

**Revisions and Errata List**  
**AISC Steel Design Guide 13, 1<sup>st</sup> Printing (Printed Copy)**  
**October 15, 2012**

The following list represents corrections to the first printing of AISC Design Guide 13, *Stiffening of Wide-Flange Columns at Moment Connections: Wind and Seismic Applications*.

**Page(s)            Item**

9                    In the left column, the definition for  $\alpha_m$  :

$$\alpha_m = 1.36 \left( \frac{p_e}{d_b} \right)^{1/4} \text{ for a four-bolt unstiffened extended end plate}$$

$$= 1.13 \left( \frac{p_e}{d_b} \right)^{1/4} \text{ for an eight-bolt stiffened extended end plate}$$

should be replaced with:

$$\alpha_m = 1.36 \left( \frac{p_e}{d_b} \right)^{1/4}$$

9                    In the left column, the definition for  $k_1$  should be replaced with, “distance along the column flange from the center of the column web to the toe of the fillet, in.”

32                  In the left column, Equation 4.4-7 should read:

$$w_{\min} = \frac{0.9 \times 0.6 F_y t_{\text{eff}}}{0.75 \times 0.6 F_{EXX}} (\sqrt{2})$$

$$= \frac