	24.4	21.4	0.15	
4	41.8	35.9	30.0	28.0	31.7	29.1	26.5	23.9	21.5	25.2	22.9	20.6	18.3	16.1	0.27	
5	33.4	28.7	24.0	22.4	25.4	23.3	21.2	19.1	17.2	20.2	18.3	16.5	14.7	12.9	0.41	
6	27.9	23.9	20.0	18.7	21.2	19.4	17.7	15.9	14.4	16.8	15.3	13.7	12.2	10.7	0.60	
7	23.9	20.5	17.2	16.0	18.1	16.6	15.1	13.7	12.3	14.4	13.1	11.8	10.5	9.2	0.81	
8	20.9	17.9	15.0	14.0	15.9	14.6	13.3	11.9	10.8	12.6	11.4	10.3	9.2	8.0	1.06	
9	18.6	16.0	13.3	12.5	14.1	12.9	11.8	10.6	9.6	11.2	10.2	9.2	8.2	7.1	1.34	
10	16.7	14.4	12.0	11.2	12.7	11.7	10.6	9.6	8.6	10.1	9.2	8.2	7.3	6.4	1.66	
11	15.2	13.1	10.9	10.2	11.5	10.6	9.6	8.7	7.8	9.2	8.3	7.5	6.7	5.8	2.00	
12	13.9	12.0	10.0	9.3	10.6	9.7	8.8	8.0	7.2	8.4	7.6	6.9	6.1	5.4	2.38	
13	12.9	11.0	9.2	8.6	9.8	9.0	8.2	7.4	6.6	7.8	7.0	6.3	5.6	4.9	2.80	
14	11.9	10.3	8.6	8.0	9.1	8.3	7.6	6.8	6.2	7.2	6.5	5.9	5.2	4.6	3.24	
15	11.1	9.6	8.0	7.5	8.5	7.8	7.1	6.4	5.7	6.7	6.1	5.5	4.9	4.3	3.72	
16	10.4	9.0	7.5	7.0	7.9	7.3	6.6	6.0	5.4	6.3	5.7	5.2	4.6	4.0	4.24	
17	9.8	8.4	7.1	6.6	7.5	6.9	6.2	5.6	5.1						4.78	
18	9.3	8.0	6.7	6.2	7.1	6.5	5.9	5.3	4.8						5.36	
19	8.8	7.6	6.3	5.9											5.98	
20	8.4	7.2	6.0	5.6											6.62	

Span in Feet	Depth and Weight of Sections												Coefficient of Deflection	
	6 Inch				5 Inch			4 Inch			3 Inch			
	15.5 lbs.	13 lbs.	10.5 lbs.	8.2 lbs.	11.5 lbs.	9 lbs.	6.7 lbs.	7.25 lbs.	6.25 lbs.	5.4 lbs.	6 lbs.	5 lbs.		4.1 lbs.
					47.2			25.6			21.4	15.5		
1	67.1	52.4	37.7	24.0	44.2	33.5	19.0	24.3	19.8	14.4	14.7	13.0	10.2	0.02
2	34.6	30.7	26.8	23.1	22.1	18.8	15.8	12.1	11.1	10.1	7.3	6.5	5.8	0.07
3	23.1	20.5	17.8	15.4	14.7	12.5	10.5	8.1	7.4	6.7	4.9	4.3	3.9	0.15
4	17.3	15.3	13.4	11.6	11.0	9.4	7.9	6.1	5.5	5.1	3.7	3.3	2.9	0.27
5	13.8	12.3	10.7	9.2	8.8	7.5	6.3	4.9	4.4	4.1	2.9	2.6	2.3	0.41
6	11.5	10.2	8.9	7.7	7.4	6.3	5.3	4.0	3.7	3.4	2.4	2.2	1.9	0.60
7	9.9	8.8	7.6	6.6	6.3	5.4	4.5	3.5	3.2	2.9	2.1	1.9	1.7	0.81
8	8.6	7.7	6.7	5.8	5.5	4.7	4.0	3.0	2.8	2.5	1.8	1.5	1.5	1.06
9	7.7	6.8	5.9	5.1	4.9	4.2	3.5	2.7	2.5	2.2				1.34
10	6.9	6.1	5.4	4.6	4.4	3.8	3.2	2.4	2.2	2.0				1.66
11	6.3	5.6	4.9	4.2	4.0	3.4	2.9							2.00
12	5.8	5.1	4.5	3.9	3.7	3.1	2.6							2.38
13	5.3	4.7	4.1	3.6										2.80
14	4.9	4.4	3.8	3.3										3.24

Loads above upper horizontal lines will produce maximum allowable shear in webs.
Loads below lower horizontal lines will produce excessive deflections.
For maximum safe loads see tables of Maximum Bending Moments and Web Resistances.

BEAM SAFE LOADS—FIBER STRESS 16,000

MISCELLANEOUS BEAMS

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are braced against lateral deflection.
For unbraced sections safe loads must be reduced, see page 101.

* Maximum Bending Stress, **16,000 Pounds per Square Inch**

H BEAMS

Span in Feet	Depth and Weight of Sections								Coefficients of Deflection
	8 Inch			6 Inch			5 Inch	4 Inch	
	37.7 lb.	34.3 lb.	32.6 lb.	26.7 lb.	24.1 lb.	22.8 lb.	18.9 lb.	13.8 lb.	
3				52.5			81.8	25.0	0.15
4	80.0			42.1	37.5		25.4	14.3	0.27
5	64.4	60.0		33.7	32.1	80.0	20.3	11.4	0.41
6	53.7	51.3	60.0	28.1	26.7	26.1	16.9	9.5	0.60
7	46.0	44.0	43.0	24.1	22.9	22.3	14.5	8.1	0.81
8	40.3	38.5	37.6	21.1	20.1	19.6	12.7	7.1	1.06
9	35.8	34.2	33.4	18.7	17.8	17.4	11.3	6.3	1.34
10	32.2	30.8	30.1	16.8	16.0	15.6	10.1	5.7	1.66
11	29.3	28.0	27.3	15.3	14.6	14.2	9.2		2.00
12	26.8	25.7	25.1	14.0	13.4	13.0	8.5		2.38
13	24.8	23.7	23.1	13.0	12.3	12.0			2.80
14	23.0	22.0	21.5	12.0	11.5	11.2			3.24
15	21.5	20.5	20.1						3.72
16	20.1	19.2	18.8						4.24
17	18.9	18.1	17.7						4.78
18	17.9	17.1	16.7						5.36

CROSS TIE SECTIONS

Span in Feet	Depth and Weight of Sections				Coefficients of Deflection*
	5.5 Inch 24.0 Pounds	5.5 Inch 20.0 Pounds	4.25 Inch 14.5 Pounds	3 Inch 9.4 Pounds	
3	41.8		21.3	12.2	
4	40.3	27.5	19.6	8.9	0.15
5	30.3	26.0	14.7	6.7	0.27
	24.2	20.8	11.7	5.3	0.41
6	20.2	17.3	9.8	4.5	0.60
7	17.3	14.8	8.4	3.8	0.81
8	15.1	13.0	7.3	3.3	1.06
9	13.4	11.5	6.5	3.0	1.34
10	12.1	10.4	5.9	2.7	1.66
11	11.0	9.4	5.3		2.00
12	10.1	8.7			2.38
13	9.3	8.0			2.80
14	8.6	7.4			3.24

Loads above upper horizontal lines will produce maximum allowable shear in webs.

Loads below lower horizontal lines will produce excessive deflections.

*To obtain deflection divide coefficient by twice the distance of neutral axis from greatest distance of extreme fiber, obtained from Elements of Cross Tie Sections.

CARNEGIE STEEL COMPANY

UNEQUAL ANGLES

Neutral Axis Parallel to Shorter Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 16,000 Pounds per Square Inch

Size, Inches	Thick-ness, Inches	1 Foot Span			Size, Inches	Thick-ness, Inches	1 Foot Span			
		Safe Load	Safe Load	Length, Feet			Safe Load	Safe Load	Length, Feet	
8 x 6	1	161.17	7.49	21.5	6 x 3½	1	83.52	5.57	15.0	
	15/16	152.21	7.04	21.6		15/16	79.04	5.24	15.1	
	7/8	143.04	6.59	21.7		7/8	74.45	4.90	15.2	
	13/16	133.87	6.14	21.8		13/16	69.87	4.57	15.3	
	3/4	124.48	5.68	21.9		3/4	65.07	4.23	15.4	
	11/16	114.88	5.22	22.0		11/16	60.27	3.89	15.5	
	5/8	105.28	4.76	22.1		5/8	55.36	3.55	15.6	
	9/16	95.47	4.30	22.2		9/16	50.35	3.21	15.7	
	1/2	85.55	3.84	22.3		1/2	45.23	2.86	15.8	
	7/16	75.41	3.37	22.4		7/16	40.00	2.52	15.9	
							3/8	34.67	2.17	16.0
							5/16	29.23	1.83	16.0
8 x 3½	1	146.03	7.53	19.4	5 x 4	7/8	53.23	4.00	13.3	
	15/16	138.03	7.08	19.5		15/16	50.03	3.73	13.4	
	7/8	129.92	6.63	19.6		3/4	46.61	3.46	13.5	
	13/16	121.60	6.17	19.7		11/16	43.20	3.19	13.5	
	3/4	113.17	5.72	19.8		5/8	39.79	2.92	13.6	
	11/16	104.53	5.23	19.9		9/16	36.16	2.64	13.7	
	5/8	95.79	4.78	20.0		1/2	32.53	2.36	13.8	
	9/16	86.93	4.32	20.1		7/16	28.80	2.07	13.9	
	1/2	77.97	3.86	20.2		3/8	24.96	1.78	14.0	
	7/16	68.80	3.39	20.3						
7 x 3½	1	112.85	6.52	17.3	5 x 3½	7/8	52.05	4.04	12.9	
	15/16	106.67	6.13	17.4		15/16	48.85	3.76	13.0	
	7/8	100.48	5.75	17.5		3/4	45.65	3.49	13.1	
	13/16	94.08	5.36	17.6		11/16	42.35	3.21	13.2	
	3/4	87.68	4.97	17.6		5/8	38.93	2.93	13.3	
	11/16	81.07	4.58	17.7		9/16	35.41	2.64	13.4	
	5/8	74.35	4.18	17.8		1/2	31.89	2.36	13.5	
	9/16	67.52	3.77	17.9		7/16	28.16	2.07	13.6	
	1/2	60.59	3.37	18.0		3/8	24.43	1.79	13.7	
	7/16	53.44	2.96	18.1		5/16	20.69	1.51	13.7	
	3/8	46.19	2.54	18.2						
6 x 4	1	85.55	5.56	15.4	5 x 3	13/16	47.47	3.77	12.6	
	15/16	80.96	5.22	15.5		3/4	44.37	3.49	12.7	
	7/8	76.27	4.89	15.6		11/16	41.17	3.22	12.8	
	13/16	71.47	4.55	15.7		5/8	37.87	2.94	12.9	
	3/4	66.67	4.22	15.8		9/16	34.45	2.65	13.0	
	11/16	61.65	3.88	15.9		1/2	31.04	2.37	13.1	
	5/8	56.64	3.54	16.0		7/16	27.52	2.09	13.2	
	9/16	51.52	3.20	16.1		3/8	23.89	1.80	13.3	
	1/2	46.29	2.85	16.2		5/16	20.16	1.51	13.4	
	7/16	40.85	2.51	16.3						
	3/8	35.41	2.16	16.4						

BEAM SAFE LOADS—FIBER STRESS 16,000

UNEQUAL ANGLES

Neutral Axis Parallel to Longer Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 16,000 Pounds per Square Inch

Size, Inches	Thick- ness, Inches	1 Foot Span			Size, Inches	Thick- ness, Inches	1 Foot Span		
		Safe Load	Safe Load	Length, Feet			Maximum Span 360x Deflection	Safe Load	Safe Load
8 x 6	1	95.15	5.44	17.5	6 x 3½	1	30.93	3.09	10.0
	15/16	89.92	5.11	17.6		15/16	29.23	2.90	10.1
	¾	84.69	4.79	17.7		¾	27.63	2.71	10.2
	13/16	79.36	4.45	17.8		13/16	25.92	2.52	10.3
	¾	73.92	4.13	17.9		¾	24.21	2.33	10.4
	11/16	68.37	3.80	18.0		11/16	22.51	2.14	10.5
	¾	62.72	3.48	18.0		¾	20.69	1.95	10.6
	9/16	56.96	3.15	18.1		9/16	18.88	1.76	10.7
	½	51.09	2.81	18.2		½	16.96	1.57	10.8
	7/16	45.12	2.47	18.3		7/16	15.04	1.38	10.9
8 x 3½	1	32.21	3.10	10.4	5 x 4	9/16	13.12	1.19	11.0
	15/16	30.40	2.90	10.5		9/16	11.09	1.00	11.1
	¾	28.69	2.71	10.6		¾	35.31	3.15	11.2
	13/16	26.88	2.52	10.7		13/16	33.17	2.93	11.3
	¾	25.07	2.33	10.8		¾	30.93	2.71	11.4
	11/16	23.15	2.13	10.9		11/16	28.69	2.50	11.5
	¾	21.33	1.94	11.0		¾	26.45	2.28	11.6
	9/16	19.41	1.74	11.1		9/16	24.11	2.06	11.7
	½	17.49	1.57	11.2		½	21.76	1.84	11.8
	7/16	15.57	1.38	11.3		7/16	19.31	1.62	11.9
7 x 3½	1	31.57	3.10	10.2	5 x 3½	¾	16.75	1.40	12.0
	15/16	29.87	2.90	10.3		¾	26.88	2.71	9.9
	¾	28.16	2.71	10.4		13/16	25.28	2.53	10.0
	13/16	26.45	2.52	10.5		¾	23.68	2.34	10.1
	¾	24.64	2.33	10.6		11/16	21.97	2.15	10.2
	11/16	22.83	2.14	10.7		¾	20.27	1.97	10.3
	¾	21.01	1.95	10.8		9/16	18.45	1.78	10.4
	9/16	19.20	1.76	10.9		½	16.64	1.60	10.4
	½	17.28	1.57	11.0		7/16	14.83	1.41	10.5
	7/16	15.36	1.38	11.1		¾	12.91	1.22	10.6
6 x 4	¾	13.44	1.19	11.2	9/16	10.88	1.02	10.7	
	1	40.43	3.55	11.4	5 x 3	13/16	18.56	2.16	8.6
	15/16	38.29	3.33	11.5		¾	17.39	2.00	8.7
	¾	36.16	3.12	11.6		11/16	16.11	1.83	8.8
	13/16	33.92	2.90	11.7		¾	14.83	1.67	8.9
	¾	31.68	2.69	11.8		9/16	13.55	1.51	9.0
	11/16	29.44	2.47	11.9		½	12.27	1.35	9.1
	¾	27.09	2.26	12.0		7/16	10.88	1.18	9.2
	9/16	24.64	2.05	12.0		¾	9.49	1.02	9.3
	½	22.19	1.84	12.1		9/16	8.00	0.85	9.4
7/16	19.73	1.62	12.2						
¾	17.07	1.39	12.3						

CARNEGIE STEEL COMPANY

UNEQUAL ANGLES

Neutral Axis Parallel to Shorter Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 16,000 Pounds per Square Inch

Size, Inches	Thick- ness, Inches	1 Foot Span			Size, Inches	Thick- ness, Inches	1 Foot Span		
		Safe Load	Safe Load	Length, Feet			Safe Load	Safe Load	Length, Feet
4½ x 3	1½	38.61	3.36	11.5	3 x 2½	¾	12.27	1.53	8.0
	¾	36.05	3.11	11.6		½	11.09	1.37	8.1
	11/16	33.49	2.87	11.7		1/8	9.92	1.22	8.1
	5/8	30.83	2.62	11.8		3/8	8.64	1.06	8.2
	9/16	28.16	2.38	11.8		1/2	7.36	0.89	8.3
	½	25.28	2.13	11.9		¾	5.97	0.71	8.4
	7/16	22.40	1.87	12.0					
	3/8	19.52	1.61	12.1		½	10.67	1.39	7.7
	5/16	16.43	1.35	12.2		1/8	9.49	1.22	7.8
							3/8	8.32	1.05
4 x 3½	1½	31.15	2.94	10.6	3 x 2	1/8	7.04	0.88	8.0
	¾	29.33	2.74	10.7		¼	5.76	0.71	8.1
	11/16	27.31	2.52	10.8					
	5/8	25.07	2.30	10.9		½	7.47	1.15	6.5
	9/16	22.93	2.08	11.0		1/8	6.72	1.02	6.6
	½	20.59	1.86	11.1		3/8	5.87	0.88	6.7
	7/16	18.35	1.64	11.2		1/2	5.01	0.74	6.8
	3/8	16.00	1.41	11.3		¾	4.05	0.59	6.9
	5/16	13.44	1.18	11.4		1	3.09	0.44	7.0
							1½	2.13	0.30
4 x 3	1½	30.61	2.97	10.3	2½ x 2				
	¾	28.59	2.75	10.4		1/8	4.69	0.73	6.4
	11/16	26.56	2.53	10.5		¼	3.84	0.59	6.5
	5/8	24.53	2.31	10.6		1/2	2.99	0.45	6.6
	9/16	22.40	2.09	10.7					
	½	20.16	1.87	10.8		1/8	5.76	1.02	5.6
	7/16	17.92	1.64	10.9		1/4	5.12	0.90	5.7
	3/8	15.57	1.42	11.0		3/8	4.48	0.77	5.8
	5/16	13.12	1.19	11.0		1/2	3.84	0.65	5.9
	¼	10.67	0.96	11.1		¾	3.20	0.53	6.0
					1	2.45	0.40	6.0	
3½ x 3	1½	23.47	2.57	9.1	2½ x 1½	1/8	3.63	0.70	5.2
	¾	21.87	2.38	9.2		1/4	3.09	0.58	5.3
	11/16	20.37	2.19	9.3		1/2	2.56	0.47	5.4
	5/8	18.77	2.00	9.4		3/8	1.92	0.35	5.5
	9/16	17.17	1.81	9.5		1/2	1.33	0.24	5.6
	½	15.47	1.62	9.5					
	7/16	13.76	1.43	9.6		1/8	2.44	0.47	5.2
	3/8	12.05	1.24	9.7		1/4	1.89	0.36	5.3
	5/16	10.24	1.05	9.8					
	¼	8.32	0.84	9.9					
3½ x 2½	1½	19.73	2.19	9.0	2 x 1¼	1/8	1.88	0.41	4.6
	5/8	18.24	2.00	9.1		1/4	1.46	0.31	4.7
	11/16	16.64	1.82	9.1		1/2	1.00	0.21	4.8
	9/16	15.04	1.63	9.2					
	½	13.44	1.44	9.3		1/8	1.71	0.44	3.9
	7/16	11.73	1.24	9.4		1/4	1.39	0.35	4.0
	3/8	9.92	1.04	9.5		1/2	1.08	0.26	4.1
	5/16	8.00	0.83	9.6					
	¼								

BEAM SAFE LOADS—FIBER STRESS 16,000

UNEQUAL ANGLES

Neutral Axis Parallel to Longer Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 16,000 Pounds per Square Inch

Size, Inches	Thick- ness, Inches	1 Foot Span			Size, Inches	Thick- ness, Inches	1 Foot Span					
		Safe Load	Safe Load	Length, Feet			Safe Load	Safe Load	Length, Feet			
4 1/2 x 3	1 3/16	18.24	2.15	8.5	3 x 2 1/2	9/16	8.75	1.25	7.0			
	3/4	17.07	1.99	8.6		1/2	7.89	1.12	7.0			
	1 1/16	15.89	1.83	8.7		7/16	7.04	0.99	7.1			
	5/8	14.61	1.67	8.8		3/8	6.19	0.85	7.2			
	9/16	13.33	1.51	8.8		5/16	5.23	0.72	7.3			
	1/2	12.05	1.35	8.9		1/4	4.27	0.58	7.4			
	7/16	10.77	1.19	9.0		3 x 2	1/2	5.01	0.88	5.7		
	3/8	9.39	1.03	9.1			7/16	4.48	0.77	5.8		
	5/16	8.00	0.87	9.2			3/8	3.95	0.67	5.9		
	4 x 3 1/2	1 3/16	24.53	2.56			9.6	5/16	3.41	0.57	6.0	
3/4		22.93	2.37	9.7	1/4		2.77	0.46	6.1			
1 1/16		21.33	2.18	9.8	2 1/2 x 2		1/2	4.91	0.89	5.5		
5/8		19.63	1.98	9.9		7/16	4.37	0.78	5.6			
9/16		17.92	1.79	10.0		3/8	3.84	0.67	5.7			
1/2		16.21	1.60	10.1		5/16	3.31	0.57	5.8			
7/16		14.40	1.41	10.2		1/4	2.67	0.46	5.9			
3/8		12.59	1.22	10.3		5/16	2.13	0.35	6.0			
5/16		10.67	1.03	10.4		3/8	1.44	0.24	6.1			
4 x 3		1 3/16	17.92	2.15		8.3	2 1/2 x 1 1/2	5/16	1.81	0.41	4.4	
	3/4	16.75	1.99	8.4		1/4		1.49	0.33	4.5		
	1 1/16	15.57	1.83	8.5		5/16		1.17	0.25	4.6		
	5/8	14.40	1.67	8.6	2 1/4 x 1 1/2	1/2		2.77	0.67	4.1		
	9/16	13.12	1.51	8.7		7/16		2.45	0.58	4.2		
	1/2	11.84	1.35	8.8		3/8		2.13	0.50	4.3		
	7/16	10.56	1.19	8.9		5/16		1.81	0.41	4.4		
	3/8	9.28	1.03	8.9		1/4		1.49	0.33	4.5		
	5/16	7.89	0.87	9.0		5/16		1.17	0.25	4.6		
	1/4	6.40	0.70	9.1		2 x 1 1/2		3/8	2.13	0.51	4.2	
3 1/2 x 3	1 3/16	17.60	2.17	8.1			5/16	1.81	0.42	4.3		
	3/4	16.43	2.01	8.2			1/4	1.49	0.34	4.4		
	1 1/16	15.36	1.85	8.3			5/16	1.17	0.26	4.5		
	5/8	14.19	1.69	8.4	3/8		0.80	0.17	4.6			
	9/16	12.91	1.52	8.5	2 x 1 1/4		1/2	1.03	0.28	3.7		
	1/2	11.73	1.36	8.6			5/16	0.80	0.21	3.8		
	7/16	10.45	1.20	8.7			1 3/4 x 1 1/4	1/4	1.01	0.28	3.6	
	3/8	9.07	1.04	8.7				5/16	0.80	0.22	3.7	
	5/16	7.68	0.87	8.8				3/8	0.56	0.15	3.8	
	1/4	6.19	0.70	8.9		1 1/2 x 1 1/4		5/16	1.21	0.35	3.4	
3 1/2 x 2 1/2	1 1/16	10.56	1.51	7.0				1/4	0.99	0.28	3.5	
	5/8	9.81	1.39	7.1				5/16	0.78	0.22	3.6	
	9/16	8.96	1.26	7.1				1 1/2 x 1 1/4	5/16	1.21	0.35	3.4
	1/2	8.11	1.13	7.2					1/4	0.99	0.28	3.5
	7/16	7.25	0.99	7.3	5/16				0.78	0.22	3.6	
	3/8	6.29	0.85	7.4	1 1/2 x 1 1/4				5/16	1.21	0.35	3.4
	5/16	5.33	0.71	7.5			1/4		0.99	0.28	3.5	
	1/4	4.37	0.58	7.6			5/16		0.78	0.22	3.6	

CARNEGIE STEEL COMPANY

EQUAL ANGLES

Neutral Axis Parallel to Either Leg

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 16,000 Pounds per Square Inch

Size, Inches	Thick- ness, Inches	1 Foot Span			Size, Inches	Thick- ness, Inches	1 Foot Span		
		Safe Load	Safe Load	Length, Feet			Safe Load	Safe Load	Length, Feet
8 x 8	1 1/8	186.99	8.31	22.5	3 1/2 x 3 1/2	1 3/16	24.00	2.55	9.4
	1 1/4	177.81	7.87	22.6		3/4	22.51	2.37	9.5
	1	168.53	7.43	22.7		1 1/4	20.91	2.18	9.6
	7/8	159.15	6.98	22.8		5/8	19.31	2.00	9.7
	3/4	149.55	6.53	22.9		7/8	17.60	1.81	9.7
	1 3/8	139.84	6.08	23.0		1/2	15.89	1.62	9.8
	3/4	130.03	5.63	23.1		7/16	14.08	1.42	9.9
	1 1/4	120.00	5.18	23.2		5/8	12.27	1.23	10.0
	5/8	109.87	4.73	23.2		7/16	10.45	1.04	10.1
	9/16	99.63	4.28	23.3		1/4	8.43	0.83	10.2
	1/2	89.28	3.82	23.4		5/8	13.87	1.69	8.2
	1	91.41	5.48	16.7		7/16	12.69	1.53	8.3
	1 3/8	86.51	5.16	16.8		1/2	11.41	1.37	8.3
	3/4	81.39	4.84	16.8		7/16	10.13	1.21	8.4
1 1/4	76.27	4.51	16.9	5/8	8.85	1.04	8.5		
6 x 6	3/4	71.04	4.18	17.0	7/8	7.57	0.88	8.6	
	1 1/4	65.81	3.85	17.1	1/4	6.19	0.71	8.7	
	5/8	60.37	3.51	17.2	1/2	7.79	1.15	6.8	
	9/16	54.83	3.17	17.3	7/16	6.93	1.01	6.9	
	1/2	49.17	2.83	17.4	5/8	6.08	0.87	7.0	
	7/16	43.41	2.48	17.5	7/8	5.12	0.72	7.1	
	3/8	37.65	2.14	17.6	1/4	4.16	0.58	7.2	
	1	61.87	4.55	13.6	7/16	3.20	0.44	7.3	
	1 3/8	58.56	4.28	13.7	5/8	2.13	0.29	7.4	
	3/4	55.15	4.00	13.8	7/16	4.27	0.79	5.4	
	1 1/4	51.73	3.73	13.9	5/8	3.73	0.68	5.5	
	5/8	48.32	3.45	14.0	7/8	3.20	0.57	5.6	
	1 1/8	44.80	3.18	14.1	1/4	2.67	0.46	5.7	
	3/4	41.17	2.90	14.2	7/16	2.03	0.35	5.8	
1 3/8	37.44	2.62	14.3	5/8	1.39	0.24	5.8		
5 x 5	1/2	33.60	2.34	14.4	7/8	3.20	0.68	4.7	
	7/16	29.76	2.06	14.5	5/8	2.77	0.60	4.7	
	3/8	25.81	1.78	14.5	7/16	2.45	0.51	4.8	
	1 3/8	32.11	2.95	10.9	1/4	2.03	0.41	4.9	
	3/4	29.97	2.73	11.0	7/8	1.49	0.30	5.0	
	1 1/4	27.84	2.51	11.1	5/8	1.07	0.21	5.1	
	5/8	25.60	2.29	11.2	7/16	2.03	0.51	4.0	
	9/16	23.36	2.07	11.3	5/8	1.73	0.42	4.1	
	1/2	21.01	1.85	11.4	1/4	1.43	0.34	4.2	
	7/16	18.67	1.63	11.4	7/16	1.11	0.26	4.3	
	3/8	16.21	1.41	11.5	5/8	0.77	0.18	4.4	
	9/16	13.76	1.19	11.6	7/16	1.19	0.36	3.3	
	1/4	11.20	0.96	11.7	1/4	0.97	0.29	3.4	
	4 x 4	7/16	18.67	1.63	11.4	7/16	0.76	0.22	3.5
3/8		16.21	1.41	11.5	5/8	0.52	0.14	3.6	
9/16		13.76	1.19	11.6	1/4	0.60	0.22	2.6	
1/4		11.20	0.96	11.7	7/16	0.47	0.17	2.7	
1 x 1					5/8	0.33	0.12	2.8	

BEAM SAFE LOADS—FIBER STRESS 16,000

TEES AND ZEES

Neutral Axis Parallel to Flanges

ALLOWABLE UNIFORM LOAD IN THOUSANDS OF POUNDS

Applicable only when sections are rigidly secured against lateral deflection

Maximum Bending Stress, 16,000 Pounds per Square Inch

TEES

Size Flange x Stem, Inches	Weight per Foot, Pounds	1 Foot Span			Maximum Span 360 x Deflection			Size Flange x Stem, Inches	Weight per Foot, Pounds	1 Foot Span			Maximum Span 360 x Deflection		
		Safe Load	Safe Load	Length, Feet	Safe Load	Safe Load	Length, Feet			Safe Load	Safe Load	Length, Feet	Safe Load	Safe Load	Length, Feet
6 ½ x 6 ½	19.8	52.80	2.77	19.1	5 x 3	11.5	11.31	1.25	9.0						
4 x 4	13.5	21.55	1.90	11.4	4 x 5	15.3	33.39	2.41	13.9						
4 x 4	10.5	16.85	1.46	11.6	4 x 5	11.9	25.92	1.84	14.1						
3 x 3	7.8	9.17	1.07	8.5	4 x 4 ½	14.4	27.09	2.15	12.6						
3 x 3	6.7	7.89	0.92	8.6	4 x 4 ½	11.2	21.12	1.65	12.8						
2 ½ x 2 ½	6.4	6.29	0.90	7.0	4 x 3	9.2	9.60	1.07	8.9						
2 ½ x 2 ½	5.5	5.33	0.75	7.1	4 x 3	7.8	8.21	0.91	9.1						
2 ¼ x 2 ¼	4.9	4.37	0.69	6.3	4 x 2 ½	8.5	6.61	0.87	7.6						
2 ¼ x 2 ¼	4.1	3.41	0.53	6.4	4 x 2 ½	7.2	5.65	0.74	7.7						
2 x 2	4.3	3.31	0.59	5.6	3 x 2 ½	6.1	5.55	0.76	7.3						
2 x 2	3.56	2.77	0.49	5.7	2 ½ x 3	6.1	7.68	0.92	8.4						
					1 ½ x 2	2.45	2.08	0.38	5.5						
					1 ½ x 1 ½	1.25	0.57	0.15	3.7						
					1 ¼ x ¾	0.88	0.14	0.08	1.9						

ZEES

Size Depth x Flange, Inches	Thick- ness, Inches	1 Foot Span			Maximum Span 360 x Deflection			Size Depth x Flange, Inches	Thick- ness, Inches	1 Foot Span			Maximum Span 360 x Deflection		
		Safe Load	Safe Load	Length, Feet	Safe Load	Safe Load	Length, Feet			Safe Load	Safe Load	Length, Feet	Safe Load	Safe Load	Length, Feet
6 ½ x 3 ¾	¾	174.93	14.18	12.3	4 ½ x 3 ¾	¾	77.44	9.32	8.3						
6 ½ x 3 ¾	1 ¼	162.35	13.30	12.2	4 ½ x 3 ¾	1 ¼	70.93	8.67	8.2						
6 x 3 ½	¾	149.76	12.40	12.1	4 x 3 ¾	¾	64.53	8.01	8.1						
6 ½ x 3 ¾	1 ½	150.40	12.19	12.3	4 ½ x 3 ¾	1 ½	65.92	7.93	8.3						
6 ½ x 3 ¾	5 8	136.75	11.20	12.2	4 ½ x 3 ¾	5 8	58.67	7.17	8.2						
6 x 3 ½	1 1 8	123.20	10.20	12.1	4 ½ x 3 ¾	1 ½	58.67	7.17	8.2						
6 ½ x 3 ¾	1 ½	119.68	9.70	12.3	4 x 3 ¾	1 1 8	51.52	6.40	8.1						
6 ½ x 3 ¾	7 16	104.85	8.59	12.2	4 ½ x 3 ¾	¾	49.81	6.00	8.3						
6 x 3 ½	¾	90.03	7.45	12.1	4 ½ x 3 ¾	7 16	41.71	5.10	8.2						
					4 x 3 ¾	1 ¼	33.49	4.16	8.1						
5 ½ x 3 ¾	1 1 8	119.47	11.58	10.3											
5 ½ x 3 ¾	¾	110.29	10.82	10.2											
5 x 3 ¾	1 1 8	101.01	10.03	10.1	3 ½ x 2 ¾	1 1 8	36.59	5.93	6.2						
5 ½ x 3 ¾	5 8	102.08	9.89	10.3	3 x 2 1 1 8	1 ½	32.64	5.40	6.1						
5 ½ x 3 ¾	1 1 8	91.95	9.02	10.2	3 ½ x 2 ¾	7 16	31.79	5.15	6.2						
5 x 3 ¾	1 ½	81.92	8.14	10.1	3 x 2 1 1 8	¾	27.41	4.54	6.1						
5 ½ x 3 ¾	7 16	79.36	7.69	10.3	3 ½ x 2 ¾	5 8	25.39	4.12	6.2						
5 ½ x 3 ¾	¾	68.16	6.69	10.2	3 x 2 1 1 8	1 ¼	20.48	3.39	6.1						
5 x 3 ¾	1 1 8	56.96	5.66	10.1											

PLATE AND ANGLE GIRDERS

Girders, built up of plates and angles, are used for heavy loads and long spans, where rolled sections are insufficient.

Loads upon a plate and angle girder develop compressive and tensile stresses resisted by the upper and lower flanges, and shearing stresses resisted by the web plate.

The most economical section is the single web girder; box girders with double or triple webs are used where great length of span combined with lateral stiffness require them.

WEB. The web plate governs the depth of the girder which, to avoid excessive deflection, should not be less than 1/15 of the span, the thickness depends upon the shear which is greatest at the point of support and should not be less than 1/160 of the unsupported distance between the flanges; the web is reinforced by stiffeners at intervals to prevent buckling.

Web Shear and Stiffeners. Web plates subjected to direct vertical shear must resist buckling; the allowable vertical shear may be obtained from the table on page 147, based on a maximum shear stress of 12000 pounds, giving allowable unit web shear, V/A , total vertical shear ÷ gross area of web, for various ratios of h/t , distance between flanges ÷ thickness of web.

Stiffeners are required at the ends and at points of concentrated loads and at other points where the clear distance between flange angles, h , exceeds allowable safe stresses obtained from table, and also where h is greater than 60 times the thickness of the web; stiffeners are generally in pairs, one on each side of the web, bearing closely against the projecting leg of the flange angles; the pitch of rivets in stiffeners should not exceed 6 inches.

FLANGES. The flange area is so proportioned that the extreme compressive or tensile stress, $f = \frac{nM}{I}$, does not exceed the maximum allowable limit, being so proportioned that the unit stress on the net section does not exceed the maximum unit stress as determined by the moment of inertia of the net section of the girder.

When the flanges are alike, as they usually are, the preliminary investigation is simplified by assuming that the stresses in the flanges are uniformly distributed, and their resultants act at the center of gravity of the flanges.

A=Area of one flange d=Effective depth t=Web thickness

Total Moment of Resistance, $M=f(A d + \frac{d^2 t}{6})=f d (A + \frac{d t}{6})$.

The net moment of resistance of the web plate, with allowance for reduction of area due to web splices is generally taken as $\frac{d t}{8}$, or:

Net Moment of Resistance, $M=f(A d + \frac{d^2 t}{8})=f d (A + \frac{d t}{8})$,

d is the approximate distance between centers of gravity of flange angles, or distance out to out of angles when flange plates are used.

The final design of the girder is obtained in accordance with the method given for the computation of compound sections.

Flange Plates. When the girder carries a uniformly distributed load, the flange areas vary as the ordinates of a parabola, and the theoretical length of the flange plates is

$$L_1=L\sqrt{\frac{a_1}{A}} \quad L_2=L\sqrt{\frac{a_2}{A}} \quad L_3=L\sqrt{\frac{a_3}{A}}$$

L —Length of girder.

A —Total Area of Flange.

L_1, L_2, L_3 —Length of flange plates, beginning with outside plate.

a_1, a_2, a_3 —Total area of flange plates, from outer to inner plates.

Sufficient length, usually from 12 to 18 inches, is added to each end of plate to take up the shear; the plate next to the flange angle is extended to full length of the girder, to resist lateral deflection.

EXAMPLE. Required the length of flange plates of a 60-inch girder, 60 feet long, the flange including flange angles, two flange plates and one-eighth of web plate; rivets $\frac{3}{4}$ " dia.

2—Angles	6" x 3½" x ½"	Net Area	9.00—1.75=7.25 sq. in.	} Total Area: 20.0 sq. in.
1—Inner Flange Plate	14" x ¾"	" "	6.13—0.77=5.36 "	
1—Outer Flange Plate	14" x ¾"	" "	5.25—0.66=4.59 "	
⅛—Web Plate	60" x ⅝"	" "	2.81 =2.81 "	

Outer plate, $L_1=60\sqrt{\frac{4.59}{20.0}}=28.8$ ft. say 32 ft. Inner plate, $L_2=60\sqrt{\frac{4.59+5.36}{20.0}}=42.3$ ft. full length.

Maximum End Flange Stress. In addition to a girder having sufficient flange area to resist the maximum bending moment, it must also be capable of withstanding stresses at the ends.

The end resistance of a riveted girder depends on: First, the resistance of the web plate to shearing; second, the resistance of the flange rivets to bearing, it being assumed that the bearing value of rivets does not exceed twice their value in single shear.

The difference in flange stress between any two points is the horizontal shear to be transmitted into the web by the flange rivets between those points, and can not be greater than that of the end reaction considered to be distributed along the flange within a length equal to the distance, a , between the center of the rivets in upper and lower flange with one line of rivets, or between the center lines of two lines of rivets, according to design.

Web Stress: $d \times t \times f = \text{Maximum End Resistance if less than flange stress.}$
 $d = \text{Depth.}$ $t = \text{Thickness of web.}$ $f = \text{Allowable unit shearing stress.}$

Flange Stress: $\frac{aR}{p} = \text{Maximum End Resistance if less than web stress.}$
 $a = \text{Effective distance } (d + 1\frac{1}{2}'') - 2 \times \text{distance from back of angles, for one or two rivet lines,}$
 $R = \text{Bearing value of one rivet.}$ $p = \text{Minimum pitch between two rivets.}$

Required the maximum end resistance of a girder, properly stiffened at ends,

EXAMPLE 1. Girder composed of 1—Web Plate, 36" x 5/16" 4—Flange Angles, 5" x 3 1/2".
 Web Stress: 36 x 5/16 x 12000 = 135,000 pounds.
 Flange Rivets: $a = (36'' + 1\frac{1}{2}'') - 4\frac{1}{2}'' = 32'';$ 32 ÷ 2 1/2 = 13 Rivets.
 Bearing value of 3/4" Dia. Rivet: 3/4 x 5/16 x 24000 = 5625 pounds.
 Flange Stress: 5625 x 13 = 73,125 pounds = Maximum End Resistance.

EXAMPLE 2. Girder composed of 1—Web Plate, 48" x 3/8" 4—Flange Angles, 6" x 6".
 Web Stress: 48 x 3/8 x 12000 = 216,000 pounds.
 Flange Rivets: $a = (48'' + 1\frac{1}{2}'') - 7'' = 41\frac{1}{2}'';$ 41 1/2 ÷ 1 1/2 = 28 Rivets.
 Bearing value of 3/4" Dia. Rivet: 3/4 x 3/8 x 24000 = 6750 pounds.
 Flange Stress: 6750 x 28 = 189,000 pounds = Maximum End Resistance.

Rivet Spacing in Flanges. It follows that the rivets connecting the web plate with the flange angles are required to transmit the horizontal shearing stress from the web to the flange, which horizontal shear in any panel is equal to the vertical shear at center of panel multiplied by its length and divided by the vertical distance, a.

As the shear increases from the point of greatest bending moment towards the supports, the number of rivets in vertical legs of the flange angles must also increase as the supports are approached.

Pitch of rivets in flange angles, $p = \frac{aR}{V}$

V = Total vertical shear at the panel under consideration.

R = Resistance of one rivet, i. e., the bearing or shearing value, whichever is smaller.

a = Effective distance between upper and lower lines of rivets.

The formula gives the theoretical rivet spacing for any point in the flanges due to the total shear, but in practice the pitch is computed from the maximum stress in each panel, in nearest 1/4 inch.

EXAMPLE. A girder composed of 5" x 3 1/2" angles and 36" x 5/16" web, 30 ft. long, divided into 3-foot panels, supports a uniformly distributed load of 72 tons, or 4800 pounds per foot

Required rivet pitch in panels, when distance between rivet lines = 32 inches.

	Shearing Stress, Pounds	Horizontal Stress, Pounds per Inch
Panel 1.	144000 ÷ 2 = 72000	72000 ÷ 32 = 2250
" 2.	72000 - (4800 x 3) = 57600	57600 ÷ 32 = 1800
" 3.	72000 - (4800 x 6) = 43200	43200 ÷ 32 = 1350
" 4.	72000 - (4800 x 9) = 28800	28800 ÷ 32 = 900
Bearing value of 3/4" Dia. Rivet: 3/4 x 5/16 x 24000 = 5625 pounds.		
Panel 1.	Rivet Pitch 5625 ÷ 2250 = 2.50	say 2 1/2" spacing
" 2.	" " 5625 ÷ 1800 = 3.13	" 3" "
" 3.	" " 5625 ÷ 1350 = 4.17	" 4" "
" 4.	" " 5625 ÷ 900 = 6.25	" 6" "

When the load rests directly on the top or bottom flanges, the rivets connecting this flange with the web plate are also required to distribute the load; then the resultant stress on rivets on the loaded flange is represented by the resultant of horizontal shear and vertical load.

EXAMPLE. Loads bearing directly on one flange only, then, first panel, foregoing example:
 Horizontal Shear 2250 pounds per inch. Vertical Load 400 pounds per inch,
 Resultant Stress $\sqrt{2250^2+400^2} = 2285$ pounds. Rivet Pitch $5625 \div 2285 = 2.46''$

Flange Plates. At the end of each flange plate, sufficient rivets must be provided to transmit the allowable stress on the net section of the plate to the adjacent members.

EXAMPLE. Required number of rivets, $\frac{3}{4}''$ Dia., for $14'' \times \frac{7}{16}''$ Inner Flange Plate.
 $14'' \times \frac{7}{16}''$ Inner Flange Plate, net area: $(6.13 - 0.77) = 5.36$ sq. in.
 Resistance $5.36 \times 18000 = 96480$ pounds.
 Shearing value of $\frac{3}{4}''$ Dia. Rivet: $0.4418 \times 12000 = 5300$ pounds.
 $96480 \div 5300 = 18$ rivets, or:
 Two lines of 9 rivets each end of plate, spaced $2\frac{1}{2}$ to 3 inches.

SPLICES. In long and deep girders or in girders to be made from stock lengths, it is often necessary to splice the web plate or also flange angles and plates.

The resistance of all splice plates must be such as to develop the full resisting strength of the rivets in the splice, in particular when rivet stresses are to be transmitted through narrow plates.

Web Splices. As there is no vertical shearing stress in the middle of the girder under a uniformly distributed load, web splices are sometimes made at that point, but, generally, the web is spliced in two places equidistant from the center.

The rivets in the web splice must transmit the web stresses so that no additional stresses are imparted to the flange rivets, these stresses being most effectively transmitted by two pairs of horizontal plates next to the flange angles to resist bending and one pair of vertical plates to resist shearing stresses.

Bending Stress—Horizontal Plates,

$$\text{Moment of Resistance, } M = \frac{R h}{2} = \frac{A h f}{2}$$

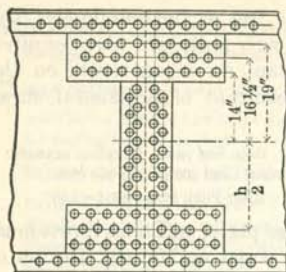
R = Total rivet value, one side of splice

A = Total sectional area of splice plate

h = Distance between centers

f = Fiber stress

The rivets are not equally stressed, the stress is zero at the neutral axis and increases uniformly to a maximum at extreme distance, d; the moment stress of each rivet is as its distance from neutral axis, and the moment of resistance as the square of its distance from neutral axis.



EXAMPLE. Required the web splice in a 48-inch girder with $\frac{3}{8}$ -inch web plate, capable of resisting the bending moment in web at 18000 pounds fiber stress. Horizontal plates to transmit bending stress in web, based on one-eighth of the web action.

Bending Moment in web plate:
 $(48 \times \frac{3}{8} \times \frac{1}{8}) \times 48 \times 18,000 = 1,944,000$ inch-pounds.

Investigating the value of rivets assumed:

Bearing value of $\frac{3}{4}$ " Dia. rivet:
 $\frac{3}{4} \times \frac{3}{8} \times 24,000 = 6750$ pounds.

Distance from neutral axis to top or bottom of girder = 24 inches.

Value of one rivet, one inch from neutral axis: $6750 \div 24 = 281$ pounds.

Moment of Resistance of rivets one side of joint above and below neutral axis is
 $M = 281(10 \times 14.0^2 + 8 \times 16.5^2 + 10 \times 19.0^2) = 2,178,000$ inch-pounds.

The resistance of plates should not be less than the required resistance of the rivets in same:

Stress in center of plates, $16\frac{1}{2}$ " from neutral axis:

$18,000 \times 16.5 \div 24 = 12,375$ lb. per sq. in.

Moment of resistance, when A is the total area of the two pairs of plates:

$12,375 \times A \times 16.5 = 204,200A$ $A = 1,944,000 \div 204,200 = 9.52$ sq. in.

Net Area of 4—8" x $\frac{7}{16}$ " plates: $A = (14.0 - 3.06) = 10.94$ sq. in.

Shearing Stress—Vertical Plates.

Shearing Resistance, $V = \frac{W}{R}$, W=Load producing shear in web.

EXAMPLE. The 13 rivets, $\frac{3}{4}$ " Dia., shown in vertical plates are sufficiently strong to resist in bearing a load of $13 \times \frac{3}{4} \times \frac{3}{8} \times 24,000 = 87,750$ pounds.

Flange Splices. Splices for flange angles and flange plates must develop the full bending moment of the girder at the joint.

Flange splices should be made at points of least flange stress, and joints in component parts of the flange should not be made at the same points.

EXAMPLE. Required top and bottom splices at separate points, for flange angles and inner flange plates as given; Rivets $\frac{3}{4}$ " Diameter.

2 Angles	6" x $3\frac{1}{2}$ " x $\frac{1}{2}$ "	Net Area 9.00—1.75=7.25 sq. in.
1 Inner Flange Plate	14" x $\frac{7}{16}$ "	" " 6.13—0.77=5.36 "
1 Outer Flange Plate	14" x $\frac{3}{8}$ "	" " 5.25—0.66=4.59 "

Splices for Flange Angles, Top and Bottom.

Stress in Angles 6" x $3\frac{1}{2}$ " x $\frac{1}{2}$ " 7.25 x 18,000=130,500 pounds.

Shearing Value of $\frac{3}{4}$ " Dia. Rivet: 0.4418 x 12000=5300 pounds.

Rivets required: $130,500 \div 5300 = 25$ rivets in single shear, each side of joint.

Splice: 1—plate 14" x $\frac{7}{16}$ ", 18 rivets, resistance 18 x 5300 = 95,400 pounds.

2—plates 3" x $\frac{9}{16}$ ", 4 " " 4 x 10600 = 42,400 "

Total resistance of angle splice: 137,800 "

Total Area of splice should not be less than that of the angles:

$(6.13 - 0.77) + (3.38 - 0.98) = 7.76$ sq. in.

Resistance of the two side plates not less than that of the 4 rivets in double shear,

4 x 10,600 = 42,400 pounds;

2—plates 3" x $\frac{9}{16}$ ", net area 2.40 x 18,000 = 43,200 pounds.

Splices for Inner Flange Plates, Top and Bottom.

Stress in Inner Flange Plate 14" x $\frac{7}{16}$ " net area 5.36 x 18,000 = 96,480 pounds.

Rivets required: $96,480 \div 5300 = 19$ rivets in single shear, each side of joint.

Splice: 1—plate 14" x $\frac{7}{16}$ ", 20 rivets, resistance 20 x 5300 = 106,000 pounds.

RIVETED GIRDERS

ALLOWABLE WEB SHEAR, V/A FOR VARIOUS RATIOS OF h/t

h/t	V/A	h/t	V/A	h/t	V/A	h/t	V/A	h/t	V/A	h/t	V/A
60	12000	70	10711	80	9529	90	8471	100	7535	110	6715
61	11867	71	10587	81	9418	91	8372	101	7448	115	6345
62	11734	72	10465	82	9308	92	8274	102	7362	120	6000
63	11604	73	10344	83	9199	93	8177	103	7277	125	5678
64	11473	74	10224	84	9091	94	8082	104	7194	130	5378
65	11343	75	10105	85	8984	95	7988	105	7111	135	5098
66	11215	76	9988	86	8879	96	7895	106	7030	140	4836
67	11087	77	9871	87	8775	97	7803	107	6950	145	4592
68	10961	78	9756	88	8672	98	7712	108	6870	150	4364
69	10835	79	9642	89	8571	99	7623	109	6792	155	4151
										160	3951

Ratio h/t=Distance between Flanges÷Thickness of Web Plate, inches.

Ratio V/A=Vertical Shear, pounds÷Gross Area of Web, sq. inches

GENERAL REQUIREMENTS FOR RIVETING.

1. In proportioning rivets the nominal diameter of the rivet shall be used, and in deducting rivet holes they shall be taken $\frac{1}{8}$ inch greater than the nominal diameter of the rivets.

2. The minimum distance between centers of rivet holes shall be three diameters of the rivet, but the distance shall preferably be not less than:

$4\frac{1}{4}" - 1\frac{1}{2}"$ rivets	$3\frac{1}{2}" - 1"$ rivets	$2\frac{1}{2}" - \frac{3}{4}"$ rivets	$1\frac{3}{4}" - \frac{1}{2}"$ rivets
$4" - 1\frac{1}{8}"$ "	$3" - \frac{7}{8}"$ "	$2" - \frac{5}{8}"$ "	

3. The maximum pitch in the line of stress of compression members composed of plates and shapes shall not exceed 16 times the thinnest outside plate or shape, nor 20 times the thinnest enclosed plate with a maximum of 12 inches, and at right angles to the direction of stress the distance between lines of rivets shall not exceed 30 times the thinnest plate or shape.

4. For angles in built-up sections with two gage lines, with rivets staggered, the maximum pitch in the line of stress in each gage line shall not exceed 24 times the thinnest plate, with a maximum of 18 inches.

5. The minimum distance from the center of any rivet hole to a sheared edge shall be:

$2\frac{1}{4}" - 1\frac{1}{2}"$ rivets	$1\frac{3}{4}" - 1"$ rivets	$1\frac{1}{4}" - \frac{3}{4}"$ rivets	$1" - \frac{1}{2}"$ rivets
$2" - 1\frac{1}{8}"$ "	$1\frac{1}{2}" - \frac{7}{8}"$ "	$1\frac{1}{8}" - \frac{5}{8}"$ "	

The maximum distance from any edge shall be 8 times the thickness of the plate.

6. The pitch of the rivets at the end of built compression members shall not exceed 4 times the diameters of the rivets for a length equal to $1\frac{1}{2}$ times the maximum width of the member.

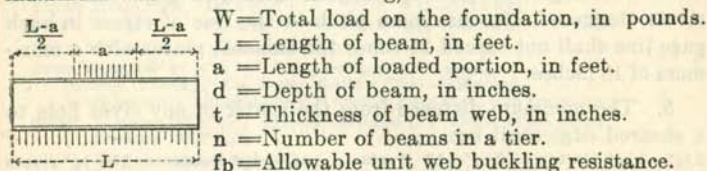
GRILLAGE FOUNDATIONS

Grillage Beams. In the design of foundations for columns, piers and walls, provision must be made for the uniform distribution of the load over the footing. This is best done by the use of a grillage of steel beams and concrete. This method of construction eliminates deep excavations and large masses of masonry and is, therefore, truly economical. For heavy loads on soils of small bearing capacity, three tiers of beams may be necessary; while for lighter loads or better soils two tiers, or even one, may suffice.

The lower tier should rest upon a solid bed of concrete of sufficient thickness to distribute the load to the soil. Good practice requires the spaces between the beams in all the tiers to be filled with, and the beams enclosed in, concrete not less than four inches thick.

The clear distance between the flanges of the beams in each tier should not be less than $2\frac{1}{2}$ inches, nor more than three times the flange width. The first requirement is necessary to permit the introduction and proper tamping of the concrete, the second, to insure uniform distribution of the load. When separators are used to hold the beams in position, they should be of gas pipe, as cast iron separators tend to break the continuity of the concrete. Grillage beams should not be painted, as concrete does not adhere well to painted surfaces but is itself an excellent preservative of steel.

To determine the area in square feet required for the foundation, divide the total load on the column, pier or wall by the allowable pressure per square foot on the soil. This gives the area of the footing, the shape of which is determined by local conditions. On the assumption that the loads on the soil are uniformly distributed, the number, size and weight of the beams required are determined from the maximum bending moment, the maximum shear, or the maximum web resistance to buckling, as follows:



The maximum bending moment occurs at the center of the beam and is equal in foot pounds to $W(L-a) + 8$; this formula is identical with the formula of maximum bending moment for a beam of length $(L-a)$ under a uniformly distributed load, W .

The proper size of beam in any tier as regards flexure at a fiber stress of 18,000 or 16,000 pounds per square inch may be found

GRILLAGE FOUNDATIONS

in the beam safe load tables for length corresponding to $(L-a)$, by dividing the total load by the number of beams.

Or may be found from the table of maximum bending moments, by dividing the total bending moment by the number of beams;

Or from the table of properties, by dividing by the number of beams in the tier the total section modulus required, which is equal to $\frac{3W(L-a)}{2f}$

Note, however, that the load on the beam for any span must not exceed the maximum tabular safe load for shear.

The maximum vertical shear occurs at the edge of the column base or at a distance in feet of $\frac{L-a}{2}$ from each end of the beam and is equal to: $V = \frac{W}{L} \times \frac{L-a}{2}$

Web thickness to resist average shear: $= \frac{W}{L} \times \frac{L-a}{2} \times \frac{1}{n \times d \times f_s}$,

Unit average vertical shear: $f_s = \frac{W}{L} \times \frac{L-a}{2} \times \frac{1}{n \times d \times t}$,

which must not exceed the allowable unit shearing stress.

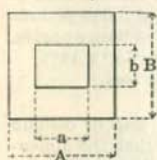
The maximum buckling stress occurs on a length in inches of $12a + d/2$ and is equal in total per lineal inch of web to $\frac{W}{12a + d/2}$.

Web thickness to resist buckling: $t = \frac{W}{n \times (12a + d/2) \times f_b}$.

Unit average web resistance to buckling: $f_b = \frac{W}{n \times (12a + d/2) \times t}$

which must not exceed the allowable buckling resistance.

Rolled Steel Slabs. To distribute the loads from columns over girders, grillage beams, etc., solid slabs of rolled steel may be advantageously used in the place of cast iron or riveted steel bases, etc. The size of the slab is usually fixed by the dimensions of the column and its thickness is determined from the maximum bending moment, on the assumption of uniform loading, as follows:



W = Total load, in pounds.

A = Width of slab, in inches.

B = Length of slab, in inches.

t = Thickness of slab, in inches.

a = Outside dimension of column, in inches.

b = Outside dimension of column, in inches.

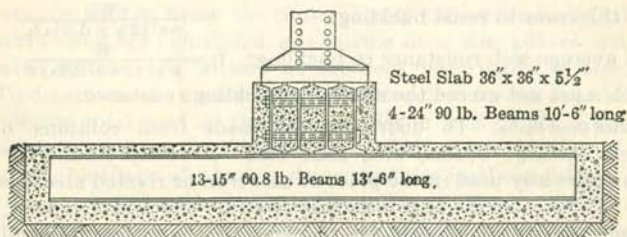
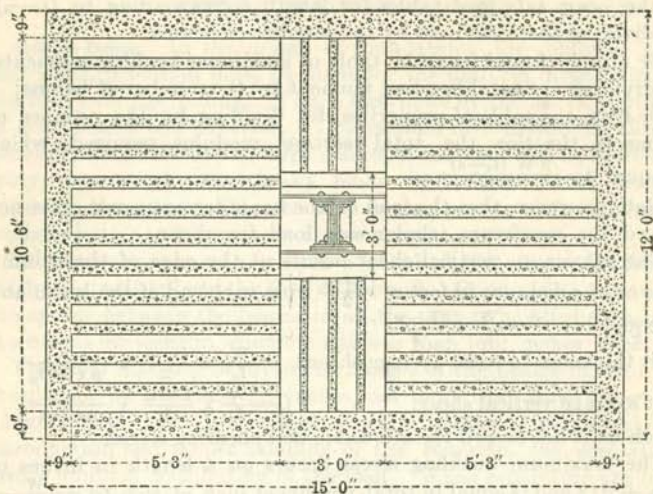
Assuming the maximum bending moment to be at center of slab:

$M = \frac{W(A-a)}{8}$ or $\frac{W(B-b)}{8}$, and the required thickness of slab:

$$t = \sqrt{\frac{3W(A-a)}{4fB}} \quad \text{or} \quad \sqrt{\frac{3W(B-b)}{4fA}}$$

Additional data are given under separate heading, "Strength of Steel Slabs".

NOTE—In the examples which follow a fiber stress of 16,000 pounds is used.

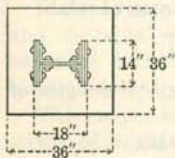


EXAMPLE: Required to design a grillage foundation for a column load of 1,040,000 pounds on soil with an allowable bearing capacity of 6,000 pounds per square foot. Column composed of 1 web plate, 14" x 5/8", 4 flange angles, 6" x 4" x 5/8" and 4 flange plates, 14" x 7/8", outside dimensions 14" x 18".

Required area of footing = $1,040,000 \div 6,000 = 173.33$ square feet.

Use area 12'-0" x 15'-0" = 180 square feet.

Assume 3'-0" square as the dimensions of the rolled steel slab or column base and allow 9" for concrete on the sides and ends of beams, then the dimensions of the steel grillage will be 10'-6" x 13'-6", concrete being assumed of sufficient thickness and strength to distribute to the edges.



Rollled Steel Slab

Thickness required, $t = \sqrt[3]{\frac{3 \times 1,040,000 \times 22}{64,000 \times 36}} = 5.46$ in.

Use 5 1/2".

GRILLAGE FOUNDATIONS

Beams—Section Modulus Method.

Bottom tier— $L=13.5$ feet; $a=3.0$ feet.

Required total section modulus, $S_s = \frac{3 \times 1,040,000 \times 10.5}{32,000} = 1,023.75 \text{ in.}^3$

Use 13—15" 60.8 lb. beams—Total section modulus=1,055.6 in.³

Average shear = $\frac{1,040,000}{13.5} \times \frac{10.5}{2} \times \frac{1}{13 \times 15 \times .59} = 3,515 \text{ lbs. per sq. in.}$

Average buckling stress = $\frac{1,040,000}{13 \times 43.5 \times .59} = 3,120 \text{ lbs. per sq. in.}$

Top tier— $L=10.5$ feet; $a=3.0$ feet.

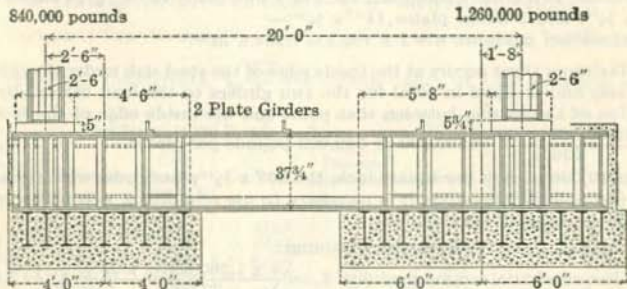
Required total section modulus, $S_s = \frac{3 \times 1,040,000 \times 7.5}{32,000} = 731.25 \text{ in.}^3$

Use 4—24" 90 lb. beams—Total section modulus=743.2 in.³

Average shear = $\frac{1,040,000}{10.5} \times \frac{7.5}{2} \times \frac{1}{4 \times 24 \times .624} = 6,200 \text{ lbs. per sq. in.}$

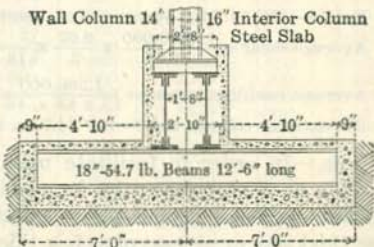
Average buckling stress = $\frac{1,040,000}{4 \times 48 \times .624} = 8,680 \text{ lbs. per sq. in.}$

Plate Girder Grillage Foundations. In those cases where columns carry very heavy loads, plate girders are used for the top tier of the grillage rather than beams. In the case of symmetrical foundations, the method of computation is the same as has already been illustrated in the case of beams. The following example indicates the procedure in the quite frequent case of unsymmetrical loading conditions:



- Make up of 1 Plate Girder
 4 Flange Angles $6 \times 4 \times \frac{5}{8}$
 2 Flange Plates $14 \times \frac{5}{8}$
 1 Web Plate $36 \times \frac{1}{2}$
 2 Web Reinf. Plates $\frac{3}{8}$ thick, each end between Flange Angles
 Web Reinf. Plates $\frac{3}{8}$ thick, each end over Flange Angles
 Stiffener Angles $5 \times 3 \frac{1}{2} \times \frac{1}{4}$
 Tie Angles $5 \times 3 \frac{1}{2} \times \frac{1}{4}$

Wall Column $14'' \times 16''$ Interior Column
 Steel Slab



CARNEGIE STEEL COMPANY

EXAMPLE:—Required to design a grillage foundation under an exterior or wall column carrying a load of 840,000 pounds, and an interior column with a load of 1,260,000 pounds, on soil with an allowable bearing capacity of 8,000 pounds per square foot.

$$\text{Required footing area of wall column} = \frac{840,000}{8,000} = 105 \text{ square feet.}$$

Use area 8'-0" x 14'-0" = 112 square feet.

$$\text{Required area of interior column footing} = \frac{1,260,000}{8,000} = 157.5 \text{ square feet.}$$

Use area 12'-0" x 14'-0" = 168 square feet.

With these dimensions and areas, the load on the soil will be uniform at 7,500 pounds per square foot, and the footings the same width, both of which are desirable from the standpoint of uniform settlement.

Rolled Steel Slabs for Column Footings: Assume a width of 30" and a length of 32", then the required thickness will be as follows:—

$$\text{Wall column, } t = \sqrt{\frac{3 \times 840,000 \times (32 - 14)}{64,000 \times 30}} = 4.86 \text{ in.; use } 5''.$$

$$\text{Interior column, } t = \sqrt{\frac{3 \times 1,260,000 \times (32 - 16)}{64,000 \times 30}} = 5.61 \text{ in.; use } 5\frac{3}{4}''.$$

Plate Girders: Maximum bending moment occurs at the inner beams of the respective footings, and is equal to the load on the column multiplied by the distance of its center from the center of moments.

$$M \text{ max. from wall column} = 840,000 \times 2'-6'' = 2,100,000 \text{ foot pounds.}$$

$$M \text{ max. from interior column} = 1,260,000 \times 1'-8'' = 2,100,000 \text{ foot pounds.}$$

$$\text{Required section modulus of two girders} = \frac{2,100,000 \times 12}{16,000} = 1,575.0 \text{ in.}^3$$

Assume two girders composed each of 1 web plate 36" x $\frac{1}{2}$ ", 4 angles 6" x 4" x $\frac{5}{8}$ ", and 2 flange plates 14" x $\frac{5}{8}$ ";—

Total section modulus, $S = 2 \times 792.3 = 1,584.6 \text{ in.}^3$

Maximum shear occurs at the inside edge of the steel slab under the interior column, and is equal in total for the two girders to the load carried by the portion of the footing between that point and the inside edge of the footing, or $\frac{1,260,000 \times 68}{126} = 680,000$ or 340,000 pounds per girder.

At 10,000 pounds per square inch, the 36" x $\frac{1}{2}$ " plate girder web is good for 180,000 pounds; therefore, it is necessary to use reinforcing web plates where the shear exceeds that amount.

Beams, Lower Tier, Interior Column:

$$\text{Required total section modulus, } S = \frac{3 \times 1,260,000 \times 9.67}{32,000} = 1,142.3 \text{ in.}^3$$

Use 13-18" 54.7 lb. beams — Total section modulus = 1,149.2 in.³

$$\text{Average shear} = \frac{1,260,000}{12.5} \times \frac{9.67}{2} \times \frac{1}{13 \times 18 \times .46} = 4,520 \text{ lbs. per sq. in.}$$

$$\text{Average buckling stress} = \frac{1,260,000}{13 \times 43 \times .46} = 4,900 \text{ lbs. per sq. in.}$$

For exterior column use 9-18" 54.7 lb. beams.

NOTE.—In order to facilitate manufacture and shipment, it is desirable to use for the entire foundation as few sizes and weights of beams as possible, and the rolled steel slabs should be of the same thickness or at least of as few thicknesses as really convenient.

GRILLAGE FOUNDATIONS

Strength of Steel Slabs. The formulas given for the bending moments in grillage beams and column base plates in the preceding pages are based on the assumption that the maximum bending moment occurs at the center of the column and grillage beams, producing also a deflection at this point. If, however, it is assumed that the column remains in perfect contact with the comparatively rigid base plate or slab, the latter may be considered rigidly fixed at the column edges, so that there is no bending moment in the center and that the maximum bending moment and also the maximum shear occurs at the fixed end of greatest cantilever projection, $\frac{B-b}{2}$.

The use of the formulas based upon the first assumption is recommended for the reason that maximum bending moment and maximum shearing stress do not occur at the same point and that, with particular reference to grillage beams, the bending stresses alone will generally decide the section to be selected.

The formulas based upon the second assumption give somewhat lower values for maximum bending moment and are frequently used in computing the resistance and the required thickness of heavy steel slabs; in which case, the corresponding values for M and t are:

$$M = \frac{w A B}{2} \times \frac{B-b}{B} \times \frac{B-b}{4} = \frac{w A (B-b)^2}{8} = \frac{f A t^2}{6}, \quad t = \frac{B-b}{2} \sqrt{\frac{3w}{f}}$$

The table gives the coefficients $\sqrt{\frac{3w}{f}}$ for various values of w.

Thickness of Slab = Maximum Projection \times Coefficient for corresponding Unit Pressure.

Projection Coefficients, $\sqrt{\frac{3w}{f}}$ for various values of Unit Pressure, w.

Unit Pressure lb. per sq. in.	Fiber Stress, 18,000 Pounds			Unit Pressure lb. per sq. in.	Fiber Stress, 16,000 Pounds		
	Coefficient	Unit Pressure lb. per sq. in.	Coefficient		Coefficient	Unit Pressure lb. per sq. in.	Coefficient
100	0.1291	1050	0.4183	100	0.1369	1050	0.4437
150	0.1581	1100	0.4282	150	0.1677	1100	0.4542
200	0.1826	1200	0.4472	200	0.1936	1200	0.4743
250	0.2041	1250	0.4564	250	0.2165	1250	0.4841
300	0.2236	1300	0.4655	300	0.2372	1300	0.4937
350	0.2415	1350	0.4743	350	0.2562	1350	0.5031
400	0.2582	1400	0.4830	400	0.2739	1400	0.5123
450	0.2739	1450	0.4916	450	0.2905	1450	0.5214
500	0.2887	1500	0.5000	500	0.3062	1500	0.5303
550	0.3028	1550	0.5083	550	0.3211	1550	0.5391
600	0.3162	1600	0.5164	600	0.3354	1600	0.5477
650	0.3291	1650	0.5244	650	0.3491	1650	0.5562
700	0.3416	1700	0.5323	700	0.3623	1700	0.5646
750	0.3536	1750	0.5401	750	0.3750	1750	0.5728
800	0.3652	1800	0.5478	800	0.3873	1800	0.5810
850	0.3764	1850	0.5553	850	0.3992	1850	0.5890
900	0.3873	1900	0.5627	900	0.4108	1900	0.5969
950	0.3979	1950	0.5701	950	0.4221	1950	0.6047
1000	0.4082	2000	0.5774	1000	0.4330	2000	0.6124

COLUMNS AND STRUTS

A compression member, subjected to longitudinal pressure, is shortened by the compression and also tends to deflect laterally, due to the fact that the load cannot be applied coincident with the longitudinal axis and that the material is not perfectly homogeneous. This flexure occurs generally in the direction of the least resisting moment of the section; the load which will cause a column to fail decreases in the ratio of length to least lateral resistance of the section, the ultimate failure being the result of combined stresses due to compression, transverse shear and flexure.

Column Formulas. Under ideal conditions, when it can be assumed that the load is applied axially and that the material is perfectly homogeneous, the resistance of the column would equal its resistance to compressive forces up to the elastic limit, and there would not be any flexure; if, however, a deflection be imparted to the column by a lateral force, the column would ultimately fail by bending.

Euler's Formula, $P = k \frac{\pi E I}{l^2}$ or $\frac{P}{A} = k \frac{\pi E}{(l/r)^2}$, is based upon the foregoing theory, and gives results close to the ultimate strength found for long and slender struts, when k is a constant varying with the condition of end bearing, ($k=4$ for columns fixed both ends). For shorter and heavier columns, or for lower ratios of l/r the results do not correspond with actual tests.

Rankine's Formula, $P = \frac{Af}{1 + c(l/r)^2} = \frac{P}{A} = \frac{f}{1 + c(l/r)^2}$, represents the type of formula now in general use and the numerous formulas for proportioning columns which are based upon this general formula agree with actual tests within certain limits. In this formula a certain compressive unit stress for direct crushing is assumed and reduced in ratio of length of column and least radius of gyration, l/r ; value of c is an empirical factor, varying with the resistance of the material and with conditions of end bearing.

Straight Line Formulas. In practice compression members of a greater ratio of slenderness, l/r , than 120 are rarely used, and within this limit the curve can be represented by a straight line, the general formula assuming the simpler form: $\frac{P}{A} = f - c \left(\frac{l}{r}\right)$.

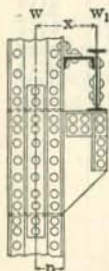
Compression formulas determining the resistance of webs in rolled beams or riveted girders against buckling, or the necessary reduction of safe loads due to lateral deflection of unbraced beams, are likewise based on one or the other type of column formulas.

Ratio of Slenderness. The ratio of slenderness, l/r , is ratio of the unsupported length of a compression member to its radius of gyration, generally the least radius, excepting when the unsupported length is rigidly braced in such a manner as to prevent deflection in the direction which corresponds to the least radius of gyration. Compression members, excepting those of square or circular section, have two principal radii of gyration. It is, therefore, necessary to determine the radii of gyration of such sections and to use the proper ratio of slenderness in any particular case.

Usual practice limits the maximum ratio of l/r to 120 for main members under steady stresses. For secondary members under temporary stress, such as those used in wind bracing, higher ratios may be used, but in no case should the ratio exceed 200.

Compressive Unit Stresses. Allowable unit stresses for ratios of slenderness, l/r , in accordance with a number of compression formulas of a more general use, for columns and struts, are given in table.

Combined Bending and Compression Stresses. Generally the loads are direct and equally distributed over the cross section of the column or balanced on opposite sides thereof. In the case of beams carried on brackets or other forms of eccentric loading, bending stresses are produced which should be taken into consideration and the column sections so proportioned that the combined stresses do not exceed the allowable axial compressive stress.



- W = Direct load W_1 = Eccentric load.
- M = Bending moment due to eccentric load = W_1x .
- I = Moment of inertia in direction of bending.
- n = Extreme fiber distance in direction of bending.
- A = Area of column section, in square inches.
- f = Allowable axial unit compression; f should be equal to or greater than $\frac{W + W_1}{A} + \frac{Mn}{I}$, the fiber stresses due to compression and bending respectively.

EXAMPLE:—Required a plate and angle column, 20 feet long, to sustain a balanced load of 210,000 pounds and an eccentric load of 40,000 pounds applied 15 inches from the column center on axis 1-1.

Assume a section made up of 14"x $\frac{3}{8}$ " web plate, four angles 6"x4"x $\frac{7}{16}$ " and two flange plates 14"x $\frac{3}{8}$ ",

$$A = 32.47, I_{1-1} = 1351, r_{2-2} = 3.09, \quad l/r = 20 \times 12 \div 3.09 = 77.$$

$$\text{Actual fiber stress} = \frac{210,000 + 40,000}{32.47} + \frac{40,000 \times 15 \times 7.625}{1351} = 11,090 \text{ pounds.}$$

If this stress is not within the limits of the allowable fiber stress for ratio of $l/r = 77$, obtained from the column formula in use, another trial is made.

CARNEGIE STEEL COMPANY

COMPRESSION FORMULAS FOR STEEL COLUMNS

ALLOWABLE UNIT STRESSES IN POUNDS PER SQUARE INCH

Ratio	Rankine	A. I. S. C. 1923	A. S. C. E. 1923	A. R. E. A. 1920	New York 1917	Chicago 1919	Philadelphia 1919
$\frac{l}{r}$	Flat End Bearing	Buildings	Ry. Bridges	Ry. Bridges	Buildings	Buildings	Buildings
	$1 + \frac{l^2}{36000r^2}$	$1 + \frac{l^2}{18000r^2}$	$1 + \frac{l^2}{13500r^2}$	$16000 - 70 \frac{l}{r}$	$16000 - 70 \frac{l}{r}$	$16000 - 70 \frac{l}{r}$	$1 + \frac{l^2}{11000r^2}$
0	12500				16000		16250
5	12491				15650		16213
10	12465				15300		16103
15	12422				14950		15924
20	12363			Maximum	14600	Maximum	15680
25	12287			14000	14250	14000	15376
30	12195			13900	13900	13900	15021
35	12089		Maximum	13550	13550	13550	14622
40	11968		14305	13200	13200	13200	14187
45	11834		13913	12850	12850	12850	13724
50	11688		13500	12500	12500	12500	13241
55	11531	Maximum	13071	12150	12150	12150	12745
60	11364	15000	12631	11800	11800	11800	12243
65	11187	14578	12186	11450	11450	11450	11741
70	11002	14148	11739	11100	11100	11100	11242
75	10811	13714	11294	10750	10750	10750	10752
80	10613	13279	10854	10400	10400	10400	10274
85	10410	12844	10422	10050	10050	10050	9808
90	10204	12414	10000	9700	9700	9700	9358
95	9995	11989	9590	9350	9350	9350	8926
100	9784	11571	9192	9000	9000	9000	8512
105	9571	11163	8808	8650	8650	8650	8116
110	9356	10764	8438	8300	8300	8300	7738
115	9142	10376	8082	7950	7950	7950	7378
120	8929	10000	7742	7600	7600	7600	7037
125	8717	9636	7416			7250	6714
130	8507	9284	7105			6900	6407
135	8299	8944	6809			6550	6116
140	8094	8617	6526			6200	5842
145	7892	8302	6256			5850	
150	7692	8000	6000			5500	
155	7496	7710	5756				
160	7305	7431	5524				
165	7118	7164	5303				
170	6934	6908	5094				
175	6754	6663	4895				
180	6579	6429	4706				
185	6408	6204	4526				
190	6242	5989	4355				
195	6080	5783	4192				
200	5921	5586	4037				

MAXIMUM RATIO OF $\frac{l}{r}$

Compression Formula	Members		Compression Formula	Members	
	Primary	Secondary		Primary	Secondary
Am. Inst. of Steel Construction	120	200	New York Bldg. Law 1917	120	120
Am. Society of Civil Engineers.	100	200	Chicago Bldg. Law 1919	120	150
Am. R'y Engineering Ass'n . . .	100	120	Philadelphia Bldg. Law . . . 1919	140	140

STRESSES IN RIVETS AND PINS

Rivets. In transmitting stresses between riveted pieces, it is customary to disregard friction and to proportion rivets to the entire stress to be transmitted. They must be of sufficient size and number to resist shear and to afford such bearing area as not to cause distortion of the metal at the rivet holes. In the case of beams which frame opposite and of single web girders, this latter condition often necessitates a greater thickness of web than required by the shearing stresses. In a plate girder with $\frac{5}{16}$ " web, $\frac{3}{4}$ " rivets connecting the web with the flange angles would have a bearing value at 24,000 pounds unit stress of 5,630 pounds per rivet, while their value in double shear at 12,000 pounds unit stress is 10,600 pounds per rivet; and it might be necessary to increase the web thickness to $\frac{3}{8}$ " or more in order that the pressure of the rivets upon the metal be not excessive.

Pins. Pins must be calculated for shearing, bending and bearing stresses, but one of the latter two will in most cases determine the size. When groups of bars are connected to the same pin, as in the lower chord of truss bridges, the size of the bars must be so chosen and the bars so placed that at no point on the pin will there be any excessive bending stress. When the size of pin has been determined from the bending stress, the thickness of the bars or web of the post should be investigated to provide sufficient bearing area, the bars being thickened or pin plates added if necessary.

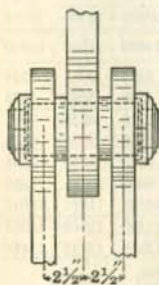
The following is the formula for flexure applied to pins: $M = f \pi d^3 + 32$ or $= f A d + 8$, in which M = moment of forces for any section through pin, f = fiber stress per square inch in bending, A = the area of section, d = diameter, $\pi = 3.14159$. The forces are assumed to act in a plane passing through the axis of the pin.

EXAMPLE 1.—Required the size of a pin carrying a load of 64,000 pounds, at a distance of 5 inches between points of support; maximum fiber stress 24,000 pounds per square inch.

Bending moment = $64,000 \times 5 + 4 = 80,000$ inch pounds; use a $3\frac{1}{4}$ inch pin; allowed moment: 80,900 inch pounds.

EXAMPLE 2.—Required the thickness of metal in the top chord of a bridge to give sufficient bearing area to a $3\frac{3}{8}$ -inch pin, having to transmit a stress of 121,400 pounds; maximum bearing pressure 24,000 pounds per square inch.

The bearing value of a $3\frac{3}{8}$ -inch pin for 1 inch thickness of metal is 81,000 pounds; therefore, the thickness of metal required = $121,400 \div 81,000 = 1\frac{1}{2}$ inch, or each web of the chord must be $\frac{3}{4}$ inch thick, including pin plates.



CARNEGIE STEEL COMPANY

RIVETS

SHEARING AND BEARING VALUES, IN POUNDS

1/2-INCH RIVETS—Area 0.19635 Square Inch

Shearing	Unit, Lbs. per Sq. In.		7500	8000	9000	10000	11000	12000	13500	15000	
	Single Shear per Rivet		1473	1571	1767	1964	2160	2356	2651	2945	
	Double Shear per Rivet		2945	3142	3534	3927	4320	4712	5301	5890	
Bearing	Unit, Lbs. per Sq. In.		15000	16000	18000	20000	22000	24000	27000	30000	
	Thickness, in Inches	1/8	938	1000	1125	1250	1375	1500	1688	1875	
		3/16	1406	1500	1688	1875	2063	2250	2531	2813	
		1/4	1875	2000	2250	2500	2750	3000	3375	3750	
		5/16	2344	2500	2813	3125	3438	3750	4219	4688	
		3/8	2813	3000	3375	3750	4125	4500	5062	5625	
		7/16	3281	3500	3938	4375	4813	5250	5906	6563	
		1/2	3750	4000	4500	5000	5500	6000	6750	7500	

5/8-INCH RIVETS—Area 0.30680 Square Inch

Shearing	Unit, Lbs. per Sq. In.		7500	8000	9000	10000	11000	12000	13500	15000	
	Single Shear per Rivet		2301	2454	2761	3068	3375	3682	4142	4602	
	Double Shear per Rivet		4602	4908	5522	6136	6750	7363	8284	9204	
Bearing	Unit, Lbs. per Sq. In.		15000	16000	18000	20000	22000	24000	27000	30000	
	Thickness, in Inches	1/8	1172	1250	1406	1563	1719	1875	2109	2344	
		3/16	1758	1875	2109	2344	2578	2813	3164	3516	
		1/4	2344	2500	2813	3125	3438	3750	4219	4688	
		5/16	2930	3125	3516	3906	4297	4688	5273	5859	
		3/8	3516	3750	4219	4688	5156	5625	6328	7031	
		7/16	4102	4375	4922	5469	6016	6563	7383	8203	
		1/2	4688	5000	5625	6250	6875	7500	8438	9375	
		5/8	5273	5625	6328	7031	7734	8438	9492	10547	

3/4-INCH RIVETS—Area 0.44179 Square Inch

Shearing	Unit, Lbs. per Sq. In.		7500	8000	9000	10000	11000	12000	13500	15000	
	Single Shear per Rivet		3313	3534	3976	4418	4860	5301	5964	6627	
	Double Shear per Rivet		6627	7069	7952	8836	9719	10603	11928	13254	
Bearing	Unit, Lbs. per Sq. In.		15000	16000	18000	20000	22000	24000	27000	30000	
	Thickness, in Inches	3/16	2109	2250	2531	2813	3094	3375	3797	4219	
		1/4	2813	3000	3375	3750	4125	4500	5063	5625	
		5/16	3516	3750	4219	4688	5156	5625	6328	7031	
		3/8	4219	4500	5063	5625	6188	6750	7594	8438	
		7/16	4922	5250	5906	6563	7219	7875	8859	9844	
		1/2	5625	6000	6750	7500	8250	9000	10125	11250	
		5/8	6328	6750	7594	8438	9281	10125	11391	12656	
		3/4	7031	7500	8438	9375	10313	11250	12656	14063	
		7/8	7734	8250	9281	10313	11343	12375	13922	15469	

Values above upper dotted lines are less than single shear.
 Values below lower dotted lines are greater than double shear.

STRESSES IN RIVETS AND PINS

RIVETS

SHEARING AND BEARING VALUES, IN POUNDS

 $\frac{3}{8}$ -INCH RIVETS—Area 0.60132 Square Inch

Shearing	Unit, Lbs. per Sq. In.	7500	8000	9000	10000	11000	12000	13500	15000	
	Single Shear per Rivet	4510	4811	5412	6013	6615	7216	8118	9020	
	Double Shear per Rivet	9020	9621	10824	12026	13229	14432	16236	18040	
Bearing	Unit, Lbs. per Sq. In.	15000	16000	18000	20000	22000	24000	27000	30000	
	Thickness, in Inches	$\frac{1}{4}$	3281	3500	3938	4375	4813	5250	5906	6563
		$\frac{5}{16}$	4102	4375	4922	5469	6016	6563	7383	8203
		$\frac{3}{8}$	4922	5250	5906	6563	7219	7875	8859	9844
		$\frac{7}{16}$	5742	6125	6891	7656	8422	9188	10336	11484
		$\frac{1}{2}$	6563	7000	7875	8750	9625	10500	11813	13125
		$\frac{9}{16}$	7383	7875	8859	9844	10828	11813	13289	14766
		$\frac{5}{8}$	8203	8750	9844	10938	12031	13125	14766	16406
		$\frac{11}{16}$	9023	9625	10828	12031	13234	14438	16242	18047
		$\frac{3}{4}$	9844	10500	11813	13125	14438	15750	17719	19688

1-INCH RIVETS—Area 0.78540 Square Inch

Shearing	Unit, Lbs. per Sq. In.	7500	8000	9000	10000	11000	12000	13500	15000	
	Single Shear per Rivet	5891	6283	7069	7854	8639	9425	10603	11781	
	Double Shear per Rivet	11781	12566	14137	15708	17279	18850	21206	23562	
Bearing	Unit, Lbs. per Sq. In.	15000	16000	18000	20000	22000	24000	27000	30000	
	Thickness, in Inches	$\frac{5}{16}$	4688	5000	5625	6250	6875	7500	8438	9375
		$\frac{3}{8}$	5625	6000	6750	7500	8250	9000	10125	11250
		$\frac{7}{16}$	6563	7000	7875	8750	9625	10500	11813	13125
		$\frac{1}{2}$	7500	8000	9000	10000	11000	12000	13500	15000
		$\frac{9}{16}$	8438	9000	10125	11250	12375	13500	15188	16875
		$\frac{5}{8}$	9375	10000	11250	12500	13750	15000	16875	18750
		$\frac{11}{16}$	10313	11000	12375	13750	15125	16500	18563	20625
		$\frac{3}{4}$	11250	12000	13500	15000	16500	18000	20250	22500
		$\frac{13}{16}$	12188	13000	14625	16250	17875	19500	21938	24375

 $1\frac{1}{8}$ -INCH RIVETS—Area 0.99402 Square Inch

Shearing	Unit, Lbs. per Sq. In.	7500	8000	9000	10000	11000	12000	13500	15000	
	Single Shear per Rivet	7455	7952	8946	9940	10934	11928	13419	14910	
	Double Shear per Rivet	14910	15904	17892	19880	21868	23856	26839	29821	
Bearing	Unit, Lbs. per Sq. In.	15000	16000	18000	20000	22000	24000	27000	30000	
	Thickness, in Inches	$\frac{3}{8}$	7383	7875	8859	9844	10828	11813	13289	14766
		$\frac{1}{2}$	8438	9000	10125	11250	12375	13500	15188	16875
		$\frac{9}{16}$	9492	10125	11391	12656	13922	15188	17086	18984
		$\frac{5}{8}$	10547	11250	12656	14063	15469	16875	18984	21094
		$\frac{11}{16}$	11602	12375	13922	15469	17016	18563	20883	23203
		$\frac{3}{4}$	12656	13500	15188	16875	18563	20250	22781	25313
		$\frac{13}{16}$	13711	14625	16453	18281	20109	21938	24650	27422
		$\frac{7}{8}$	14766	15750	17719	19688	21656	23625	26578	29531
		$\frac{15}{16}$	15820	16875	18984	21094	23203	25313	28477	31641

Values above upper dotted lines are less than single shear.
 Values below lower dotted lines are greater than double shear.

CARNEGIE STEEL COMPANY

PINS

BEARING VALUES ON METAL ONE INCH THICK, IN POUNDS

Dia. x 1 x Unit Stress

Pin		Fiber Stress in Pounds per Square Inch								
Dia., Inches	Area, Sq. In.	15000	16000	18000	20000	22000	24000	25000	27000	30000
1	.785	15000	16000	18000	20000	22000	24000	25000	27000	30000
1 1/4	1.227	18750	20000	22500	25000	27500	30000	31250	33750	37500
1 1/2	1.767	22500	24000	27000	30000	33000	36000	37500	40500	45000
1 3/4	2.405	26250	28000	31500	35000	38500	42000	43750	47250	52500
2	3.142	30000	32000	36000	40000	44000	48000	50000	54000	60000
2 1/4	3.976	33750	36000	40500	45000	49500	54000	56250	60750	67500
2 1/2	4.909	37500	40000	45000	50000	55000	60000	62500	67500	75000
2 3/4	5.940	41250	44000	49500	55000	60500	66000	68750	74250	82500
3	7.069	45000	48000	54000	60000	66000	72000	75000	81000	90000
3 1/4	8.296	48750	52000	58500	65000	71500	78000	81250	87750	97500
3 1/2	9.621	52500	56000	63000	70000	77000	84000	87500	94500	105000
3 3/4	11.045	56250	60000	67500	75000	82500	90000	93750	101250	112500
4	12.566	60000	64000	72000	80000	88000	96000	100000	108000	120000
4 1/4	14.186	63750	68000	76500	85000	93500	102000	106250	114750	127500
4 1/2	15.904	67500	72000	81000	90000	99000	108000	112500	121500	135000
4 3/4	17.721	71250	76000	85500	95000	104500	114000	118750	128250	142500
5	19.635	75000	80000	90000	100000	110000	120000	125000	135000	150000
5 1/4	21.648	78750	84000	94500	105000	115500	126000	131250	141750	157500
5 1/2	23.758	82500	88000	99000	110000	121000	132000	137500	148500	165000
5 3/4	25.967	86250	92000	103500	115000	126500	138000	143750	155250	172500
6	28.274	90000	96000	108000	120000	132000	144000	150000	162000	180000
6 1/4	30.680	93750	100000	112500	125000	137500	150000	156250	168750	187500
6 1/2	33.183	97500	104000	117000	130000	143000	156000	162500	175500	195000
6 3/4	35.785	101250	108000	121500	135000	148500	162000	168750	182250	202500
7	38.485	105000	112000	126000	140000	154000	168000	175000	189000	210000
7 1/4	41.282	108750	116000	130500	145000	159500	174000	181250	195750	217500
7 1/2	44.179	112500	120000	135000	150000	165000	180000	187500	202500	225000
7 3/4	47.173	116250	124000	139500	155000	170500	186000	193750	209250	232500
8	50.265	120000	128000	144000	160000	176000	192000	200000	216000	240000
8 1/4	53.456	123750	132000	148500	165000	181500	198000	206250	222750	247500
8 1/2	56.745	127500	136000	153000	170000	187000	204000	212500	229500	255000
8 3/4	60.132	131250	140000	157500	175000	192500	210000	218750	236250	262500
9	63.617	135000	144000	162000	180000	198000	216000	225000	243000	270000
9 1/4	67.201	138750	148000	166500	185000	203500	222000	231250	249750	277500
9 1/2	70.882	142500	152000	171000	190000	209000	228000	237500	256500	285000
9 3/4	74.662	146250	156000	175500	195000	214500	234000	243750	263250	292500
10	78.540	150000	160000	180000	200000	220000	240000	250000	270000	300000
10 1/4	82.516	153750	164000	184500	205000	225500	246000	256250	276750	307500
10 1/2	86.590	157500	168000	189000	210000	231000	252000	262500	283500	315000
10 3/4	90.763	161250	172000	193500	215000	236500	258000	268750	290250	322500
11	95.033	165000	176000	198000	220000	242000	264000	275000	297000	330000
11 1/4	99.402	168750	180000	202500	225000	247500	270000	281250	303750	337500
11 1/2	103.869	172500	184000	207000	230000	253000	276000	287500	310500	345000
11 3/4	108.434	176250	188000	211500	235000	258500	282000	293750	317250	352500
12	113.097	180000	192000	216000	240000	264000	288000	300000	324000	360000

STRESSES IN RIVETS AND PINS

PINS

BENDING MOMENTS IN THOUSANDS OF INCH POUNDS

Dia.³ x 0.098175 x Unit Stress

Pin		Fiber Stress in Pounds per Square Inch							
Dia., Inches	Area, Sq. In.	15000	16000	18000	20000	22000	24000	25000	27000
1	.785	1.5	1.6	1.8	2.0	2.2	2.4	2.5	2.7
1 1/4	1.227	2.9	3.1	3.5	3.8	4.2	4.6	4.8	5.2
1 1/2	1.767	5.0	5.3	6.0	6.6	7.3	8.0	8.3	8.9
1 3/4	2.405	7.9	8.4	9.5	10.5	11.6	12.6	13.2	14.2
2	3.142	11.8	12.6	14.1	15.7	17.3	18.8	19.6	21.2
2 1/4	3.976	16.8	17.9	20.1	22.4	24.6	26.8	28.0	30.2
2 1/2	4.909	23.0	24.5	27.6	30.7	33.7	36.8	38.3	41.4
2 3/4	5.940	30.6	32.7	36.8	40.8	44.9	49.0	51.0	55.1
3	7.069	39.8	42.4	47.7	53.0	58.3	63.6	66.3	71.6
3 1/4	8.296	50.6	53.9	60.7	67.4	74.1	80.9	84.3	91.0
3 1/2	9.621	63.1	67.3	75.8	84.2	92.6	101.0	105.2	113.7
3 3/4	11.045	77.7	82.8	93.2	103.5	113.9	124.3	129.4	139.8
4	12.566	94.2	100.5	113.1	125.7	138.2	150.8	157.1	169.6
4 1/4	14.186	113.0	120.6	135.7	150.7	165.8	180.9	188.4	203.5
4 1/2	15.904	134.2	143.1	161.0	178.9	196.8	214.7	223.7	241.6
4 3/4	17.721	157.8	168.3	189.4	210.4	231.5	252.5	263.0	284.1
5	19.635	184.1	196.4	220.9	245.4	270.0	294.5	306.8	331.3
5 1/4	21.648	213.1	227.3	255.7	284.1	312.5	340.9	355.2	383.6
5 1/2	23.758	245.0	261.3	294.0	326.7	359.3	392.0	408.3	441.0
5 3/4	25.967	280.0	298.6	336.0	373.3	410.6	447.9	466.6	503.9
6	28.274	318.1	339.3	381.7	424.1	466.5	508.9	530.1	572.6
6 1/4	30.680	359.5	383.5	431.4	479.4	527.3	575.2	599.2	647.1
6 1/2	33.183	404.4	431.4	485.3	539.2	593.1	647.1	674.0	728.0
6 3/4	35.785	452.9	483.1	543.5	603.9	664.3	724.6	754.8	815.2
7	38.485	505.1	538.8	606.1	673.5	740.8	808.2	841.8	909.2
7 1/4	41.282	561.2	598.6	673.4	748.2	823.1	897.9	935.3	1010.1
7 1/2	44.179	621.3	662.7	745.5	828.4	911.2	994.0	1035.4	1118.3
7 3/4	47.173	685.5	731.2	822.6	914.0	1005.4	1096.8	1142.5	1233.9
8	50.265	754.0	804.3	904.8	1005.3	1105.8	1206.4	1256.6	1357.2
8 1/4	53.456	826.9	882.0	992.3	1102.5	1212.8	1323.0	1378.2	1488.4
8 1/2	56.745	904.4	964.7	1085.3	1205.8	1326.4	1447.0	1507.3	1627.9
8 3/4	60.132	986.5	1052.3	1183.9	1315.4	1446.9	1578.5	1644.2	1775.8
9	63.617	1073.5	1145.1	1288.3	1431.4	1574.5	1717.7	1789.2	1932.4
9 1/4	67.201	1165.5	1243.2	1398.6	1554.0	1709.4	1864.8	1942.5	2097.9
9 1/2	70.882	1262.6	1346.8	1515.1	1683.5	1851.8	2020.1	2104.3	2272.7
9 3/4	74.662	1364.9	1455.9	1637.9	1819.9	2001.9	2183.9	2274.9	2456.8
10	78.540	1472.6	1570.8	1767.1	1963.5	2159.8	2356.2	2454.4	2650.7
10 1/4	82.516	1585.9	1691.6	1903.0	2114.5	2325.9	2537.4	2643.1	2854.5
10 1/2	86.590	1704.7	1818.4	2045.7	2273.0	2500.3	2727.6	2841.2	3068.5
10 3/4	90.763	1829.4	1951.4	2195.3	2439.2	2683.2	2927.1	3049.1	3293.0
11	95.033	1960.1	2090.7	2352.1	2613.4	2874.8	3136.1	3266.8	3528.1
11 1/4	99.402	2096.8	2236.5	2516.1	2795.7	3075.2	3354.8	3494.6	3774.2
11 1/2	103.869	2239.7	2389.0	2687.6	2986.2	3284.9	3583.5	3732.8	4031.4
11 3/4	108.434	2388.9	2548.2	2866.7	3185.3	3503.8	3822.3	3981.6	4300.1
12	113.097	2544.7	2714.3	3053.6	3392.9	3732.2	4071.5	4241.2	4580.5

FLOOR PLATES

FLAT RECTANGULAR PLATES

Rectangular steel plates, plain, checkered or indented, are frequently used in mill floor construction, supported by the floor beams on two sides or on all four sides and more or less securely fixed to the flanges of the supporting beams.

The resistance of rectangular plates to superimposed loads may be obtained from the formulas given below; the formulas given for plates supported on four sides apply generally to rectangular plates subjected to pressures normal to surface of plates.

- M = Bending moment, due to uniform or concentrated load, inch-pounds.
- f = Unit fiber stress, pounds per square inch.
- w = Unit load, pounds per square inch.
- a, b = Sides of plate, inches, (a < b) t = Thickness of plate, inches.
- c = Perpendicular distance, from corner to diagonal, d, of plate, inches.
- φ = Limiting values for steel plates, fixed and not fixed to supports (v. Bach).

Plate supported on two sides, a—Uniformly distributed load.

$$M = \frac{w a b^2}{8} = f S \qquad S = \frac{a t^2}{6} \qquad f = \frac{3}{4} b^2 \frac{w}{t^2}$$

Plate supported on four sides, a, b—Uniformly distributed load.

$$f = \phi \frac{1}{2} \frac{a^2 b^2}{a^2 + b^2} \frac{w}{t^2} \qquad f = \phi \frac{1}{2} c^2 \frac{w}{t^2} \qquad \phi = \frac{9}{16} \text{ to } \frac{3}{4}$$

Plate supported on four sides, a, b—Concentrated load in center.

$$f = \phi \frac{3}{8} \frac{ab}{a^2 + b^2} \frac{P}{t^2} \qquad f = \phi \frac{3}{8} \frac{c}{d} \frac{P}{t^2} \qquad \phi = 1\frac{1}{4} \text{ to } 1\frac{1}{2}$$

BUCKLE PLATES

Buckle plates are generally used on highway bridges with paved floors, and may be subjected to concentrated live loads, due to the weight of truck wheels and to a uniform load due to the paving.

The resistance of buckle plates, when the buckle is turned up and in compression may be computed from the formulas (Winkler):

Total uniformly distributed load

$$W = 4 f d t, \text{ pounds per buckle.}$$

Total concentrated load, in addition to uniform dead load.

$$P = \frac{t (100 f d t - 25.2 w a b)}{6 d + 15 t}, \text{ pounds per buckle}$$

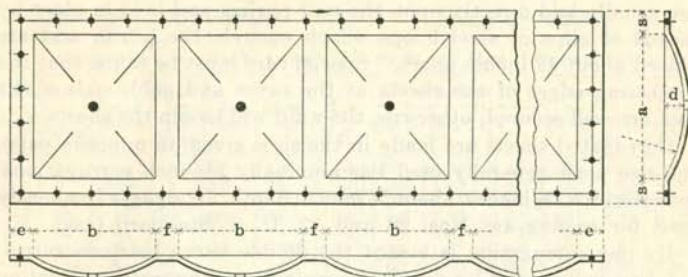
- a, b = Sides. t = Thickness. d = Rise of Buckle, inches.
- w = Unit load, pounds per square inch. f = Allowable fiber stress, 9000 lbs. per sq. inch.

Buckle plates are generally placed with the convex side of the buckle turned down and in tension, in which case the strength of the buckle plates is about three times greater.

FLOOR PLATES

BUCKLE PLATES—DIMENSIONS

AMERICAN BRIDGE COMPANY STANDARD



Size of Buckle, Inches			Die No.	Size of Buckle, Inches			Die No.	Size of Buckle, Inches			Die No.
a	b	d		a	b	d		a	b	d	
21	21	2½	39	36	36	2	13	45	37	3	6
23½	24	2½	38	36	37	3	29	45	42	3	24
23½	28½	2½	36	37	36	3	28	45	45	3	5
23½	47	2½	18	37	38	3	26	47	23½	2½	17
24	23½	2½	37	37	45	3	7	47	42	3	4
24	30	2½	30	38	37	3	27	47	54	3½	2
26	44	2	12	41	42	3	23	48	48	3	34
28½	23½	2½	35	42	41	3	22	51¼	61	4	16
30	24	2½	31	42	45	3	25	54	47	3½	1
30	30	2½	21	42	47	3	3	61	51¼	4	15
30	33	2½	20	42	66	3½	32	66	42	3½	33
32	44	2	10	44	26	2	11				
33	30	2½	19	44	32	2	9				
33	33	3	14	44	44	2	8				

MAXIMUM WIDTH = 94" for plates not over 12 ft. long. 71" for plates not over 35 ft. long.

MAXIMUM LENGTH = 35 ft. Plates of greater length can be obtained by splicing.

ALLOWABLE OVERRUN in length or width must be given on drawing, where clearance is close.

END FLANGES e = 2" Minimum 18" Maximum

SIDE FLANGES s = 2" Minimum 6½" Maximum 4" or less, preferred.

FILLETS f = 2" Minimum 6" Maximum 4" or less, preferred.

END FLANGES to be made alike if possible. If over 18", stiffen with angles across plate.

SIDE FLANGES to be made alike if possible. When side flanges must be of unequal width, the plate should be ordered wide enough to make two flanges of the greater width. After plate is buckled, it will be sheared to required width.

BUCKLES can be lengthwise or crosswise of plate, but different sizes should not be used in the same plate. Plates are buckled one buckle at a time, and the number of buckles is determined by size of buckles, fillets and end flanges, and by length of plate that can be fabricated.

A plate 35 ft. long could have:—14 buckles No. 11, b = 2'-2", with f = 3½" and e = 5¼"

or, 9 buckles No. 12, b = 3'-8", with f = 2¼" and e = 3"

CONNECTION HOLES are usually for ⅝", ¾" or ⅞" rivets or bolts. Holes of different sizes in the same plate increase the cost. SPACING: Crosswise, usually 6", with 4½" Min. Lengthwise, from 6" to 12". Odd spaces at ends, in even ¼".

DRAWING must show **Top View** of plate, give **Die Number**, and state whether buckles are turned **up** or **down**. When buckles are turned **down**, the drawing must show a **Drain Hole** in the center of each buckle.

CORRUGATED SHEETS

Corrugated sheets are used for roofs and sides of buildings. They are usually laid directly upon the roof purlins and held in place by means of clips of steel hoops which encircle the purlin and are placed about 12 inches apart. Special care must be taken that the projecting edges of the sheets at the eaves and gable ends of the roof are well secured, otherwise the wind will loosen the sheets.

Corrugated sheets are made in the sizes given on opposite page, the size most generally used has nominally 2½-inch corrugations, actual width 2⅝ inches, about ½ inch in depth. The gages frequently used for roofing are Nos. 20 and 22, U. S. Standard Gage.

By one corrugation is meant the double curve between corresponding points, and by depth of corrugation the greatest deviation of the curved surfaces from the straight line.

One and one-half corrugations are allowed for lap in the width of the sheet and 6 inches in the length for the usual quarter pitch roof; one corrugation in width and 4 inches in the length of the sheet is usually allowed for sidings.

Corrugated sheets of 2, 2½ and 3 corrugations are furnished in standard lengths of 5, 6, 7, 8, 9 and 10 feet and with a standard covering width of 24 inches, when laid with a lap of either one or one and one-half corrugations.

By experiment it has been determined that corrugated sheet steel, ⅝ inch deep and No. 20 gage spanning 6 feet, began to give a permanent deflection with a load of 30 pounds per sq. foot, and that it collapsed with a load of 60 pounds per sq. foot. The distance between centers of purlins should, therefore, not exceed 6 feet and should preferably be less than this.

Approximately the uniformly distributed safe load of corrugated sheets may be obtained from the formulas given below, using the following notations:—

W=Total allowable uniform load, in pounds.

b=Curvilinear width of sheet, in inches,
=1.075 x covering width.

l=Unsupported length of sheet, in inches.

t=Thickness of sheet, in inches.

d=Depth of corrugations, in inches.

f=Allowable fiber stress, in pounds per sq. inch.

$$\text{Then: } W = \frac{8fS}{1} = \frac{8f}{1} \times \frac{4bdt}{15} = \frac{32fbd}{15l}$$

$$\text{for } f = 10000, \quad W = \frac{21,333 \text{ bdt}}{1}$$

CORRUGATED SHEETS

CORRUGATED SHEETS—DIMENSIONS

AMERICAN SHEET AND TIN PLATE COMPANY

DESCRIPTION OF SHEETS

AREAS OF SHEETS

Corrugations			Width, Inches		Length of Sheet, Inches	Sq. Ft. in 1 Sheet			Sheets in 100 Sq. Ft.			
Nominal	Actual	Depth, Inches	Number per Sheet	Full Sheet		Covering	Corrugations			Corrugations		
							5"	3" 2 1/2"	1 1/8"	5"	3" 2 1/2"	1 1/8"
5	5	7/8	6	28	25	60	11.67	10.83	10.42	8.57	9.23	9.60
3	3	9/16	9	26	24	72	14.00	13.00	12.50	7.14	7.69	8.00
*2 1/2	2 2/3	1/2	10 1/2	27 1/2	24	84	16.33	15.17	14.58	6.12	6.59	6.86
2 1/2	2 2/3	1/2	10	26	24	96	18.67	17.33	16.67	5.36	5.77	6.00
2	2	7/16	13	26	24	108	21.00	19.50	18.75	4.76	5.13	5.33
1 1/4	1 1/4	3/8	20	25	23 3/4	120	23.33	21.67	20.83	4.29	4.62	4.80
5/8	5/8	3/16	40	25	24 3/4	144	28.00	26.00	25.00	3.57	3.85	4.00

Standard lengths 5, 6, 7, 8, 9 and 10 ft. Maximum length, 12 ft. except for 5/8" corrugation. Sizes denoted *2 1/2 are for the 27 1/2" width.

PAINTED SHEETS—Weights in Pounds per 100 Square Feet.

Corrug., In.	Thickness, United States Standard Gage														
	10	12	14	16	18	20	21	22	23	24	25	26	27	28	29
5		470	336	269	215	162	148	135	122	108	95	81	75	68	
3		472	338	270	216	163	149	136	122	109	95	82	75	68	..
*2 1/2	615	478	342	274	219	165	151	137	124	110	97	83	76	69	..
2 1/2	607	472	338	270	216	163	149	136	122	109	95	82	75	68	..
2				270	216	163	149	136	122	109	95	82	75	68	..
1 1/4						169	155	141	127	113	99	85	78	71	..
5/8										113	99	85	78	71	..

GALVANIZED SHEETS—Weights in Pounds per 100 Square Feet.

Corrug., In.	Thickness, United States Standard Gage														
	10	12	14	16	18	20	21	22	23	24	25	26	27	28	29
5		486	352	285	231	178	164	151	137	124	111	97	90	84	77
3		488	353	286	232	178	165	151	138	125	111	98	91	84	77
*2 1/2	631	494	358	290	235	181	167	153	140	126	113	99	92	85	78
2 1/2	623	488	353	286	232	178	165	151	138	125	111	98	91	84	77
2				286	232	178	165	151	138	125	111	98	91	84	77
1 1/4						186	172	158	144	130	116	102	95	88	81
5/8										130	116	102	95	88	81

The weights per 100 square feet given in preceding tables do not include allowances for end or side laps. The following table gives the approximate number of square feet of sheeting necessary to cover an area of 100 square feet and is based on sheets of standard width, 96 inches long. If longer or shorter sheets are used, the number of square feet required will vary accordingly.

SQ. FEET OF 2 1/2 IN. STANDARD SHEETS TO COVER AREA OF 100 SQ. FT.

Side Lap	End Lap, Inches					
	1	2	3	4	5	6
1 Corrugation	109	111	112	113	114	116
1 1/2 "	116	117	118	120	121	122
2 "	123	124	126	127	129	130

CARNEGIE STEEL COMPANY

PIPE—BLACK AND GALVANIZED

NATIONAL TUBE COMPANY STANDARD

STANDARD PIPE

Size, In.	Diameters, Inches		Thick- ness, Inches	Weight per Foot, Pounds		Threads per Inch	Couplings		
	External	Internal		Plain Ends	Threads and Couplings		Diameter, Inches	Length, Inches	Weight, Pounds
1/8	.405	.269	.068	.244	.245	27	.562	7/8	.029
1/4	.540	.364	.088	.424	.425	18	.685	1	.043
3/8	.675	.493	.091	.567	.568	18	.848	1 1/8	.070
1/2	.840	.622	.109	.850	.852	14	1.024	1 1/4	.116
3/4	1.050	.824	.113	1.130	1.134	14	1.281	1 3/8	.209
1	1.315	1.049	.133	1.678	1.684	11 1/2	1.576	1 3/4	.343
1 1/4	1.660	1.380	.140	2.272	2.281	11 1/2	1.950	2 1/8	.535
1 1/2	1.900	1.610	.145	2.717	2.731	11 1/2	2.218	2 1/4	.743
2	2.375	2.067	.154	3.652	3.678	11 1/2	2.760	2 3/8	1.208
2 1/2	2.875	2.469	.203	5.793	5.819	8	3.276	2 3/4	1.720
3	3.500	3.068	.216	7.575	7.616	8	3.948	3 1/4	2.498
3 1/2	4.000	3.548	.226	9.109	9.202	8	4.591	3 3/8	4.241
4	4.500	4.026	.237	10.790	10.889	8	5.091	3 3/4	4.741
4 1/2	5.000	4.506	.247	12.538	12.642	8	5.591	3 3/4	5.241
5	5.563	5.047	.258	14.617	14.810	8	6.296	4 1/8	8.091
6	6.625	6.065	.280	18.974	19.185	8	7.358	4 3/8	9.554
7	7.625	7.023	.301	23.544	23.769	8	8.358	4 3/8	10.932
8	8.625	8.071	.277	24.696	25.000	8	9.358	4 3/4	13.905
8	8.625	7.981	.322	28.554	28.809	8	9.358	4 3/8	13.905
9	9.625	8.941	.342	33.907	34.188	8	10.358	5 1/8	17.236
10	10.750	10.192	.279	31.201	32.000	8	11.721	6 1/8	29.877
10	10.750	10.136	.307	34.240	35.000	8	11.721	6 1/8	29.877
10	10.750	10.020	.365	40.483	41.132	8	11.721	6 1/8	29.877
11	11.750	11.000	.375	45.557	46.247	8	12.721	6 1/8	32.550
12	12.750	12.090	.330	43.773	45.000	8	13.958	6 1/8	43.098
12	12.750	12.000	.375	49.562	50.706	8	13.958	6 1/8	43.098
13	14.000	13.250	.375	54.568	55.824	8	15.208	6 3/8	47.152
14	15.000	14.250	.375	58.573	60.375	8	16.446	6 3/8	59.493
15	16.000	15.250	.375	62.579	64.500	8	17.446	6 3/8	63.294

The permissible variation in weight is 5 per cent. above and 5 per cent. below.

Furnished with threads and couplings and in random lengths unless otherwise ordered.

Taper of threads is 3/4" diameter per foot length for all sizes.

The weight per foot of pipe with threads and couplings is based on a length of 20 feet, including the coupling, but shipping lengths of small sizes will usually average less than 20 feet.

All weights and dimensions are nominal. On sizes made in more than one weight, weight desired must be specified.

STEEL PIPE

PIPE—BLACK AND GALVANIZED—Concluded

NATIONAL TUBE COMPANY STANDARD

EXTRA STRONG PIPE

DOUBLE EXTRA STRONG PIPE

Size, In.	Diameters, Inches		Thick-ness, Inches	Weight, per Foot, Pounds	Size, In.	Diameters, Inches		Thick-ness, Inches	Weight per Foot, Pounds
	External	Internal		Plain Ends		External	Internal		Plain Ends
1/8	.405	.215	.095	.314	1/2	.840	.252	.294	1.714
1/4	.540	.302	.119	.535	3/4	1.050	.434	.308	2.440
3/8	.675	.423	.126	.738	1	1.315	.599	.358	3.659
1/2	.840	.546	.147	1.087	1 1/4	1.660	.896	.382	5.214
3/4	1.050	.742	.154	1.473	1 1/2	1.900	1.100	.400	6.408
1	1.315	.957	.179	2.171	2	2.375	1.503	.436	9.029
1 1/4	1.660	1.278	.191	2.996	2 1/2	2.875	1.771	.552	13.695
1 1/2	1.900	1.500	.200	3.631	3	3.500	2.300	.600	18.583
2	2.375	1.939	.218	5.022	3 1/2	4.000	2.728	.636	22.850
2 1/2	2.875	2.323	.276	7.661	4	4.500	3.152	.674	27.541
3	3.500	2.900	.300	10.252	4 1/2	5.000	3.580	.710	32.530
3 1/2	4.000	3.364	.318	12.505	5	5.563	4.063	.750	38.552
4	4.500	3.826	.337	14.983	6	6.625	4.897	.864	53.160
4 1/2	5.000	4.290	.355	17.611	7	7.625	5.875	.875	63.079
5	5.563	4.813	.375	20.778	8	8.625	6.875	.875	72.424
6	6.625	5.761	.432	28.573					
7	7.625	6.625	.500	38.048					
8	8.625	7.625	.500	43.388					
9	9.625	8.625	.500	48.728					
10	10.750	9.750	.500	54.735					
11	11.750	10.750	.500	60.075					
12	12.750	11.750	.500	65.415					
13	14.000	13.000	.500	72.091					
14	15.000	14.000	.500	77.431					
15	16.000	15.000	.500	82.771					

Furnished with plain ends and in random lengths unless otherwise ordered.

Permissible variation in weight, for extra strong pipe, 5 per cent. above and 5 per cent. below.

For double extra strong pipe, 10 per cent. above and 10 per cent. below.

All weights and dimensions are nominal.

LARGE O. D. PIPE

Size, In.	Weight per Foot, Pounds									
	Thickness, Inches									
	1/4	5/16	3/8	7/16	1/2	9/16	5/8	3/4	7/8	1
14	36.713	45.682	54.568	63.371	72.091	80.726	89.279	106.134	122.654	138.842
15	39.383	49.020	58.573	68.044	77.431	86.734	95.954	114.144	132.000	149.522
16	42.053	52.357	62.579	72.716	82.771	92.742	102.629	122.154	141.345	160.202
17	44.723	55.695	66.584	77.389	88.111	98.749	109.304	130.164	150.690	170.882
18	47.393	59.032	70.589	82.061	93.451	104.757	115.979	138.174	160.035	181.562
20		65.708	78.599	91.407	104.131	116.772	129.330	154.194	178.725	202.923
21		69.045	82.604	96.079	109.471	122.780	136.005	162.204		
22		72.383	86.609	100.752	114.811	128.787	142.680	170.215		
24			94.619	110.097	125.491	140.802	156.030	186.235		
26			102.629	119.442	136.172	152.818	169.380	202.255		
28					128.787	146.852	164.833	182.730	218.275	
30					138.132	157.532	176.848	196.081	234.296	

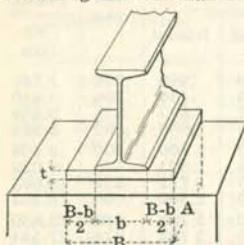
Furnished with plain ends and in random lengths, unless otherwise ordered.
All weights and dimensions are nominal.

BEARING PLATES

Steel Bearing Plates are provided for the ends of beams resting on masonry, to distribute the pressure over a sufficient area, the allowable unit pressure depending upon the class of masonry used.

The size, area and thickness of a bearing plate depends on the end reaction, the length and width of bearing and allowable unit stress.

Assuming that the maximum bending moment occurs in the center of bearing:



- A = Length of bearing plate, in inches.
- B = Width of bearing plate, in inches.
- t = Thickness of bearing plate, in inches.
- b = Flange width of beam, in inches.
- w = Allowable unit pressure on masonry.
- R = Reaction on bearing plates in pounds = $w \times A \times B$

$$M = \frac{R}{2} \times \frac{B}{4} - \frac{R}{2} \times \frac{b}{4} = \frac{R(B-b)}{8}$$

$$= \frac{w A B (B-b)}{8} = \frac{f A t^2}{6}$$

$$t = \sqrt{\frac{3 w B (B-b)}{4 f}}, \quad B(B-b) = \frac{4 f t^2}{3 w}$$

Taking moments at toe of beam flange, for cantilever projection $\frac{B-b}{2}$

$$M = \frac{w A (B-b)^2}{8} = \frac{f A t^2}{6} \quad t = \frac{1}{2} (B-b) \sqrt{\frac{3 w}{f}}$$

These formulas give lower values for M and t, and are applicable only when it can be assumed that middle part, b, of the plate is rigidly held in place, and that there are no bending stresses in center of plate.

Standard Bearing Plates—Sizes, Allowable Pressures and Limiting Spans.

Beam Section			Bearing Plates				Fiber Stress 18,000 Pounds			Fiber Stress 16,000 Pounds		
Depth	Weight	Flange, b	Dimensions			Weight	Total Pressure	Pressure	Min. Span	Total Pressure	Pressure	Min. Span
			A	B	t							
Ins.	Lbs.	Ins.	Inches			Lbs.	Lbs.	Lb. per sq. in.	Feet	Lbs.	Lb. per sq. in.	Feet
24	105.9	7.88	16	16	1	72.5	47262	184.6	29.74	42010	164.1	29.74
24	79.9	7.0	16	16	1	72.5	42667	166.7	24.46	37926	148.1	24.46
20	81.4	7.0	16	16	1	72.5	42667	166.7	20.62	37926	148.1	20.62
20	65.4	6.25	16	16	1	72.5	39385	153.8	17.82	35009	136.7	17.82
18	75.6	7.0	16	16	1	72.5	42667	166.7	17.84	37926	148.1	17.84
18	54.7	6.0	16	16	1	72.5	38400	150.0	13.81	34133	133.3	13.81
15	60.8	6.0	16	16	1	72.5	38400	150.0	12.69	34133	133.3	12.69
15	42.9	5.5	12	16	1	54.4	27429	142.9	12.88	24381	127.0	12.88
12	40.8	5.25	12	12	$\frac{3}{4}$	30.6	24000	166.7	11.21	21333	148.1	11.21
12	31.8	5.0	12	12	$\frac{3}{4}$	30.6	23143	160.7	9.32	20571	142.9	9.32
10	25.4	4.66	8	12	$\frac{3}{4}$	20.4	14714	153.3	9.96	13079	136.2	9.96
9	21.8	4.33	8	12	$\frac{3}{4}$	17.0	9778	101.9	11.58	8692	90.5	11.58
8	18.4	4.0	8	8	$\frac{3}{4}$	11.3	18750	293.0	4.55	16667	260.4	4.55
7	15.3	3.66	8	8	$\frac{3}{4}$	11.3	17281	270.0	3.59	15361	240.0	3.59
6	12.5	3.33	6	6	$\frac{1}{2}$	5.1	13483	374.5	3.23	11985	332.9	3.23
5	10.0	3.0	6	6	$\frac{1}{2}$	5.1	12000	333.3	2.42	10667	296.3	2.42
4	7.7	2.66	4	4	$\frac{3}{8}$	1.7	10075	629.7	1.78	8956	559.8	1.78
3	5.7	2.33	4	4	$\frac{3}{8}$	1.7	8084	505.3	1.23	7186	449.1	1.23

Limiting Span = $4 \times$ Bending Moment of Beam \div Total Pressure on Bearing Plate.

STRUCTURAL DETAILS

SPECIAL BEARING PLATES

Plates of special sizes may be computed from the foregoing formulas or from the Projection Coefficients, $B(B-b)$, after the required surface of the bearing plate has been determined from the reaction of the beam and the allowable pressure on the masonry.

EXAMPLE: Required a bearing plate with a wall bearing of 20 inches on masonry sustaining a safe unit pressure of 250 pounds per square inch, to distribute the end reaction of a 24"—100 lb. beam, supporting a uniformly distributed load over a span of 11 feet, beam and plates calculated for fiber stress of 18000 pounds.

Reaction, R , of 24"—100 lb. beam, 11 ft. span = 107,800 pounds.

Area of Plate = Reaction ÷ Unit Pressure, $107,800 \div 250 = 431.2$ sq. inches.

Dimensions of Bearing Plate: $A = 22''$, $B = 20''$, Area = 440 sq. inches.

Projection Coefficient: $B(B-b)$, $20(20-7.25) = 255.0$.

Referring to table of Projection Coefficients: nearest value for unit pressure of 250 pounds and fiber stress of 18000 pounds is 253.5, given for a $1\frac{5}{8}''$ -plate.

$$\text{Exact value from formula: } -f = \sqrt{\frac{3 w B (B-b)}{4 f}} = \sqrt{\frac{3 \times 250 \times 20 (20 - 7.25)}{4 \times 18000}} = 1.63''.$$

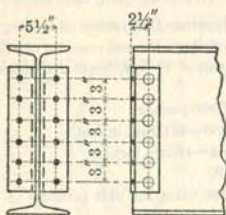
Projection Coefficients, $B(B-b)$, for various values of w and t

Thickness, Inch	Unit Pressure, w , in Pounds per Square Inch													
	75	100	125	150	175	200	225	250	275	300	325	350	375	400
t	Fiber Stress 18,000 Pounds													
$\frac{3}{8}$	45.0	33.8	27.0	22.5										
$\frac{1}{2}$	80.0	60.0	48.0	40.0	34.3	30.0								
$\frac{3}{4}$	125.0	93.8	75.0	62.5	53.6	46.9	41.7	37.5	34.1	31.3				
1	180.0	135.0	108.0	90.0	77.1	67.5	60.0	54.0	49.1	45.0	41.5	38.6	36.0	33.8
$1\frac{1}{8}$	240.0	183.8	147.0	122.5	105.0	91.9	81.7	73.5	66.8	61.3	56.5	52.5	49.0	45.9
$1\frac{1}{4}$	325.0	240.0	192.0	160.0	137.1	120.0	106.7	96.0	87.3	80.0	73.8	68.6	64.0	60.0
$1\frac{3}{8}$	405.0	303.8	243.0	202.5	173.6	151.9	135.0	121.5	110.5	101.3	93.4	86.8	81.0	75.9
$1\frac{1}{2}$	500.0	375.0	300.0	250.0	214.3	187.5	166.7	150.0	136.4	125.0	115.4	107.1	100.0	93.8
$1\frac{5}{8}$		453.8	363.0	302.5	259.3	226.9	201.7	181.5	165.0	151.3	139.6	129.6	121.0	113.4
$1\frac{3}{4}$			432.0	360.0	308.6	270.0	240.0	216.0	196.4	180.0	166.2	154.3	144.0	135.0
$1\frac{7}{8}$				422.5	362.1	316.9	281.7	253.5	230.5	211.3	195.0	181.1	169.0	158.4
2				490.0	420.0	367.5	326.7	294.0	267.3	245.0	226.2	210.0	196.0	183.8
					482.1	421.9	375.0	337.5	306.8	281.3	259.6	241.1	225.0	210.9
						480.0	426.7	384.0	349.1	320.0	295.4	274.3	256.0	240.0
t	Fiber Stress 16,000 Pounds													
$\frac{3}{8}$	40.0	30.0	24.0	20.0										
$\frac{1}{2}$	71.1	53.3	42.7	35.6	30.5	26.7								
$\frac{3}{4}$	111.1	83.3	66.7	55.6	47.6	41.7	37.0	33.3	30.3	27.8				
1	160.0	120.0	96.0	80.0	68.6	60.0	53.3	48.0	43.6	40.0	36.9	34.3	32.0	30.0
$1\frac{1}{8}$	217.8	163.3	130.7	108.9	93.3	81.7	72.6	65.3	59.4	54.4	50.2	46.7	43.6	40.8
$1\frac{1}{4}$	284.4	213.3	170.7	142.2	121.9	106.7	94.8	85.3	77.6	71.1	65.6	61.0	56.9	53.3
$1\frac{3}{8}$	360.0	270.0	216.0	180.0	154.3	135.0	120.0	108.0	98.2	90.0	83.1	77.1	72.0	67.5
$1\frac{1}{2}$	444.4	333.3	266.7	222.2	190.5	166.7	148.1	133.3	121.2	111.1	102.6	95.2	88.9	83.3
$1\frac{5}{8}$		403.3	322.7	268.9	230.5	201.7	179.2	161.3	146.7	134.4	124.1	115.2	107.6	100.9
$1\frac{3}{4}$		480.0	384.0	320.0	274.3	240.0	213.3	192.0	174.6	160.0	147.7	137.1	128.0	120.0
$1\frac{7}{8}$			450.7	375.6	321.9	281.7	250.4	225.3	204.8	187.8	173.3	161.0	150.2	140.8
2				435.6	373.3	326.7	290.4	261.3	237.5	217.8	201.0	186.7	174.2	163.3
				500.0	428.6	375.0	333.3	300.0	272.7	250.0	230.8	214.3	200.0	187.5
					487.6	426.7	379.2	341.3	310.3	284.4	262.6	243.8	227.6	213.3

CARNEGIE STEEL COMPANY

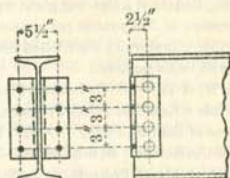
STANDARD BEAM CONNECTIONS

24"



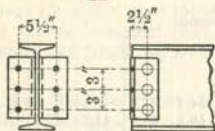
$2L^4 \times 4 \times \frac{1}{2} \times 1'-5\frac{1}{2}"$
Weight 39 lbs.

20," 18," 15"



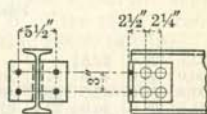
$2L^4 \times 4 \times \frac{1}{16} \times 0'-11\frac{1}{2}"$
Weight 23 lbs.

12"



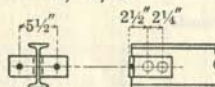
$2L^4 \times 4 \times \frac{1}{16} \times 0'-8\frac{1}{2}"$
Weight 17 lbs.

10," 9," 8"



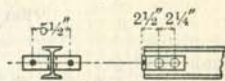
$2L^6 \times 4 \times \frac{3}{8} \times 0'-5\frac{1}{2}"$
Weight 13 lbs.

7," 6," 5"



$2L^6 \times 4 \times \frac{3}{8} \times 0'-3"$
Weight 7 lbs.

4," 3"



$2L^6 \times 4 \times \frac{3}{8} \times 0'-2"$
Weight 5 lbs.

Rivets and bolts $\frac{3}{4}$ " diameter.

Weights given are for $\frac{3}{4}$ -inch shop rivets and angle connections; about 20 per cent should be added for field rivets or bolts.

STRUCTURAL DETAILS

BEAM CONNECTIONS—LIMITING VALUES

Beam Section			Connection		Max. Values of Rivets and Bolts		Min. Span, Floor Beams		Girder Beam
Depth	Weight	Web	Shop	Field	Web Connection	Outstanding Legs	18,000 lb.	16,000 lb.	Web, t'
Inches	Lbs.	Inch	Rivets	Bolts	Pounds	Pounds	Feet	Feet	Inch
24	105.9	.625	6	12	84375	53014	26.52	23.57	.589
24	79.9	.500	6	12	67500	53014	19.69	17.50	.589
20	81.4	.600	4	8	54000	35343	24.89	22.13	.589
20	65.4	.500	4	8	45000	35343	19.85	17.65	.589
18	75.6	.560	4	8	50400	35343	21.54	19.14	.589
18	54.7	.460	4	8	41400	35343	15.01	13.34	.589
15	60.8	.590	4	8	53100	35343	13.78	12.25	.589
15	42.9	.410	4	8	36900	35343	10.00	8.89	.589
12	40.8	.460	3	6	31050	26506	10.15	9.02	.589
12	31.8	.350	3	6	23625	26506	9.13	8.12	.525
10	25.4	.310	4	4	27900	17671	8.29	7.37	.589
9	21.8	.290	4	4	26100	17671	6.41	5.70	.589
8	18.4	.270	4	4	24300	17671	4.83	4.29	.589
7	15.3	.250	2	2	11250	8836	7.03	6.25	.589
6	12.5	.230	2	2	10350	8836	4.93	4.38	.589
5	10.0	.210	2	2	9450	8836	3.28	2.92	.589
4	7.7	.190	2	2	8550	8836	2.09	1.86	.570
3	5.7	.170	2	2	7650	8836	1.30	1.15	.510

Limiting Span = $4 \times$ Bending Moment of Beam \div Least Value of Beam Connection.

The beam connections in above table are based on the resistance of $\frac{3}{4}$ " rivets in bearing and shear only, no moment stresses being considered, and the values given agree with tests made on beams under ordinary conditions of use.

Web Connection: Bearing Stress—Rivets enclosed—Shop 30,000 lb. per sq. in.

Outstanding Legs: Shearing Stress—Rivets or Bolts—Field 10,000 lb. per sq. in.

Web Thickness, t' : Bearing Stress—Rivets or Bolts—Field 20,000 lb. per sq. in.

Web thicknesses, t' , of girder beam correspond to required resistance in bearing, when beams frame opposite.

Special connections must be used when limiting conditions are exceeded, such as: End reaction of beam greater than value of connection—Short span with beam fully loaded—Thinner webs of girders resisting maximum end reactions of beams.

RIVETS AND BOLTS—UNIT STRESSES.

Bearing Stresses.

Rivets and Turned Bolts, enclosed, Shop 30,000 lb. per sq. in.

Rivets and Turned Bolts, one side, Shop 24,000 "

Rivets and Rough Bolts, enclosed, Field 20,000 "

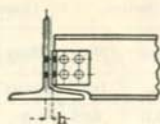
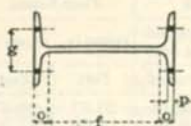
Rivets and Rough Bolts, one side, Field 16,000 "

Shearing Stresses.

Rivets and Turned Bolts, single shear, Shop 13,500 lb. per sq. in.

Rivets and Rough Bolts, single shear, Field 10,000 "

STANDARD GAGES AND DIMENSIONS FOR BEAMS



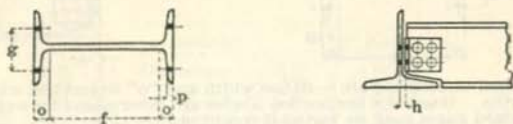
Nominal dimensions are:—flange width and "o" in eighths, web thickness in sixteenths. Gages for connection angles are determined by 1/2 web thickness.

Standard gages may be varied if conditions require.

Depth of Beam	Weight per Foot	Flange Width	Web Thickness	1/2 Web Thickness	Gage g	Grip p	Distance			Max. Rivet in Flange
							f	o	h	
In.	Lbs.	In.	In.	In.	In.	In.	In.	In.	In.	
24	120.0	8	1 3/16	3/8	5	1 1/8	20 1/4	1 7/8	1 1/2	7/8
	115.0	8	3/4	3/8	5	1 1/8	20 1/4	1 7/8	7/16	
	110.0	7 7/8	1 1/16	5/16	5	1 1/8	20 1/4	1 7/8	7/16	
	105.9	7 7/8	3/8	5/16	5	1 1/8	20 1/4	1 7/8	3/8	
24	100.0	7 1/4	3/4	3/8	4	7/8	20 3/4	1 5/8	7/16	7/8
	95.0	7 1/4	1 1/16	5/16	4	7/8	20 3/4	1 5/8	7/16	
	90.0	7 1/4	5/8	5/16	4	7/8	20 3/4	1 5/8	3/8	
	85.0	7 1/8	5/16	5/16	4	7/8	20 3/4	1 5/8	3/8	
	79.9	7	1/2	1/4	4	7/8	20 3/4	1 5/8	5/16	
20	100.0	7 1/4	7/8	7/16	4	1	16	2	1 1/2	7/8
	95.0	7 1/4	1 3/16	3/8	4	1	16	2	1 1/2	
	90.0	7 3/8	3/4	3/8	4	1	16	2	7/16	
	85.0	7	3/8	5/16	4	1	16 1/2	1 3/4	3/8	
	81.4	7	3/8	5/16	4	1	16 1/2	1 3/4	3/8	
20	75.0	6 3/8	5/8	5/16	4	3/4	16 1/2	1 3/4	3/8	7/8
	70.0	6 3/8	5/16	5/16	4	3/4	17	1 1/2	3/8	
	65.4	6 1/4	1/2	1/4	4	3/4	17	1 1/2	5/16	
18	90.0	7 1/4	1 3/16	3/8	4	1	14 1/2	1 3/4	1 1/2	7/8
	85.0	7 1/4	1 1/16	3/8	4	1	14 1/2	1 3/4	7/16	
	80.0	7 1/8	5/8	5/16	4	1	14 1/2	1 3/4	3/8	
	75.6	7	5/16	1/4	4	1	14 1/2	1 3/4	3/8	
18	70.0	6 1/4	1 1/16	3/8	3 1/2	3/4	14 3/4	1 5/8	7/16	7/8
	65.0	6 1/4	3/8	5/16	3 1/2	3/4	14 3/4	1 5/8	3/8	
	60.0	6 3/8	5/16	1/4	3 1/2	3/4	15 1/4	1 3/8	3/8	
	54.7	6	7/16	1/4	3 1/2	3/4	15 1/4	1 3/8	5/16	
15	75.0	6 1/4	7/8	7/16	3 1/2	7/8	11 3/4	1 5/8	1 1/2	3/4
	70.0	6 1/4	3/4	3/8	3 1/2	7/8	11 3/4	1 5/8	7/16	
	65.0	6 3/8	5/16	5/16	3 1/2	7/8	11 3/4	1 5/8	7/16	
	60.8	6	5/16	5/16	3 1/2	7/8	11 3/4	1 5/8	3/8	
15	55.0	5 3/4	5/8	5/16	3 1/2	5/8	12 1/2	1 1/4	3/8	3/4
	50.0	5 3/8	5/16	1/4	3 1/2	5/8	12 1/2	1 1/4	3/8	
	45.0	5 1/2	7/16	1/4	3 1/2	5/8	12 1/2	1 1/4	5/16	
	42.9	5 1/2	7/16	5/16	3 1/2	5/8	12 1/2	1 1/4	1/4	

STRUCTURAL DETAILS

STANDARD GAGES AND DIMENSIONS FOR BEAMS



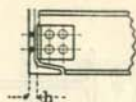
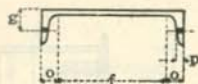
Nominal dimensions are:—flange width and "o" in eighths, web thickness in sixteenths. Gages for connection angles are determined by $\frac{1}{2}$ web thickness.

Standard gages may be varied if conditions require.

Depth of Beam	Weight per Foot	Flange Width	Web Thickness	$\frac{1}{2}$ Web Thickness	Gage g	Grip p	Distance			Max. Rivet in Flange
							f	o	h	
In.	Lbs.	In.	In.	In.	In.	In.	In.	In.	In.	
12	55.0	5 $\frac{5}{8}$	1 $\frac{3}{16}$	$\frac{3}{8}$	3 $\frac{1}{2}$	$\frac{3}{4}$	9 $\frac{1}{4}$	1 $\frac{3}{8}$	$\frac{1}{2}$	¾
	50.0	5 $\frac{1}{2}$	1 $\frac{1}{16}$	$\frac{5}{16}$	3 $\frac{1}{2}$	$\frac{3}{4}$	9 $\frac{1}{4}$	1 $\frac{3}{8}$	$\frac{7}{16}$	
	45.0	5 $\frac{3}{8}$	$\frac{9}{16}$	$\frac{5}{16}$	3	$\frac{3}{4}$	9 $\frac{1}{4}$	1 $\frac{3}{8}$	$\frac{3}{8}$	
	40.8	5 $\frac{1}{4}$	$\frac{7}{16}$	$\frac{1}{4}$	3	$\frac{3}{4}$	9 $\frac{1}{4}$	1 $\frac{3}{8}$	$\frac{9}{16}$	
12	35.0	5 $\frac{1}{8}$	$\frac{7}{16}$	$\frac{5}{16}$	3	$\frac{9}{16}$	9 $\frac{3}{4}$	1 $\frac{1}{8}$	$\frac{9}{16}$	¾
	31.8	5	$\frac{5}{8}$	$\frac{5}{16}$	3	$\frac{9}{16}$	9 $\frac{3}{4}$	1 $\frac{1}{8}$	$\frac{1}{4}$	
10	40.0	5 $\frac{1}{8}$	$\frac{5}{8}$	$\frac{3}{8}$	2 $\frac{3}{4}$	$\frac{1}{2}$	7 $\frac{1}{2}$	1 $\frac{1}{4}$	$\frac{7}{16}$	¾
	35.0	5	$\frac{5}{8}$	$\frac{5}{16}$	2 $\frac{3}{4}$	$\frac{1}{2}$	7 $\frac{1}{2}$	1 $\frac{3}{4}$	$\frac{3}{8}$	
	30.0	4 $\frac{3}{4}$	$\frac{7}{16}$	$\frac{1}{4}$	2 $\frac{3}{4}$	$\frac{1}{2}$	8	1	$\frac{5}{16}$	
	25.4	4 $\frac{5}{8}$	$\frac{9}{16}$	$\frac{1}{8}$	2 $\frac{3}{4}$	$\frac{1}{2}$	8	1	$\frac{1}{4}$	
9	35.0	4 $\frac{3}{4}$	$\frac{5}{8}$	$\frac{3}{8}$	2 $\frac{1}{2}$	$\frac{1}{2}$	7	1	$\frac{7}{16}$	¾
	30.0	4 $\frac{5}{8}$	$\frac{9}{16}$	$\frac{1}{4}$	2 $\frac{1}{2}$	$\frac{1}{2}$	7	1	$\frac{3}{8}$	
	25.0	4 $\frac{1}{2}$	$\frac{5}{8}$	$\frac{5}{16}$	2 $\frac{1}{2}$	$\frac{1}{2}$	7	1	$\frac{1}{4}$	
	21.8	4 $\frac{3}{8}$	$\frac{9}{16}$	$\frac{1}{8}$	2 $\frac{1}{2}$	$\frac{1}{2}$	7	1	$\frac{9}{16}$	
8	25.5	4 $\frac{1}{4}$	$\frac{9}{16}$	$\frac{1}{4}$	2 $\frac{1}{4}$	$\frac{1}{2}$	6 $\frac{1}{4}$	$\frac{7}{8}$	$\frac{5}{16}$	¾
	23.0	4 $\frac{1}{8}$	$\frac{7}{16}$	$\frac{1}{4}$	2 $\frac{1}{4}$	$\frac{7}{16}$	6 $\frac{1}{4}$	$\frac{7}{8}$	$\frac{5}{16}$	
	20.5	4 $\frac{1}{8}$	$\frac{5}{8}$	$\frac{5}{16}$	2 $\frac{1}{4}$	$\frac{7}{16}$	6 $\frac{1}{4}$	$\frac{7}{8}$	$\frac{1}{4}$	
	18.4	4	$\frac{1}{4}$	$\frac{1}{8}$	2 $\frac{1}{4}$	$\frac{7}{16}$	6 $\frac{1}{4}$	$\frac{7}{8}$	$\frac{9}{16}$	
7	20.0	3 $\frac{7}{8}$	$\frac{7}{16}$	$\frac{1}{4}$	2 $\frac{1}{4}$	$\frac{3}{8}$	5 $\frac{1}{4}$	$\frac{7}{8}$	$\frac{5}{16}$	¾
	17.5	3 $\frac{3}{4}$	$\frac{3}{8}$	$\frac{5}{16}$	2 $\frac{1}{4}$	$\frac{3}{8}$	5 $\frac{1}{4}$	$\frac{7}{8}$	$\frac{1}{4}$	
	15.3	3 $\frac{5}{8}$	$\frac{1}{4}$	$\frac{1}{8}$	2 $\frac{1}{4}$	$\frac{3}{8}$	5 $\frac{1}{4}$	$\frac{7}{8}$	$\frac{9}{16}$	
6	17.25	3 $\frac{5}{8}$	$\frac{7}{16}$	$\frac{1}{4}$	2	$\frac{3}{8}$	4 $\frac{1}{2}$	$\frac{3}{4}$	$\frac{5}{16}$	¾
	14.75	3 $\frac{1}{2}$	$\frac{3}{8}$	$\frac{5}{16}$	2	$\frac{3}{8}$	4 $\frac{1}{2}$	$\frac{3}{4}$	$\frac{1}{4}$	
	12.5	3 $\frac{3}{8}$	$\frac{1}{4}$	$\frac{1}{8}$	2	$\frac{3}{8}$	4 $\frac{1}{2}$	$\frac{3}{4}$	$\frac{9}{16}$	
5	14.75	3 $\frac{3}{8}$	$\frac{1}{2}$	$\frac{1}{4}$	1 $\frac{3}{4}$	$\frac{3}{8}$	3 $\frac{1}{2}$	$\frac{3}{4}$	$\frac{5}{16}$	½
	12.25	3 $\frac{1}{8}$	$\frac{3}{8}$	$\frac{5}{16}$	1 $\frac{3}{4}$	$\frac{3}{8}$	3 $\frac{1}{2}$	$\frac{3}{4}$	$\frac{1}{4}$	
	10.0	3	$\frac{9}{16}$	$\frac{1}{8}$	1 $\frac{3}{4}$	$\frac{3}{8}$	3 $\frac{1}{2}$	$\frac{3}{4}$	$\frac{9}{16}$	
4	10.5	2 $\frac{7}{8}$	$\frac{3}{8}$	$\frac{5}{16}$	1 $\frac{1}{2}$	$\frac{5}{16}$	2 $\frac{3}{4}$	$\frac{5}{8}$	$\frac{1}{4}$	½
	9.5	2 $\frac{3}{4}$	$\frac{9}{16}$	$\frac{5}{16}$	1 $\frac{1}{2}$	$\frac{5}{16}$	2 $\frac{3}{4}$	$\frac{5}{8}$	$\frac{1}{4}$	
	8.5	2 $\frac{3}{4}$	$\frac{1}{4}$	$\frac{1}{8}$	1 $\frac{1}{2}$	$\frac{5}{16}$	2 $\frac{3}{4}$	$\frac{5}{8}$	$\frac{3}{16}$	
	7.7	2 $\frac{5}{8}$	$\frac{9}{16}$	$\frac{1}{8}$	1 $\frac{1}{2}$	$\frac{5}{16}$	2 $\frac{3}{4}$	$\frac{5}{8}$	$\frac{9}{16}$	
3	7.5	2 $\frac{1}{2}$	$\frac{3}{8}$	$\frac{5}{16}$	1 $\frac{1}{2}$	$\frac{5}{16}$	1 $\frac{3}{4}$	$\frac{5}{8}$	$\frac{1}{4}$	¾
	6.5	2 $\frac{3}{8}$	$\frac{1}{4}$	$\frac{1}{8}$	1 $\frac{1}{2}$	$\frac{5}{16}$	1 $\frac{3}{4}$	$\frac{5}{8}$	$\frac{3}{16}$	
	5.7	2 $\frac{5}{8}$	$\frac{9}{16}$	$\frac{1}{16}$	1 $\frac{1}{2}$	$\frac{5}{16}$	1 $\frac{3}{4}$	$\frac{5}{8}$	$\frac{3}{8}$	

CARNEGIE STEEL COMPANY

STANDARD GAGES AND DIMENSIONS FOR CHANNELS



Nominal dimensions are:—flange width and "o" in eighths, web thickness in sixteenths. Gages for connection angles are determined by web thickness. Standard gages may be varied if conditions require.

Depth of Channel	Weight per Foot	Flange Width	Web Thickness	1/2 Web Thickness	Gage g	Grip p	Distance			Max. Rivet in Flange		
							f	o	h			
In.	Lbs.	In.	In.	In.	In.	In.	In.	In.	In.			
15	55.0	3 7/8	1 3/16	5/16	2 1/2	1 1/16	12 1/4	1 3/8	7/8	3/8		
	50.0	3 3/8	1 1/16	3/8	2 1/2	1 1/16	12 1/4	1 3/8	1 3/16			
	45.0	3 3/8	5/8	5/16	2	5/8	12 3/4	1 3/8	1 1/16			
	40.0	3 1/2	1/2	1/4	2	5/8	12 3/4	1 3/8	5/16			
	35.0	3 3/8	7/16	5/16	2	5/8	12 1/4	1 3/8	1/2			
	33.9	3 3/8	3/8	5/16	2	5/8	12 1/4	1 3/8	1/2			
13	50.0	4 1/2	1 3/16	3/8	2 1/2	5/8	10 1/2	1 1/4	7/8	3/8		
	45.0	4 3/8	1 1/16	5/16	2 1/2	5/8	10 1/2	1 1/4	3/4			
	40.0	4 1/4	5/16	1/4	2 1/2	5/8	10 1/2	1 1/4	5/8			
	37.0	4 3/8	1/2	1/4	2 1/2	5/16	10 1/2	1 1/4	5/16			
	35.0	4 3/8	7/16	1/4	2 1/2	5/16	10 1/2	1 1/4	1/2			
	31.8	4	3/8	5/16	2 1/2	5/16	10 1/2	1 1/4	7/16			
12	40.0	3 3/8	3/4	3/8	2	5/8	9	1	1 3/16	3/8		
	35.0	3 1/4	5/8	5/16	2	5/8	9	1	1 1/16			
	30.0	3 3/8	1/2	1/4	1 3/4	1/2	10	1	5/16			
	25.0	3	3/8	5/16	1 3/4	1/2	10	1	7/16			
	20.7	3	1/4	3/8	1 3/4	1/2	10	1	3/8			
	10	35.0	3 1/8	1 3/16	7/16	1 3/4	1/2	7 3/4	1 1/8		7/8	3/4
30.0		3	1 1/16	5/16	1 3/4	1/2	7 3/4	1 1/8	3/4			
25.0		2 7/8	1/2	1/4	1 3/4	1/2	7 3/4	1 1/8	5/16			
20.0		2 3/4	3/8	5/16	1 1/2	7/16	8 1/4	7/8	7/16			
15.3		2 5/8	1/4	3/8	1 1/2	7/16	8 1/4	7/8	5/16			
9		25.0	2 7/8	5/8	5/16	1 1/2	1/2	7	1	1 1/16	3/4	
	20.0	2 3/8	7/16	1/4	1 1/2	1/2	7	1	1/2			
	15.0	2 1/2	5/16	3/8	1 3/8	7/16	7 1/4	7/8	3/8			
	13.4	2 3/8	1/4	3/8	1 3/8	7/16	7 1/4	7/8	5/16			
	21.25	2 5/8	5/16	5/16	1 1/2	7/16	6 1/4	7/8	1 1/16			
	18.75	2 1/2	1/2	1/4	1 1/2	7/16	6 1/4	7/8	5/16			
8	16.25	2 3/8	3/8	5/16	1 1/2	7/16	6 1/4	7/8	1/2	3/4		
	13.75	2 3/8	5/16	3/8	1 3/8	3/8	6 1/4	7/8	3/8			
	11.5	2 1/4	1/4	3/8	1 3/8	3/8	6 1/4	7/8	5/16			
	7	19.75	2 1/2	5/8	5/16	1 1/2	7/16	5 1/4	7/8		1 1/16	5/8
		17.25	2 3/8	1/2	1/4	1 1/2	7/16	5 1/4	7/8		5/16	
		14.75	2 1/4	7/16	3/8	1 1/4	7/16	5 1/4	7/8		1/2	
12.25		2 1/4	5/16	5/16	1 1/4	3/4	5 1/2	3/4	3/8			
9.8		2 1/8	5/16	3/8	1 1/4	3/8	5 1/2	3/4	5/16			
6		15.5	2 1/4	5/16	1/4	1 3/8	3/8	4 1/2	3/4	5/8	5/8	
	13.0	2 1/8	7/16	1/4	1 3/8	3/8	4 1/2	3/4	1/2			
	10.5	2	5/16	5/16	1 3/8	3/8	4 1/2	3/4	3/8			
	8.2	1 7/8	5/16	3/8	1 3/8	5/16	4 1/2	3/4	1/4			
	5	11.5	2	1/2	1/4	1 3/8	5/16	3 1/2	3/4	5/16		1/2
		9.0	1 7/8	5/16	5/16	1 3/8	5/16	3 3/4	3/8	3/8		
6.7		1 3/4	5/16	3/8	1 3/8	5/16	3 3/4	3/8	1/4			
4		7.25	1 3/4	5/16	5/16	1	5/16	2 3/4	3/8	3/8	1/2	
		6.25	1 3/8	1/4	3/8	1	5/16	2 3/4	3/8	5/16		
		5.4	1 3/8	5/16	1/16	1	5/16	2 3/4	3/8	1/4		
	3	6.0	1 5/8	3/8	5/16	7/8	1/4	1 3/4	3/8	7/16		1/2
		5.0	1 1/2	1/4	3/8	7/8	1/4	1 3/4	3/8	5/16		
		4.1	1 3/8	5/16	1/16	7/8	1/4	1 3/4	3/8	1/4		

STRUCTURAL DETAILS

BEAM SEPARATORS

AMERICAN BRIDGE COMPANY STANDARD

Depth, Inches	Beams		Separator				3/4" Bolts				Diagrams		
	Weight per Foot, Pounds	Center to Center of Beams, Inches	Out to Out of Flanges, Inches	Dimensions				Weight, Pounds	Increase in Weight for 1" Add. Width	Length, Inches		Hex. Head and Nut	Increase in Weight for 1" Add. Length
				w	h	d	t						
24	120 to 105.9	8 3/4	10 3/4	8	20	12	5 3/8	31	3.6	10 1/2	3.4	0.25	<p style="text-align: center;">3/8" Cored Holes</p>
	100	8	15 1/2	7 1/4	20	12	5 3/8	28	3.6	10	3.2	0.25	
24	95 and 90	8	15 1/4	7 1/4	20	12	5 3/8	28	3.6	10	3.2	0.25	
	85	8	15 1/4	7 1/2	20	12	5 3/8	29	3.6	9 1/2	3.1	0.25	
	79.9	8	15	7 1/2	20	12	5 3/8	29	3.6	9 1/2	3.1	0.25	
20	100 and 95	8	15 3/4	7	16	12	5 3/8	22	2.9	10	3.2	0.25	
	90	7 1/2	14 3/4	6 3/4	16	12	5 3/8	22	2.9	9 1/2	3.1	0.25	
	85 and 81.4	7 1/2	14 3/4	6 3/4	16	12	5 3/8	22	2.9	9	3.0	0.25	
20	75	7 1/2	14	6 3/4	16	12	5 3/8	22	2.9	9	3.0	0.25	
	70	7	13 3/4	6 1/2	16	12	5 3/8	21	2.9	9	3.0	0.25	
	65.4	7	13 3/4	6 1/2	16	12	5 3/8	21	2.9	8 1/2	3.0	0.25	
18	90	8	15 1/4	7	14	9	5 3/8	20	2.5	10	3.2	0.25	
	85 and 80	8	15 3/8	7 1/4	14	9	5 3/8	21	2.5	10	3.2	0.25	
	75.6	8	15	7 1/2	14	9	5 3/8	21	2.5	10	3.2	0.25	
18	70 and 65	7	13 1/4	6 1/4	14	9	5 3/8	18	2.5	9	3.0	0.25	
	60	7	13 1/4	6 1/4	14	9	5 3/8	19	2.5	8 1/2	3.0	0.25	
	54.7	7	13	6 1/2	14	9	5 3/8	19	2.5	8 1/2	3.0	0.25	
15	75	7	13 3/4	6	12	9	5 3/8	12	1.6	9	3.0	0.25	
	70 and 65	7	13 3/4	6 1/4	12	9	5 3/8	12	1.6	9	3.0	0.25	
	60.8	6 1/2	12 1/2	5 3/4	12	9	5 3/8	12	1.6	8	2.7	0.25	
15	55	6 1/2	12 3/4	6	12	9	5 3/8	12	1.6	8	2.7	0.25	
	50 and 45	6 1/2	12	6	12	9	5 3/8	12	1.6	8	2.7	0.25	
	42.9	6 1/2	12	6	12	9	5 3/8	12	1.6	8	2.7	0.25	
12	55	6	11 1/4	5 1/4	9	6	5 3/8	9	1.3	8	2.7	0.25	
	50	6	11 1/2	5 3/4	9	6	5 3/8	9	1.3	8	2.7	0.25	
12	45	6	11 1/4	5 1/4	9	6	5 3/8	9	1.3	7 1/2	2.6	0.25	
	40.8 and 35	6	11 3/4	5 1/2	9	6	5 3/8	9	1.3	7 1/2	2.6	0.25	
	31.8	6	11	5 1/2	9	6	5 3/8	9	1.3	7 1/2	2.6	0.25	
10	40	5 1/2	10 3/4	4 3/4	7 1/2	5	5 3/8	6	1.1	7 1/2	1.3	0.13	<p style="text-align: center;">3/8" Cored Hole</p>
	35	5 1/2	10 1/2	4 3/4	7 1/2	5	5 3/8	6	1.1	7	1.3	0.13	
	30	5 1/2	10 1/2	5	7 1/2	5	5 3/8	7	1.1	7	1.3	0.13	
	25.4	5 1/2	10	5	7 1/2	5	5 3/8	7	1.1	7	1.3	0.13	
9	35	5	10	4 1/4	6 1/2	4	5 3/8	5	0.9	7	1.3	0.13	
	30	5	9 1/2	4 1/4	6 1/2	4	5 3/8	5	0.9	6 1/2	1.2	0.13	
	25	5	9 1/2	4 1/2	6 1/2	4	5 3/8	5	0.9	6 1/2	1.2	0.13	
	21.8	5	9 1/4	4 1/2	6 1/2	4	5 3/8	5	0.9	6 1/2	1.2	0.13	
8	25.5	4 1/2	9	4	5 1/2	4	5 3/8	4	0.8	6	1.1	0.13	
	23	4 1/2	8 3/4	4	5 1/2	4	5 3/8	4	0.8	6	1.1	0.13	
	20.5 and 18.4	4 1/2	8 1/2	4	5 1/2	4	5 3/8	4	0.8	6	1.1	0.13	
7	20	4 1/2	8 1/2	4	5	4	5 3/8	4	0.7	6	1.1	0.13	
	17.5	4 1/2	8 1/2	4	5	4	5 3/8	4	0.7	6	1.1	0.13	
	15.3	4 1/2	8 1/4	4 1/4	5	4	5 3/8	4	0.7	6	1.1	0.13	
6	17.25	4	7 3/4	3 1/2	4 1/2	4	5 3/8	4	0.6	5 1/2	1.1	0.13	
	14.75	4	7 1/2	3 1/2	4 1/2	4	5 3/8	4	0.6	5 1/2	1.1	0.13	
	12.5	4	7 1/2	3 3/4	4 1/2	4	5 3/8	4	0.6	5 1/2	1.1	0.13	

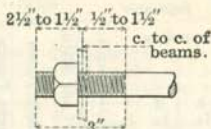
For 5", 4" and 3" beams, use 1" gas pipe 3 1/4", 3" and 2 3/4" long respectively.

CARNEGIE STEEL COMPANY

TIE RODS AND ANCHORS

AMERICAN BRIDGE COMPANY STANDARD

3/4-INCH TIE RODS

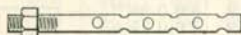


C. to C.	Length	Weight	C. to C.	Length	Weight	C. to C.	Length	Weight	C. to C.	Length	Weight
Ft.-In.	Ft.-In.	Pounds	Ft.-In.	Ft.-In.	Pounds	Ft.-In.	Ft.-In.	Pounds	Ft.-In.	Ft.-In.	Pounds
1-0	1-3	2.30	1-3	1-6	2.67	1-6	1-9	3.05	1-9	2-0	3.42
2-0	2-3	3.80	2-3	2-6	4.17	2-6	2-9	4.55	2-9	3-0	4.92
3-0	3-3	5.30	3-3	3-6	5.67	3-6	3-9	6.05	3-9	4-0	6.42
4-0	4-3	6.80	4-3	4-6	7.17	4-6	4-9	7.55	4-9	5-0	7.92
5-0	5-3	8.30	5-3	5-6	8.67	5-6	5-9	9.05	5-9	6-0	9.42
6-0	6-3	9.80	6-3	6-6	10.17	6-6	6-9	10.55	6-9	7-0	10.92
7-0	7-3	11.30	7-3	7-6	11.67	7-6	7-9	12.05	7-9	8-0	12.42
8-0	8-3	12.80	8-3	8-6	13.17	8-6	8-9	13.55	8-9	9-0	13.92

Weights include two Nuts.

ANCHORS

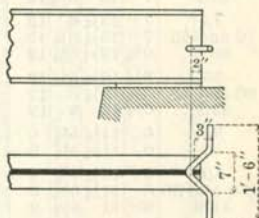
SWEDGE BOLT



Diameter	Length	Weight
Inches	Feet - Inches	Pounds
1	1-0	3.1
1 1/4	1-3	6.1
1 1/2	1-3	8.9

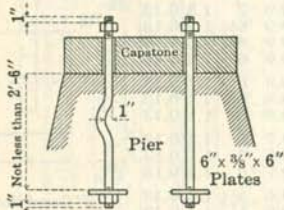
Weight includes Nut

GOVERNMENT ANCHOR



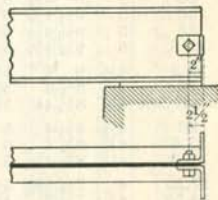
3/4" Rod 1' 9" long. Wt., 3 lbs.

BUILT-IN ANCHOR BOLTS



When center to center of anchors is less than width of washer, use washer with two holes.

ANGLE ANCHOR



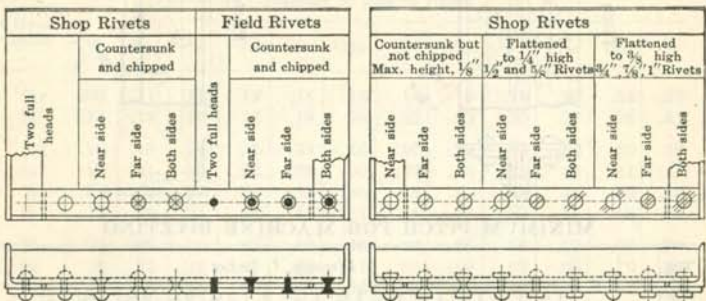
2 Angles 6" x 4" x 3/8" x 0'-3"
Weight with 3/4" bolts, 7 lbs.

STRUCTURAL DETAILS

DETAILS FOR PUNCHING AND RIVETING

AMERICAN BRIDGE COMPANY STANDARD

CONVENTIONAL SIGNS FOR RIVETING



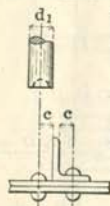
DIMENSIONS OF STRUCTURAL RIVETS



	Diameter of Rivet, d, Inches									
	3/8	1/2	5/8	3/4	7/8	1	1 1/8	1 1/4	1 3/8	1 1/2
w	1 1/16	7/8	1 1/16	1 1/4	1 7/16	1 5/8	1 11/16	2	2 3/16	2 3/8
h	5/16	3/8	7/16	1/2	9/8	1 1/16	3/4	7/8	1 5/16	1
r	7/16	9/16	1 1/16	1 3/16	1 5/16	1	1 1/8	1 1/4	1 5/8	1 1/2
w1	9/16	3/4	1	1 1/16	1 3/8	1 9/16	1 3/4	2	2 3/16	2 3/8
h1	3/16	1/4	5/16	3/8	7/16	1/2	9/16	5/8	1 1/16	3/4

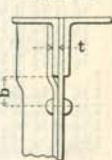
$w = 1\frac{1}{2}d + \frac{1}{8}''$ $h = 0.425w$ $r = 1\frac{1}{2}h$

DRIVING CLEARANCE



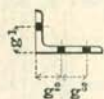
	Diameter of Rivet, d, Inches									
	3/8	1/2	5/8	3/4	7/8	1	1 1/8	1 1/4	1 3/8	1 1/2
d1	1 1/2	1 3/4	2	2 1/4	2 1/2	2 3/4	3	3 1/4	3 1/2	3 3/4
c	3/8	1	1 1/8	1 1/4	1 3/8	1 1/2	1 5/8	1 3/4	1 7/8	2

CRIMPS



$b = t + 1\frac{1}{2}''$ (min. 2'')

GAGES FOR ANGLES

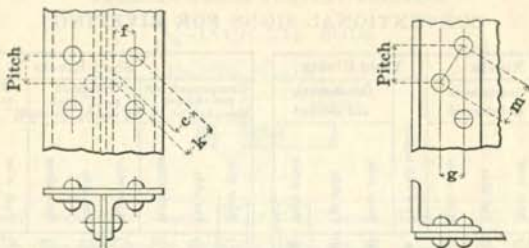


	Width of Leg, Inches														
	8	7	6	5	4	3 1/2	3	2 1/2	2	1 3/4	1 1/2	1 3/8	1 1/4	1	3/4
g1	4 1/2	4	3 1/2	3	2 1/2	2	1 3/4	1 3/8	1 1/8	1	7/8	7/8	3/4	3/8	1/2
g2	3	2 1/2	2 1/2	2											
g3	3	3	2 1/4	1 3/4											
Max. rivet	1 1/8	1	3/8	7/8	7/8	3/8	3/8	3/4	5/8	1/2	3/8	3/8	3/8	1/4	1/4

CARNEGIE STEEL COMPANY

RIVET SPACING

AMERICAN BRIDGE COMPANY STANDARD



MINIMUM PITCH FOR MACHINE RIVETING

Dia. of Rivet	c	k	Distance, f, Inches														
			1 1/8	1 1/4	1 3/8	1 1/2	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3
3/8	3/8	1 3/16	1/4	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1/2	1 1/8	1 3/8	1 1/8	1 1/4	1 3/8	1 1/2	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3
5/8	1 1/4	1 5/8	1 1/4	1 1/2	1 3/4	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3	0
3/4	1 1/2	1 3/4	1 1/2	1 3/4	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3	0	0
7/8	1 3/8	1 5/8	1 3/8	1 1/2	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3	0	0
1	1 1/2	1 3/4	1 1/2	1 3/4	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3	0	0
1 1/8	1 3/4	1 5/8	1 3/4	1 1/2	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3	0	0
1 1/4	1 5/8	1 3/4	1 1/2	1 3/4	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3	0	0
1 1/2	1 3/4	1 5/8	1 3/4	1 1/2	1 5/8	1 3/4	1 7/8	2	2 1/8	2 1/4	2 3/8	2 1/2	2 5/8	2 3/4	3	0	0

MINIMUM PITCH TO MAINTAIN 3 DIAMETERS C. TO C.

Dia. of Rivet	m	Distance, g, Inches														
		1	1 1/4	1 1/2	1 3/4	2	2 1/4	2 1/2	2 3/4	3	3 1/4	3 1/2	3 3/4	4	4 1/4	4 1/2
3/8	1 1/8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1/2	1 1/4	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8	1 1/8
5/8	1 1/2	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4	1 1/4
3/4	1 3/8	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2
7/8	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2
1	1 1/4	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2
1 1/8	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2
1 1/4	1 3/4	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2
1 1/2	1 3/4	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2	1 1/2

COVER PLATE RIVETING

a, Ins.	d, Ins.	Diagram	b, Ins.	d, Ins.
1/2	2 1/2			1/2
1	2 3/4	3/4		2 3/4
1 1/4	3 1/4	1		2 3/4
1 1/2	3 3/4	1 1/4		2 3/4
2	4 1/4	1 1/2		2 3/4
2 1/2	4 3/4	2		2 3/4
3	5 1/4	2 1/4		2 3/4
3 1/2	5 3/4	2 1/2		2 3/4
4	6 1/4	2 3/4		2 3/4
5	6 3/4	3 1/4		2 3/4
6	7 1/4	4		2 3/4

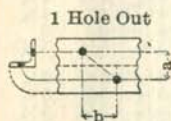
STRUCTURAL DETAILS

REDUCTION OF AREA FOR RIVET HOLES

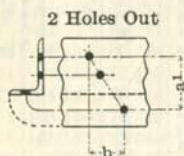
Area in Square Inches=Diameter of Hole by Thickness of Metal

Thickness of Metal, Inches	Diameter of Hole in Inches											
	½	⅝	¾	⅞	1	1 ⅛	1 ¼	1 ⅝	1 ¾	2	2 ⅛	2 ¼
⅜	.09	.11	.12	.13	.14	.15	.16	.18	.19	.20	.21	.23
¼	.13	.14	.16	.17	.19	.20	.22	.23	.25	.27	.28	.31
⅝	.16	.18	.20	.21	.23	.25	.27	.29	.31	.33	.35	.39
¾	.19	.21	.23	.26	.28	.30	.33	.35	.38	.40	.42	.47
⅞	.22	.25	.27	.30	.33	.36	.38	.41	.44	.46	.49	.55
1	.25	.28	.31	.34	.38	.41	.44	.47	.50	.53	.56	.63
1 ⅛	.28	.32	.35	.39	.42	.46	.49	.53	.56	.60	.63	.70
1 ¼	.31	.35	.39	.43	.47	.51	.55	.59	.63	.66	.70	.78
1 ⅝	.34	.39	.43	.47	.52	.56	.60	.64	.69	.73	.77	.86
1 ¾	.38	.42	.47	.52	.56	.61	.66	.70	.75	.80	.84	.94
2			.51	.56	.61	.66	.71	.76	.81	.86	.91	1.02
2 ⅛			.55	.60	.66	.71	.77	.82	.88	.93	.98	1.09
2 ¼			.59	.64	.70	.76	.82	.88	.94	1.00	1.05	1.17
2 ⅝			.63	.69	.75	.81	.88	.94	1.00	1.06	1.13	1.25
2 ¾				.80	.86	.93	1.00	1.06	1.13	1.20	1.27	1.41
3				.84	.91	.98	1.05	1.13	1.20	1.27	1.34	1.48
3 ⅛				.89	.96	1.04	1.11	1.19	1.26	1.34	1.41	1.56
3 ¼				.94	1.02	1.09	1.17	1.25	1.33	1.41	1.48	1.64
3 ⅝						1.15	1.23	1.31	1.39	1.48	1.55	1.72
3 ¾						1.20	1.29	1.38	1.46	1.55	1.62	1.80
4						1.26	1.35	1.44	1.53	1.62	1.69	1.88
4 ⅛						1.31	1.41	1.50	1.59	1.69	1.78	1.98

PITCH OF RIVETS TO MAINTAIN NET SECTION



1 Hole Out



2 Holes Out

Dimensions in Inches

a	¾"	⅞"	a ¹	¾"	⅞"
	Rivet	Rivet		Rivet	Rivet
b	b	b	b	b	
1	1 ⅝	1 ¾	5	3 ⅝	3 ⅞
1 ½	1 ⅞	2	5 ½	3 ¾	3 ⅞
2	2 ⅛	2 ¼	6	3 ⅞	3 ⅞
2 ½	2 ¼	2 ⅝	6 ½	3 ⅞	3 ⅞
3	2 ⅝	2 ¾	7	3 ⅞	3 ⅞
3 ½	2 ¾	2 ⅞	7 ½	3 ⅞	4
4	2 ⅞	3	8	3 ⅞	4 ⅛
4 ½	2 ⅞	3 ⅝	8 ½	4	4 ¼

$y = \text{diameter of rivet} + \frac{1}{8}''$

$$a - y = \sqrt{a^2 + b^2 - 2y} \quad a^1 - 2y = \sqrt{a^2 + b^2 - 3y}$$

$$b = \sqrt{2ay + y^2}$$

$$b = \sqrt{2ay + y^2}$$

a, a¹ = sum of gages minus thickness of angle.

⅝" rivets, can be taken at ⅛" less than for ¾" rivets.

1" rivets, can be taken at ⅛" more than for ¾" rivets.

STRUCTURAL DETAILS

STRUCTURAL RIVETS

AMERICAN BRIDGE COMPANY STANDARD

WEIGHT IN POUNDS PER 100 RIVETS WITH BUTTON HEADS

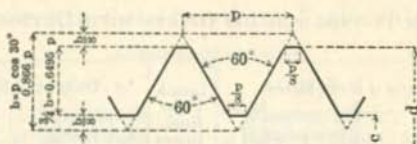
Length Under Head, Inches	Diameter of Rivet, Inches								Length Under Head, Inches	Diameter of Rivet, Inches							
	$\frac{3}{8}$	$\frac{1}{2}$	$\frac{5}{8}$	$\frac{3}{4}$	$\frac{7}{8}$	1	$1\frac{1}{8}$	$1\frac{1}{4}$		$\frac{3}{8}$	$\frac{1}{2}$	$\frac{5}{8}$	$\frac{3}{4}$	$\frac{7}{8}$	1	$1\frac{1}{8}$	$1\frac{1}{4}$
									5	18	33	53	78	109	146	190	252
									$\frac{1}{8}$	18	34	54	80	111	149	193	256
$1\frac{1}{4}$	6	12							$\frac{1}{4}$	19	34	55	82	113	152	197	260
$\frac{3}{8}$	7	13							$\frac{3}{8}$	19	35	56	83	115	155	200	265
$\frac{1}{2}$	7	13	23	35	50	68	91	130	$\frac{1}{2}$	20	36	57	85	118	157	204	269
$\frac{5}{8}$	7	14	24	36	52	71	95	134	$\frac{5}{8}$	20	36	58	86	120	160	207	273
$\frac{3}{4}$	8	15	25	37	54	74	98	139	$\frac{3}{4}$	20	37	60	88	122	163	211	278
$\frac{7}{8}$	8	15	26	39	56	77	102	143	$\frac{7}{8}$	21	38	61	89	124	166	214	282
2	9	16	27	41	58	80	105	148	6	21	38	62	91	126	169	218	287
$\frac{1}{8}$	9	17	28	43	60	82	109	152	$\frac{1}{8}$	22	39	63	93	128	171	222	291
$\frac{1}{4}$	9	18	29	44	62	85	112	156	$\frac{1}{4}$	22	40	64	94	130	174	225	295
$\frac{3}{8}$	10	18	30	46	64	88	116	161	$\frac{3}{8}$	22	40	65	96	132	177	229	300
$\frac{1}{2}$	10	19	31	47	67	91	119	165	$\frac{1}{2}$	23	41	66	97	135	180	232	304
$\frac{5}{8}$	11	20	32	49	69	93	123	169	$\frac{5}{8}$	23	42	67	99	137	182	236	308
$\frac{3}{4}$	11	20	34	50	71	96	126	174	$\frac{3}{4}$	24	43	68	100	139	185	239	313
$\frac{7}{8}$	11	21	35	52	73	99	130	178	$\frac{7}{8}$	24	43	69	102	141	188	243	317
3	12	22	36	54	75	102	133	182	7	24	44	70	104	143	191	246	321
$\frac{1}{8}$	12	22	37	55	77	105	137	187	$\frac{1}{8}$	25	45	71	105	145	194	250	326
$\frac{1}{4}$	13	23	38	57	79	107	141	191	$\frac{1}{4}$	25	45	73	107	147	196	253	330
$\frac{3}{8}$	13	24	39	58	81	110	144	195	$\frac{3}{8}$	26	46	74	108	149	199	257	334
$\frac{1}{2}$	13	24	40	60	84	113	148	200	$\frac{1}{2}$	26	47	75	110	152	202	260	339
$\frac{5}{8}$	14	25	41	61	86	116	151	204	$\frac{5}{8}$	26	47	76	111	154	205	264	343
$\frac{3}{4}$	14	26	42	63	88	118	155	208	$\frac{3}{4}$	27	48	77	113	156	207	267	347
$\frac{7}{8}$	15	27	43	64	90	121	158	213	$\frac{7}{8}$	27	49	78	114	158	210	271	352
4	15	27	44	66	92	124	162	217	8	27	50	79	116	160	213	274	356
$\frac{1}{8}$	15	28	45	68	94	127	165	221	$\frac{1}{8}$	28	50	80	118	162	216	278	360
$\frac{1}{4}$	16	29	47	69	96	130	169	226	$\frac{1}{4}$	28	51	81	119	164	219	281	365
$\frac{3}{8}$	16	29	48	71	98	132	172	230	$\frac{3}{8}$	29	52	82	121	166	221	285	369
$\frac{1}{2}$	16	30	49	72	101	135	176	234	$\frac{1}{2}$	29	52	83	122	169	224	288	373
$\frac{5}{8}$	17	31	50	74	103	138	179	239	$\frac{5}{8}$	29	53	84	124	171	227	292	378
$\frac{3}{4}$	17	31	51	75	105	141	183	243	$\frac{3}{4}$	30	54	86	125	173	230	295	382
$\frac{7}{8}$	18	32	52	77	107	143	186	247	$\frac{7}{8}$	30	54	87	127	175	232	299	386

Button Heads	Diameter of Rivets, Inches							
	$\frac{3}{8}$	$\frac{1}{2}$	$\frac{5}{8}$	$\frac{3}{4}$	$\frac{7}{8}$	1	$1\frac{1}{8}$	$1\frac{1}{4}$
100 Heads as made on rivets, Pounds	2.4	5.0	9.7	16.0	24.0	35.0	49.0	78.0
100 Heads as driven in work, Pounds	1.9	4.0	7.5	12.5	18.5	27.0	37.5	51.0

CARNEGIE STEEL COMPANY

SCREW THREADS

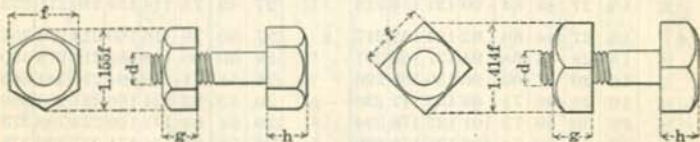
UNITED STATES AND AMERICAN BRIDGE COMPANY STANDARD



Diameter		Area		Number of Threads per Inch	Diameter		Area		Number of Threads per Inch
Total d, In.	Net, c, In.	Total Dia., d, Sq. In.	Net Dia., c, Sq. In.		Total, d, In.	Net, c, In.	Total Dia., d, Sq. In.	Net Dia., c, Sq. In.	
1/4	.185	.049	.027	20	2 1/2	2.175	4.909	3.716	4
3/8	.294	.110	.068	16	2 3/4	2.425	5.940	4.619	4
1/2	.400	.196	.126	13	3	2.629	7.069	5.428	3 1/2
5/8	.507	.307	.202	11	3 1/4	2.879	8.296	6.509	3 1/2
3/4	.620	.442	.302	10	3 1/2	3.100	9.621	7.549	3 1/4
7/8	.731	.601	.419	9	3 3/4	3.317	11.045	8.641	3
1	.838	.785	.551	8	4	3.567	12.566	9.993	3
1 1/8	.939	.994	.693	7	4 1/4	3.798	14.186	11.330	2 3/4
1 1/4	1.064	1.227	.890	7	4 1/2	4.028	15.904	12.741	2 3/4
1 3/8	1.158	1.485	1.054	6	4 3/4	4.255	17.721	14.221	2 3/8
1 1/2	1.283	1.767	1.294	6	5	4.480	19.635	15.766	2 1/2
1 3/4	1.389	2.074	1.515	5 1/2	5 1/4	4.730	21.648	17.574	2 1/2
1 7/8	1.490	2.405	1.744	5	5 1/2	4.953	23.758	19.268	2 1/4
2	1.615	2.761	2.049	5	5 3/4	5.203	25.967	21.262	2 1/4
2 1/4	1.711	3.142	2.300	4 1/2	6	5.423	28.274	23.095	2 1/4
2 3/4	1.961	3.976	3.021	4 1/2					

BOLT HEADS AND NUTS

UNITED STATES AND AMERICAN BRIDGE COMPANY STANDARD



Heads and Nuts		U. S. Standard	A. B. Co. Standard
Head	Height, h	$0.75 d + 1/16''$	0.75 d
	Short Dia., f	$1.50 d + 1/8''$	1.50 d
Nut	Height, g	d	d
	Short Dia., f	$1.50 d + 1/8''$	$1.50 d + 1/8''$

Heads for Bolts 1 1/2'' and under, A. B. Co. Standard.

Heads for Bolts 1 3/8'' and over, U. S. Standard.

STRUCTURAL DETAILS

BOLT HEADS AND NUTS

AMERICAN BRIDGE COMPANY STANDARD

Dia. of Bolt, In.	Head						Dia. of Bolt, In.	Nut					
	Hexagon		Height, In.	Square		Dia. of Bolt, In.		Hexagon		Height, In.	Square		
	Diameter, In.			Diameter, In.				Diameter, In.			Diameter, In.		
	Long	Short	Long	Short	Long	Short		Long	Short	Long	Short		
1/4	3/16	3/8	3/16	1/2	3/8	1/4	9/16	1/2	1/4	1 1/16	1/2		
3/8	5/8	5/8	1/4	5/8	9/16	3/8	1 1/16	1 1/16	3/8	1 1/16	1 1/16		
1/2	7/8	3/4	3/8	1 1/8	5/4	1/2	1	7/8	1/2	1 1/4	7/8		
5/8	1 1/8	1 1/16	1/2	1 3/8	1 1/4	5/8	1 1/4	1 1/16	5/8	1 3/8	1 1/16		
3/4	1 1/4	1 1/8	9/16	1 5/8	1 1/2	3/4	1 1/2	1 1/4	3/4	1 3/4	1 1/4		
7/8	1 1/2	1 1/16	5/8	1 7/8	1 3/4	7/8	1 3/8	1 1/16	7/8	2	1 1/16		
1	1 3/4	1 1/2	3/4	2 1/4	1 1/2	1	1 7/8	1 1/2	1	2 1/4	1 1/2		
1 1/8	2	1 11/16	7/8	2 3/8	1 11/16	1 1/8	2 1/8	1 13/16	1 1/2	2 5/8	1 11/16		
1 1/4	2 1/4	1 7/8	1 1/2	2 5/8	1 7/8	1 1/4	2 3/4	2	1 3/4	2 7/8	2		
1 1/2	2 3/4	2 1/16	1	2 7/8	2 1/16	1 1/2	2 5/2	2 2/16	1 3/4	3 1/8	2 3/16		
1 3/8	2 5/8	2 1/8	1 1/8	3 1/8	2 1/4	1 3/8	2 7/8	2 5/8	1 3/2	3 3/8	2 5/8		
1 5/8	3	2 9/16	1 1/4	3 3/8	2 9/16	1 5/8	3	2 9/16	1 3/2	3 3/8	2 9/16		
1 3/4	3 1/8	2 5/16	1 3/8	3 7/8	2 5/8	1 3/4	3 1/8	2 5/8	1 3/4	3 7/8	2 5/8		
1 7/8	3 3/8	2 15/16	1 1/2	4 1/8	2 15/16	1 7/8	3 3/8	2 15/16	1 3/2	4 1/8	2 15/16		
2	3 5/8	3 1/8	1 5/8	4 5/8	3 1/8	2	3 5/8	3 1/8	2	4 5/8	3 1/8		
2 1/4	4	3 1/2	1 3/4	5	3 1/2	2 1/4	4	3 1/2	2 1/4	5	3 1/2		
2 1/2	4 1/2	3 7/8	1 11/16	5 1/2	3 7/8	2 1/2	4 1/2	3 7/8	2 1/2	5 1/2	3 7/8		
2 3/4	4 3/4	4 1/4	2 1/8	6	4 1/4	2 3/4	4 3/4	4 1/4	2 3/4	6	4 1/4		
3	5 1/8	4 5/8	2 1/16	6 1/2	4 5/8	3	5 1/8	4 5/8	3	6 1/2	4 5/8		
3 1/4	5 1/4	5	2 1/2	7	5	3 1/4	5 1/4	5	3 1/4	7	5		
3 1/2	6 1/4	5 5/8	2 11/16	7 5/8	5 5/8	3 1/2	6 1/4	5 5/8	3 1/2	7 5/8	5 5/8		
3 3/4	6 3/4	5 3/4	2 7/8	8 1/8	5 3/4	3 3/4	6 3/4	5 3/4	3 3/4	8 1/8	5 3/4		
4	7	6 1/8	3 1/16	8 5/8	6 1/8	4	7	6 1/8	4	8 5/8	6 1/8		
4 1/4	7 1/2	6 1/2	3 1/4	9 1/4	6 1/2	4 1/4	7 1/2	6 1/2	4 1/4	9 1/4	6 1/2		
4 1/2	8	6 5/8	3 7/16	9 3/4	6 5/8	4 1/2	8	6 5/8	4 1/2	9 3/4	6 5/8		
4 3/4	8 3/4	7 1/4	3 3/8	10 1/4	7 1/4	4 3/4	8 3/4	7 1/4	4 3/4	10 1/4	7 1/4		
5	8 3/4	7 3/8	3 11/16	10 3/4	7 3/8	5	8 3/4	7 3/8	5	10 3/4	7 3/8		
5 1/4	9 1/4	8	4	11 1/4	8	5 1/4	9 1/4	8	5 1/4	11 1/4	8		
5 1/2	9 5/8	8 3/8	4 3/16	11 5/8	8 3/8	5 1/2	9 5/8	8 3/8	5 1/2	11 5/8	8 3/8		
5 3/4	10 1/8	8 3/4	4 3/8	12 1/8	8 3/4	5 3/4	10 1/8	8 3/4	5 3/4	12 1/8	8 3/4		
6	10 1/2	9 1/2	4 9/16	12 1/2	9 1/2	6	10 1/2	9 1/2	6	12 1/2	9 1/2		

LENGTH OF BOLT THREADS

Length of Bolt, In.	Diameter of Bolt, Inches									
	1/4	3/8	1/2	5/8	3/4	7/8	1	1 1/8	1 1/4	
1	to 1 1/2	3/4	3/4	1	1 1/4					
1 3/8	to 2	3/4	3/4	1	1 1/4	1 1/2	1 1/2			
2 1/8	to 2 1/2	3/4	3/4	1	1 1/4	1 1/2	1 1/2	1 3/4		
2 3/8	to 3	3/4	3/4	1	1 1/4	1 1/2	1 1/2	1 3/4	1 3/4	2 1/4
3 1/8	to 4	3/4	3/4	1 1/4	1 1/4	1 1/2	1 1/2	1 3/4	1 3/4	2 1/4
4 1/8	to 8	1	1	1 1/4	1 1/4	1 1/2	1 1/2	1 3/4	1 3/4	2 1/4
8 1/2	to 12	1	1	1 1/2	1 1/2	1 3/4	2	2 1/4	2 1/4	3
12 3/8	to 20	1	1	1 1/2	2	2	2 1/4	2 1/4	2 1/2	3

Bolts not listed are threaded about 3 times the diameter; in no case are standard bolts threaded closer to the head than 1/4 inch.

CARNEGIE STEEL COMPANY

BOLTS WITH SQUARE HEADS AND NUTS

AMERICAN BRIDGE COMPANY STANDARD

WEIGHT IN POUNDS PER 100 BOLTS

Length Under Head, Inches	Diameter of Bolt, Inches								
	¼	⅝	¾	⅞	1½	¾	1	1¼	1
1	4	7	11	15	22	37	56		
1½	4	7	11	16	23	39	59		
1½	5	8	12	17	24	41	62		
1¾	5	8	13	18	26	43	64		
2	5	9	14	19	27	45	67	101	144
2¼	6	9	15	20	28	47	71	104	150
2½	6	10	15	21	30	49	74	109	155
2¾	6	10	16	22	31	51	77	113	161
3	7	11	17	24	33	54	80	117	167
3½	7	12	18	25	35	58	86	126	178
4	8	13	20	28	38	62	92	134	189
4½	9	14	21	30	41	66	98	142	198
5	10	15	23	32	43	71	104	151	209
5½	10	16	25	34	46	75	111	159	220
6	11	17	26	36	49	79	117	168	232
6½			28	38	52	84	123	176	243
7			29	40	55	88	129	185	254
7½			31	42	57	92	136	193	265
8			32	45	60	97	142	202	276
9			34	49	65	105	154	218	298
10				53	71	114	167	235	320
12				61	82	131	192	269	364
14					93	148	217	303	409
Per Inch Additional	1.4	2.2	3.1	4.3	5.6	8.7	12.5	17.0	22.3

SQUARE NUTS AND BOLT HEADS

AMERICAN BRIDGE COMPANY STANDARD

WEIGHT IN POUNDS FOR ONE HEAD AND ONE NUT

Diameter of Bolt, Inches	1¼	1½	1¾	2	2½	3
Square Head and Nut....	2.05	3.51	5.48	8.08	15.5	26.2
Weight of Shank per Inch	.3477	.5007	.6815	.8900	1.391	2.003

STRUCTURAL DETAILS

BOLTS WITH HEXAGON HEADS AND NUTS

AMERICAN BRIDGE COMPANY STANDARD

WEIGHT IN POUNDS PER 100 BOLTS

Length Under Head, Inches	Diameter of Bolt, Inches					Length Under Head, Inches	Diameter of Bolt, Inches				
	½	⅝	¾	⅞	1		½	⅝	¾	⅞	1
1	19	33	52			8	58	92	137	194	264
1½	20	34	54			8½	60	96	143	202	274
1¾	22	36	57			9	63	100	149	210	285
1¾	23	38	60			9½	66	105	156	219	296
2	24	40	63	93	132	10	68	109	162	227	307
2¼	26	43	66	97	137	10½	71	114	168	236	318
2½	27	45	69	101	143	11	74	118	174	244	329
2¾	29	47	72	105	148	11½	77	122	181	253	341
3	30	49	75	109	154	12	80	127	187	261	352
3¼	31	51	78	114	160	12½	82	131	193	270	363
3½	33	54	82	118	165	13	85	135	199	278	374
3¾	34	56	85	122	171	13½	88	139	206	287	385
4	35	58	88	126	176	14	91	144	212	295	396
4¼	37	60	90	130	180	14½	93	148	218	304	407
4½	38	62	94	134	186	15	96	152	225	312	418
4¾	39	64	97	138	191	15½	99	157	231	321	430
5	41	66	100	143	197	16	102	161	237	329	441
5¼	42	68	103	147	202	16½	105	165	243	338	452
5½	44	71	106	151	208	17	107	170	250	346	463
5¾	45	73	109	156	213	17½	110	174	256	355	474
6	46	75	112	160	219	18	113	177	262	364	485
6¼	48	77	115	164	225	18½	116	183	268	372	496
6½	49	79	119	168	230	19	119	187	275	381	507
6¾	51	81	122	173	236	19½	121	191	281	389	519
7	52	84	125	177	241	20	124	196	287	398	530
7¼	53	86	128	181	247						
7½	55	88	131	185	252						
7¾	56	90	134	190	258						
Per Inch Additional	5.6	8.7	12.5	17.0	22.3	Per Inch Additional	5.6	8.7	12.5	17.0	22.3

HEXAGON NUTS AND BOLT HEADS

AMERICAN BRIDGE COMPANY STANDARD

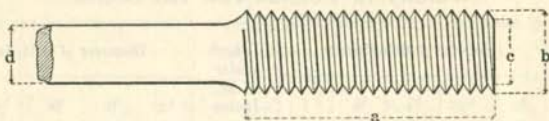
WEIGHT IN POUNDS FOR ONE HEAD AND ONE NUT

Diameter of Bolt, Inches	1¼	1½	1¾	2	2½	3
Hexagon Head and Nut..	1.73	2.95	4.61	6.79	13.0	22.0
Weight of Shank per Inch	.3477	.5007	.6815	.8900	1.391	2.003

CARNEGIE STEEL COMPANY

UPSET SCREW ENDS FOR SQUARE BARS

AMERICAN BRIDGE COMPANY STANDARD



Thread: Shape and Pitch, U. S. Standard.

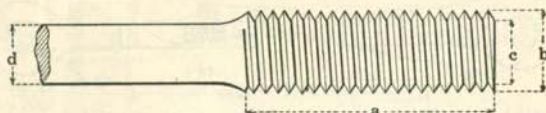
BAR			UPSET					
Side of Square d, Inches	Area, Sq. Inches	Weight per Foot, Lbs.	Diameter b, Inches	Length a, Inches	Additional Length for Upset +10%, Inches	Diameter at Root of Thread c, Inches	Area	
							At Root of Thread, Sq. Inches	Excess Over Area of Bar, %
* 3/4	0.563	1.91	1 1/8	4	4	0.939	0.693	23.2
* 7/8	0.766	2.60	1 1/4	4	4	1.064	0.890	16.2
1	1.000	3.40	1 1/2	4	4	1.283	1.294	29.4
1 1/8	1.266	4.30	1 5/8	4	3 1/2	1.389	1.515	19.7
1 1/4	1.563	5.31	1 3/4	4 1/2	4 1/2	1.615	2.049	31.1
1 3/8	1.891	6.43	2	4 1/2	4	1.711	2.300	21.7
1 1/2	2.250	7.65	2 1/4	5	5	1.961	3.021	34.3
1 5/8	2.641	8.98	2 3/8	5	4 1/2	2.086	3.419	29.5
1 3/4	3.063	10.41	2 1/2	5 1/2	4 1/2	2.175	3.716	21.3
1 7/8	3.516	11.95	2 3/4	5 1/2	5	2.425	4.619	31.4
2	4.000	13.60	2 7/8	6	5	2.550	5.108	27.7
2 1/8	4.516	15.35	3	6	4 1/2	2.629	5.428	20.2
2 1/4	5.063	17.21	3 1/4	6 1/2	5 1/2	2.879	6.509	28.6
2 3/8	5.641	19.18	3 3/4	7	6 1/2	3.100	7.549	33.8
2 1/2	6.250	21.25	3 3/4	7	7	3.317	8.641	38.3
2 5/8	6.891	23.43	3 3/4	7	5 1/2	3.317	8.641	25.4
2 3/4	7.563	25.71	4	7 1/2	6 1/2	3.567	9.993	32.1
2 7/8	8.266	28.10	4 1/4	8	7 1/2	3.798	11.330	37.1
3	9.000	30.60	4 1/4	8	6	3.798	11.330	25.9
3 1/8	9.766	33.20	4 1/2	8 1/2	7	4.028	12.741	30.5
3 1/4	10.563	35.91	4 3/4	8 1/2	7 1/2	4.255	14.221	34.6

*Upsets are special.

STRUCTURAL DETAILS

UPSET SCREW ENDS FOR ROUND BARS

AMERICAN BRIDGE COMPANY STANDARD



Thread: Shape and Pitch, U. S. Standard.

BAR			UPSET					
Diameter d, Inches	Area, Sq. Inches	Weight per Foot, Lbs.	Diameter b, Inches	Length a, Inches	Additional Length for Upset +10%, Inches	Diameter at Root of Thread c, Inches	Area	
							At Root of Thread, Sq. Inches	Excess Over Area of Bar, %
* 3/8	0.442	1.50	1	4	5	0.838	0.551	24.7
* 7/8	0.601	2.04	1 1/4	4	5 1/2	1.064	0.890	48.0
1	0.785	2.67	1 3/8	4	4	1.158	1.054	34.2
1 1/8	0.994	3.38	1 1/2	4	4	1.283	1.294	30.2
1 1/4	1.227	4.17	1 5/8	4	4	1.389	1.515	23.5
1 3/8	1.485	5.05	1 3/4	4	4	1.490	1.744	17.5
1 1/2	1.767	6.01	2	4 1/2	4 1/2	1.711	2.300	30.2
1 5/8	2.074	7.05	2 1/8	4 1/2	4	1.836	2.649	27.7
1 3/4	2.405	8.18	2 1/4	5	4	1.961	3.021	25.6
1 7/8	2.761	9.39	2 3/8	5	4	2.086	3.419	23.8
2	3.142	10.68	2 1/2	5 1/2	4	2.175	3.716	18.3
2 1/8	3.547	12.06	2 5/8	5 1/2	3 1/2	2.300	4.156	17.2
2 1/4	3.976	13.52	2 7/8	6	4 1/2	2.550	5.108	28.4
2 3/8	4.430	15.06	3	6	4 1/2	2.629	5.428	22.5
2 1/2	4.909	16.69	3 1/4	6 1/2	5 1/2	2.879	6.509	32.6
2 5/8	5.412	18.40	3 1/2	6 1/2	4 1/2	2.879	6.509	20.3
2 3/4	5.940	20.19	3 3/4	7	5 1/2	3.100	7.549	27.1
2 7/8	6.492	22.07	3 3/4	7	6	3.317	8.641	33.1
3	7.069	24.03	3 3/4	7	5	3.317	8.641	22.2
3 1/8	7.670	26.08	4	7 1/2	6	3.567	9.993	30.3
3 1/4	8.296	28.21	4	7 1/2	5	3.567	9.993	20.5
3 3/8	8.946	30.42	4 1/4	8	5 1/2	3.798	11.330	26.6
3 1/2	9.621	32.71	4 1/4	8	5	3.798	11.330	17.8
3 5/8	10.321	35.09	4 1/2	8 1/2	5 1/2	4.028	12.741	23.4
3 3/4	11.045	37.55	4 3/4	8 1/2	6	4.255	14.221	28.8
3 7/8	11.793	40.10	4 3/4	8 1/2	5 1/2	4.255	14.221	20.6

*Upsets are special.

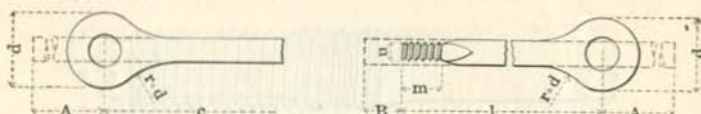
CARNEGIE STEEL COMPANY

EYE BARS

AMERICAN BRIDGE COMPANY STANDARD

ORDINARY EYE BAR

ADJUSTABLE EYE BAR



Minimum length, l, for short end is 6'-6", preferably 7'-0". Left thread.

Thread: Shape and Pitch, U. S. Standard.

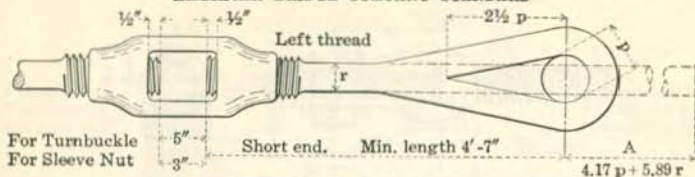
BAR				HEAD				BAR				SCREW END			
Width In.	Thickness		Dia. d, In.	Maximum Pin		Additional Material, A, Ft. and In.		Width In.	Min. thickness In.	Dia. u, In.	Excess over Bar %	Length m, In.	Additional Material, B, Ft. and In.		
	Max. In.	Min. In.		Dia. In.	Excess Head over Bar, %	For ordering Bar	For figuring Weight						For ordering Bar	For figuring Bar Wt.	
2	1	1/2	4 1/2	1 3/4	0-10 1/2	0-7	2	* 5/8	1 3/8	39.6	4	1-0	8		
		* 6 1/2	5 1/2	2 3/4	1-2 1/2	0-11								31.4	4 1/2
2 1/2	1	5/8	6	2 1/2	1-1 1/4	0-10	2 1/2	* 3/4	2 1/8	41.2	4 1/2	1-0	8		
		* 8	7	3 1/2	1-5 3/4	1-2								38.1	5
3	1 1/2	5/8	7 1/2	3 1/4	1-4 1/2	1-1	3	* 3/4	2 1/4	34.3	5	1-0	7 1/2		
		* 9 1/2	8 1/2	4 1/4	1-9 1/2	1-5								23.9	5 1/2
4	1 3/4	3/4	10	4 1/2	1-9	1-6	4	* 3/4	2 1/2	33.0	5 1/2	1-1	8 1/2		
		* 12	11	5 1/2	2-3	1-10								23.9	6 1/2
5	2	3/4	12	5 1/4	1-10 1/2	1-8	5	* 3/4	2 3/8	36.2	6	1-0	8		
		* 15	13 1/2	6 3/4	2-6	2-2								24.1	6
6	2	3/4	14	5 3/4	2-1	1-10	6	1 1/8	3 1/2	34.2	7	1-1	8 1/2		
		* 16 1/2	14 3/4	6 1/2	2-4	2-1								38.3	7
7	2	1	16 1/2	7	2-4 1/2	2-2	7	* 1 1/8	3 3/4	25.8	7 1/2	1-0	8		
		1 1/8	17 1/2	8	2-11	2-6								28.0	7
8	2	1 1/8	18	9	2-5 1/2	2-3	8	* 1 1/8	4 1/4	26.9	7 1/2	1-0	8		
		1 1/4	19	8	2-9 1/2	2-6								29.5	8
9	2	1 1/4	20	7 1/2	2-8 1/2	2-6	9	* 1 1/4	4 1/2	32.4	8 1/2	1-2	9 1/2		
		1 1/2	22	9 1/2	3-4	2-11								35.4	8 1/2
10	2	1 1/4	22 1/2	9	3-2 1/2	2-10	10	* 1 1/4	4 3/4	25.9	8 1/2	1-1	8 1/2		
		1 1/2	24	10 1/2	3-9	3-3								27.4	8 1/2
12	2	1 1/2	26 1/2	11	4-1	3-7	12	* 1 1/2	5 1/4	31.4	9 1/2	1-2	9		
		1 3/4	28	11 1/2	4-2	3-8								35.2	9 1/2
14	2	1 3/4	31	13	4-2	3-8	14	* 1 3/4	5 1/2	28.3	9 1/4	1-1	8		
		1 5/8	33	14	4-8	4-1								30.2	9 1/4
16	2	1 3/4	36	14	3-11	3-9	16	* 1 3/4	5 3/4	27.8	9 3/4	1-1	7 1/2		
		1 7/8	37 1/2	16	4-7	4-5								28.4	9 3/4
					5-5	4-8				30.8	10	1-1	8		
										28.3	10 1/2	1-1	8		
										29.6	10 3/4	1-1	8		
										29.7	11	1-1	8		

*Bars are special. †33", 14", 13 1/2" and over, add 4'-5 1/2" for material, 4'-2" for weight. Pin hole to be deducted in estimating weight of Eye Bars.

STRUCTURAL DETAILS

LOOP RODS AND STUB ENDS

AMERICAN BRIDGE COMPANY STANDARD



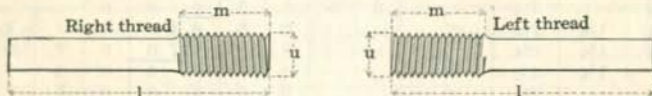
Thread: Shape and Pitch, U. S. Standard.

LENGTH A FOR ONE LOOP IN FEET AND INCHES

Diam. of Pin, p	Size of Square or Round Bar, in Inches											
	3/4	7/8	1	1 1/8	1 1/4	1 3/8	1 1/2	1 5/8	1 3/4	1 7/8	2	
1 1/8	0-9 1/2	0-10	0-11	0-11 1/2								
1 1/4	0-10	0-10 1/2	0-11 1/2	1-0	1-1							
1 1/2	0-11	0-11 1/2	1-0 1/2	1-1	1-2	1-2 1/2						
1 3/4	1-0	1-0 1/2	1-1 1/2	1-2	1-3	1-3 1/2	1-4 1/2	1-5	1-6			
2	1-1	1-1 1/2	1-2 1/2	1-3	1-4	1-4 1/2	1-5 1/2	1-6	1-7	1-7 1/2	1-8 1/2	
2 1/4	1-2	1-3	1-3 1/2	1-4 1/2	1-5	1-5 1/2	1-6 1/2	1-7	1-8	1-8 1/2	1-9 1/2	
2 1/2	1-3	1-4	1-4 1/2	1-5 1/2	1-6	1-7	1-7 1/2	1-8	1-9	1-9 1/2	1-10 1/2	
2 3/4	1-4	1-5	1-5 1/2	1-6 1/2	1-7	1-8	1-8 1/2	1-9 1/2	1-10	1-11	1-11 1/2	
3	1-5	1-6	1-6 1/2	1-7 1/2	1-8	1-9	1-9 1/2	1-10 1/2	1-11	2-0	2-0 1/2	
*3 1/4	1-6	1-7	1-7 1/2	1-8 1/2	1-9	1-10	1-10 1/2	1-11 1/2	2-0	2-1	2-1 1/2	
3 1/2	1-7 1/2	1-8	1-8 1/2	1-9 1/2	1-10	1-11	1-11 1/2	2-0 1/2	2-1	2-2	2-2 1/2	
*3 3/4	1-8 1/2	1-9	1-10	1-10 1/2	1-11	2-0	2-0 1/2	2-1 1/2	2-2	2-3	2-3 1/2	
4	1-9 1/2	1-10	1-11	1-11 1/2	2-0 1/2	2-1	2-2	2-2 1/2	2-3	2-4	2-4 1/2	
*4 1/4		1-11	2-0	2-0 1/2	2-1 1/2	2-2	2-3	2-3 1/2	2-4 1/2	2-5	2-6	
4 1/2		2-0	2-1	2-1 1/2	2-2 1/2	2-3	2-4	2-4 1/2	2-5 1/2	2-6	2-7	
*4 3/4		2-1	2-2	2-2 1/2	2-3 1/2	2-4	2-5	2-5 1/2	2-6 1/2	2-7	2-8	
5		2-2 1/2	2-3	2-3 1/2	2-4 1/2	2-5	2-6	2-6 1/2	2-7 1/2	2-8	2-9	
*5 1/4			2-4	2-5	2-5 1/2	2-6	2-7	2-7 1/2	2-8 1/2	2-9	2-10	
5 1/2			2-5	2-6	2-6 1/2	2-7 1/2	2-8	2-9	2-9 1/2	2-10	2-11	
*5 3/4			2-6	2-7	2-7 1/2	2-8 1/2	2-9	2-10	2-10 1/2	2-11 1/2	3-0	
6			2-7	2-8	2-8 1/2	2-9 1/2	2-10	2-11	2-11 1/2	3-0 1/2	3-1	
*6 1/4				2-9	2-9 1/2	2-10 1/2	2-11	3-0	3-0 1/2	3-1 1/2	3-2	
6 1/2				2-10	2-10 1/2	2-11 1/2	3-0	3-1	3-1 1/2	3-2 1/2	3-3	
*6 3/4				2-11	3-0	3-0 1/2	3-1	3-2	3-2 1/2	3-3 1/2	3-4	
7				3-0	3-1	3-1 1/2	3-2 1/2	3-3	3-3 1/2	3-4 1/2	3-5	

*Pins are special.

Maximum shipping length of long end = 35 feet.

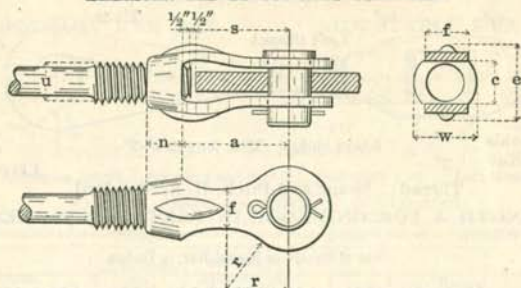


Dia. of Round	3/4	7/8	1	1 1/8	1 1/4	1 3/8	1 1/2	1 5/8	1 3/4	1 7/8	2
Side of Square		3/4	7/8	1	1 1/8	1 1/4	1 3/8	1 1/2	1 5/8	1 3/4	1 7/8
Dia. of Upset, u	4	4 1/4	4 1/2	4 3/4	4 1/2	4 3/4	4 1/2	4 3/4	4 1/2	4 3/4	4 1/2
Length of Upset, m	1	1 1/8	1 1/4	1 3/8	1 1/2	1 3/8	1 1/2	1 3/8	1 1/2	1 3/8	1 1/2
Length, l	9 1/2	9 1/2	10	10 1/2	10 1/2	11	11 1/2	11 1/2	11 1/2	12	12 1/2

CARNEGIE STEEL COMPANY

CLEVISES

AMERICAN BRIDGE COMPANY STANDARD



Grip = thickness of plate + 1/4" but must not exceed dimension, c.

Thread: Shape and Pitch, U. S. Standard.

Clevise Number	Upset		Pin		Head			Fork			Nut			Weight, Pounds	
	Min.	Max.	Min.	Max.	d	t	r	f	s	c	d	n	w		e
	u	u	p	p											
3	1	1 1/8	1	1 1/2	3	1/2	2 1/4	1 1/2	4	1 1/4	5	1 1/2	2 1/4	3 1/16	4
4	1 1/8	1 3/8	1 1/4	2	4	1/2	3	2	5	1 3/4	6	1 3/4	2 7/8	3 5/8	8
5	1 1/2	2 1/8	1 1/2	2 1/2	5	5/8	3 3/4	2 1/2	6	2 1/4	7	2 1/4	3 3/4	4 1/2	17
6	2	2 3/8	2	3	6	3/4	4 1/2	3	7	2 3/4	8	2 1/2	4 3/8	5 3/8	26
7	2 1/4	3	2 1/2	3 1/2	7	3/8	5 1/4	3 1/2	8	3 1/4	9	3	5	6 3/16	40

CLEVIS NUMBERS FOR VARIOUS RODS AND PINS

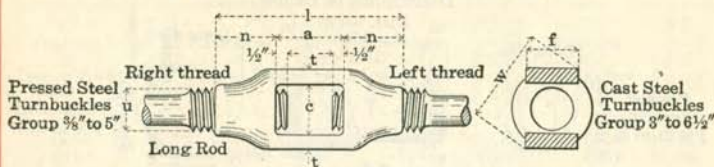
Rods			Pins										
Round	Square	Upset	1	1 1/4	1 1/2	1 3/4	2	2 1/4	2 1/2	2 3/4	3	3 1/4	3 1/2
3/4	1	3	3	3								
.....	3/4	1 1/8	3	3	3	4	4						
7/8	3/8	1 1/4		4	4	4	4						
1	1 3/8		4	4	4	4						
1 1/8	1	1 1/2		4	4	4	4	5	5				
1 1/4	1 1/8	1 5/8		4	4	4	4	5	5				
1 3/8	1 3/4			5	5	5	5	5				
.....	1 1/2	1 7/8			5	5	5	5	5	5			
1 1/2	1 3/8	2			5	5	5	5	5	6	6		
1 5/8	2 1/8			5	5	5	5	5	6	6		
1 3/4	1 1/2	2 1/4					6	6	6	6	6	7	7
1 7/8	1 5/8	2 3/8					6	6	6	6	6	7	7
2	1 3/4	2 1/2					6	6	6	6	6	7	7
2 1/8	2 3/8					6	6	6	6	6	7	7
.....	1 7/8	2 3/4						7	7	7	7	7	7
2 1/4	2	2 7/8						7	7	7	7	7	7
2 3/8	2 1/8	3						7	7	7	7	7	7

Clevises above and to right of zigzag line may be used with forks straight, clevises below and to left of this line should have forks closed so as not to overstrain pin.

STRUCTURAL DETAILS

TURNBUCKLES AND SLEEVE NUTS

AMERICAN BRIDGE COMPANY STANDARD

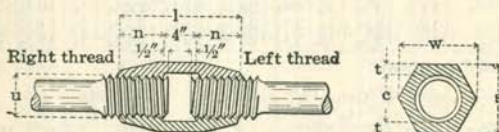


Thread: Shape and Pitch, U. S. Standard.

Dia. of Screw, u	Dimensions in Inches							Weight, Pounds	Dia. of Screw, u	Dimensions in Inches							Weight, Pounds
	w	l	n	c	t	f	w			l	n	c	t	f			
3/8	1 1/16	7 3/8	9/16	9/16	3/16	1/2	0.4	3 1/4	7	15 3/4	4 7/8	3 3/4	1 3/8	4			
7/16	1 3/8	7 1/4	5/8	5/8	1/4	5/8	0.4	3 1/2	7 1/2	16 1/2	5 1/4	3 7/8	1 3/16	4 1/2			
1/2	1 5/8	7 1/2	3/4	3/4	1/4	3/4	0.6	3 3/4	8 3/8	17 1/4	5 5/8	4 1/8	1 1/4	5 1/4			
9/16	1 7/8	7 3/4	7/8	7/8	13/16	5/8	0.6	4	9 3/8	18	6	4 5/8	1 3/8	6			
5/8	1 9/16	8	1	1	13/16	5/8	1.0	4 1/4	10 3/8	21 1/2	6 1/4	5 3/8	1 3/4	6 1/2			
3/4	2	8 1/4	1 1/8	1 1/8	1 1/16	5/8	1.4	4 1/2	10 3/4	22 1/2	6 3/4	5 3/8	1 3/4	6 1/2			
7/8	2 1/4	8 1/2	1 1/4	1 1/4	1/4	5/8	1.7	4 3/4	11 3/8	23 1/2	7 1/4	5 1/2	2	6 1/2			
1	2 7/16	9	1 1/2	1 1/2	1 1/16	7/16	2.3	5	12	24	7 1/2	5 7/8	2 1/4	6 1/2			
1 1/8	2 9/16	9 1/2	1 3/4	1 3/4	1 1/16	1/2	3.2										
1 1/4	2 3/4	9 3/4	1 7/8	1 7/8	1 1/16	1/2	4.0	3	7 1/4	15	4 1/2	3 5/8	1 1/4	4	50		
1 3/8	3 1/16	10	2	1 11/16	1/2	1 1/8	4.9	3 1/2	7 3/4	15 3/4	4 7/8	3 7/8	1 3/8	4 1/4	62		
1 1/2	3 3/16	10 1/2	2 1/4	1 3/4	9/16	1 1/4	6.3	3 1/2	8 1/4	16 1/2	5 1/4	4 1/2	1 1/2	4 3/4	79		
1 5/8	3 1/2	11	2 1/2	2	9/16	1 1/8	7.0	3 3/4	9	17 1/4	5 5/8	4 1/2	1 5/8	4 3/4	94		
1 3/4	3 3/4	11 1/4	2 5/8	2 1/8	5/8	2	8.4										
1 7/8	3 7/8	11 1/2	2 3/4	2 1/16	1 1/16	2 1/8	10.5	4	9 1/2	18	6	4 3/4	1 3/4	5 1/4	116		
2	4 1/4	12	3	2 3/8	1 1/16	2 1/4	11.9	4 1/4	10 1/2	21 3/4	6 3/8	5 1/4	1 7/8	5 5/8	158		
2 1/8	4 3/4	12 3/4	3 3/8	2 11/16	1 3/16	2 1/2	18.6	4 3/4	11 1/2	23 1/4	7 1/8	5 3/4	2	6 1/2	173		
2 1/4	4 3/4	12 3/4	3 3/8	2 11/16	1 3/16	2 1/2	18.6	5	12	24	7 1/2	6	2	7	198		
2 3/8	4 5/8	13 1/4	3 3/4	2 11/16	1 3/16	2 1/2	25.5	5 1/4	12 1/2	24 3/4	7 7/8	6 1/4	2 1/8	7 3/8	231		
2 1/2	5 3/8	13 1/2	3 3/4	3 1/16	7/8	2 3/4	25.5	5 1/2	13 1/4	25 1/2	8 1/4	6 1/2	2 1/4	7 3/4	264		
2 5/8	5 3/4	14 1/4	4 1/8	3 1/4	1 5/16	3	34.2	5 3/4	13 3/4	26 1/4	8 3/8	6 3/4	2 3/8	8	301		
2 3/4	5 3/4	14 1/4	4 1/8	3 1/4	1 5/16	3	34.2	6	14 1/2	27	9	7	2 5/8	8 1/4	344		
2 7/8	6 3/8	15	4 1/2	3 3/8	1	3 1/4	40.5	6 1/4	15	27 3/4	9 3/8	7 1/4	2 3/4	8 1/2	400		
3	6 3/4	15	4 1/2	3 3/8	1	3 1/4	40.5	6 1/2	15 1/2	28 1/2	9 3/4	7 1/2	2 7/8	8 3/4	487		
															583		

a = 6" for screws 4" or under.

a = 9" for screws 4 1/2" or over.



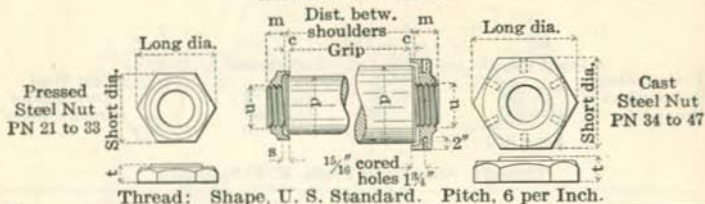
Dia. of Screw, u	Dimensions in Inches							Weight, Pounds	Dia. of Screw, u	Dimensions in Inches							Weight, Pounds
	w	l	n	c	t	s	w			l	n	c	t	s			
4	7 1/8	13	4 1/2	4 1/4	1 5/16	6 1/8	63	5	8 3/8	15	5 1/2	5 1/4	1 3/16	7 5/8	110		
4 1/4	7 1/2	13 1/2	4 3/4	4 1/2	1	6 1/2	73	5 1/4	9 1/4	15 1/2	5 3/4	5 1/2	1 1/4	8	122		
4 1/2	8	14	5	4 3/4	1 1/16	6 7/8	84	5 1/2	9 3/4	16	6	5 3/4	1 3/16	8 3/8	142		
4 3/4	8 3/8	14 1/2	5 1/4	5	1 1/8	7 1/4	98	5 3/4	10 1/8	16 1/2	6 1/4	6	1 3/8	8 3/4	157		
								6	10 3/8	17	6 1/2	6 1/4	1 3/16	9 1/8	176		

CARNEGIE STEEL COMPANY

RECESSED PIN NUTS AND COTTER PINS

AMERICAN BRIDGE COMPANY STANDARD

Dimensions in Inches



Diameter of Pin p	Pin				Nut							Weight, Pounds	Pattern No.
	Thread		c	Thick- ness t	Diameter		Recess		Diameter Rough Hole	Weight, Pounds	Pattern No.		
	u	m			Short Dia.	Long Dia.	Rough Dia.	s					
2	2 1/4	1 1/2	1 1/8	3/8	3/8	3	3 3/8	2 5/8	1/4	1 1/4	1	PN 21	
2 1/2	2 3/4	2	1 1/8	1	3/8	3 3/8	4 1/8	3 1/8	1/4	1 1/4	2	PN 22	
*3	*3 1/4	2 1/2	1 1/4	1 1/8	1 1/8	4 3/8	5	3 7/8	1/4	1 3/4	3	PN 23	
*3 1/2	*4	3	1 3/8	1 1/4	1 1/4	4 7/8	5 5/8	4 3/8	1/4	2 1/4	4	PN 24	
*4 1/4	*4 1/2	*4 3/4	3 1/2	1 1/2	1 1/2	5 3/4	6 5/8	5 1/4	1/2	3 3/4	5	PN 25	
5	5 1/4	4	1 3/8	1 1/2	1 1/2	6 1/4	7 3/4	5 3/4	1/2	3 3/4	6	PN 26	
5 1/2	5 3/4	4 1/2	1 3/4	1 3/8	1 3/8	6 3/4	8 1/8	6 1/2	1/2	4 1/4	8	PN 27	
*6	*6 1/4	5	1 3/8	1 3/4	1 3/4	7	8 3/8	7	1/2	4 3/4	10	PN 28	
*6 1/2	*7	5 1/2	2	1 3/8	1 3/8	7 3/8	9 3/8	8 1/2	1/2	5 1/4	12	PN 29	
*7 1/4	*7 1/2	6	2 1/4	2 1/8	2 1/8	8 1/4	10	9 1/2	1/2	5 3/4	14	PN 30	
*8	*8 1/4	6 1/2	2 1/2	2 1/4	2 1/4	8 3/4	10 7/8	10 1/2	1/2	5 3/4	19	PN 31	
*8 1/2	*9	7	2 3/4	2 3/8	2 3/8	9 1/4	11 1/8	11 1/2	1/2	5 3/4	24	PN 32	
*9	*9 1/4	7 1/2	2 3/4	2 3/4	2 3/4	10 1/4	13	12 1/2	1/2	5 3/4	32	PN 33	
*9 1/2	*10	8	2 3/8	2 3/4	2 3/4	11 1/4	13	13 1/2	1/2	5 3/4	32	PN 33	
11	11 1/4	8 1/2	2 3/8	2 3/4	2 3/4	12 1/2	14 3/8	14 1/2	3/4	5 3/4	60	PN 34	
12	12 1/4	9	2 3/8	2 3/4	2 3/4	13 1/2	15 3/8	15 1/2	3/4	5 3/4	73	PN 35	
13	13 1/4	9 1/2	2 3/4	2 3/4	2 3/4	14 1/2	16 3/8	16 1/2	3/4	6 3/4	81	PN 36	
14	14 1/4	10	2 3/4	2 3/4	2 3/4	15 1/2	17 3/8	17 1/2	3/4	6 3/4	94	PN 37	
15	15 1/4	10 1/2	2 3/4	2 3/4	2 3/4	16 1/2	19	18 1/2	3/4	7 3/4	103	PN 38	
16	16 1/4	11	2 3/4	2 3/4	2 3/4	17 1/2	20 1/8	19 1/2	3/4	7 3/4	126	PN 39	
17	17 1/4	11 1/2	2 3/4	2 3/4	2 3/4	18 1/2	21 1/8	20 1/2	3/4	8 3/4	145	PN 40	
18	18 1/4	12	2 3/4	2 3/4	2 3/4	19 1/2	22 1/8	21 1/2	3/4	8 3/4	173	PN 41	
19	19 1/4	12 1/2	2 3/4	2 3/4	2 3/4	20 1/2	23 1/8	22 1/2	3/4	9 3/4	196	PN 42	
20	20 1/4	13	2 3/4	2 3/4	2 3/4	21 1/2	24 1/8	23 1/2	3/4	9 3/4	222	PN 43	
21	21 1/4	13 1/2	2 3/4	2 3/4	2 3/4	22 1/2	26	25 1/2	3/4	10 3/4	240	PN 44	
22	22 1/4	14	2 3/4	2 3/4	2 3/4	23 1/2	27 1/8	26 1/2	3/4	10 3/4	266	PN 45	
23	23 1/4	14 1/2	2 3/4	2 3/4	2 3/4	24 1/2	28 1/8	27 1/2	3/4	11 3/4	282	PN 46	
24	24 1/4	15	2 3/4	2 3/4	2 3/4	25 1/2	29 1/8	28 1/2	3/4	11 3/4	310	PN 47	

Sizes marked * are special.



Pin	Head	Cotter		Pin	Head	Cotter	
p	h	c	d	p	h	c	d
1 1/4	1 1/2	2	1/4	2 3/4	3 1/8	4	3/8
1 1/2	1 3/4	2 1/2	1/4	3	3 1/2	5	3/8
1 3/4	2	2 3/4	1/4	3 1/4	3 3/4	5	1/2
2	2 3/8	3	3/8	3 1/2	4	6	1/2
2 1/4	2 5/8	3 1/4	3/8	3 3/4	4 1/4	6	1/2
2 1/2	2 3/4	3 3/4	3/8				

DECIMAL TABLE

DECIMAL OF AN INCH AND OF A FOOT

Fractions of Inch or Foot	Inch Equivalents to Foot Fractions	Fractions of Inch or Foot	Inch Equivalents to Foot Fractions	Fractions of Inch or Foot	Inch Equivalents to Foot Fractions	Fractions of Inch or Foot	Inch Equivalents to Foot Fractions
.0052	1/160	.2552	3 1/40	.5052	6 1/100	.7552	9 1/100
.0104	1/80	.2604	3 1/80	.5104	6 1/80	.7604	9 1/80
1/64 .015625	1/64	1 7/64 .265625	3 3/64	3 3/64 .515625	6 3/64	4 9/64 .765625	9 3/64
.0208	1/48	.2708	3 1/48	.5208	6 1/48	.7708	9 1/48
.0260	1/36	.2760	3 1/36	.5260	6 1/36	.7760	9 1/36
1/32 .03125	1/32	9 3/32 .28125	3 3/32	1 7/32 .53125	6 3/32	2 5/32 .78125	9 3/32
.0365	1/27	.2865	3 1/27	.5365	6 1/27	.7865	9 1/27
.0417	1/24	.2917	3 1/24	.5417	6 1/24	.7917	9 1/24
3/64 .046875	3/64	1 9/64 .296875	3 3/64	2 5/64 .546875	6 3/64	3 1/64 .796875	9 3/64
.0521	1/19	.3021	3 5/19	.5521	6 5/19	.8021	9 5/19
.0573	1/16	.3073	3 1/16	.5573	6 1/16	.8073	9 1/16
1/10 .0625	1/10	5/10 .3125	3 3/10	9/10 .5625	6 3/10	1 3/10 .8125	9 3/10
.0677	1 1/16	.3177	3 1/16	.5677	6 1/16	.8177	9 1/16
.0729	3/40	.3229	3 3/40	.5729	6 3/40	.8229	9 3/40
5/64 .078125	5/64	2 1/64 .328125	3 1/64	2 3/64 .578125	6 1/64	2 5/64 .828125	9 1/64
.0833	1	.3333	4	.5833	7	.8333	10
.0885	1 1/10	.3385	4 1/10	.5885	7 1/10	.8385	10 1/10
3/32 .09375	1 1/32	1 1/32 .34375	4 1/32	1 9/32 .59375	7 1/32	2 1/32 .84375	10 1/32
.0990	1 1/10	.3490	4 1/10	.5990	7 1/10	.8490	10 1/10
.1042	1 1/4	.3542	4 1/4	.6042	7 1/4	.8542	10 1/4
7/64 .109375	1 1/64	2 3/64 .359375	4 1/64	2 5/64 .609375	7 1/64	2 7/64 .859375	10 1/64
.1146	1 3/8	.3646	4 3/8	.6146	7 3/8	.8646	10 3/8
.1198	1 1/8	.3698	4 1/8	.6198	7 1/8	.8698	10 1/8
1/8 .1250	1 1/2	3/8 .3750	4 1/2	5/8 .6250	7 1/2	7/8 .8750	10 1/2
.1302	1 1/10	.3802	4 1/10	.6302	7 1/10	.8802	10 1/10
.1354	1 3/8	.3854	4 3/8	.6354	7 3/8	.8854	10 3/8
9/64 .140625	1 1/64	2 5/64 .390625	4 1/64	2 7/64 .640625	7 1/64	2 9/64 .890625	10 1/64
.1458	1 3/4	.3958	4 3/4	.6458	7 3/4	.8958	10 3/4
.1510	1 1/10	.4010	4 1/10	.6510	7 1/10	.9010	10 1/10
5/32 .15625	1 5/32	1 5/32 .40625	4 5/32	2 1/32 .65625	7 5/32	2 3/32 .90625	10 5/32
.1615	1 1/10	.4115	4 1/10	.6615	7 1/10	.9115	10 1/10
.1667	2	.4167	5	.6667	8	.9167	11
1 1/64 .171875	2 1/64	2 1/64 .421875	5 1/64	2 3/64 .671875	8 1/64	2 5/64 .921875	11 1/64
.1771	2 1/8	.4271	5 1/8	.6771	8 1/8	.9271	11 1/8
.1823	2 1/10	.4323	5 1/10	.6823	8 1/10	.9323	11 1/10
3/10 .1875	2 1/4	7/10 .4375	5 1/4	1 1/10 .6875	8 1/4	1 3/10 .9375	11 1/4
.1927	2 1/10	.4427	5 1/10	.6927	8 1/10	.9427	11 1/10
.1979	2 3/8	.4479	5 3/8	.6979	8 3/8	.9479	11 3/8
1 3/64 .203125	2 3/64	2 3/64 .453125	5 3/64	2 5/64 .703125	8 3/64	2 7/64 .953125	11 3/64
.2083	2 1/2	.4583	5 1/2	.7083	8 1/2	.9583	11 1/2
.2135	2 1/10	.4635	5 1/10	.7135	8 1/10	.9635	11 1/10
7/32 .21875	2 5/32	1 5/32 .46875	5 5/32	2 1/32 .71875	8 1/32	2 3/32 .96875	11 5/32
.2240	2 1/10	.4740	5 1/10	.7240	8 1/10	.9740	11 1/10
.2292	2 3/4	.4792	5 3/4	.7292	8 3/4	.9792	11 3/4
1 5/64 .234375	2 5/64	2 5/64 .484375	5 5/64	2 7/64 .734375	8 5/64	2 9/64 .984375	11 5/64
.2396	2 3/8	.4896	5 3/8	.7396	8 3/8	.9896	11 3/8
.2448	2 1/4	.4948	5 1/4	.7448	8 1/4	.9948	11 1/4
1/4 .2500	3	1/2 .5000	6	3/4 .7500	9	1 .1000	12

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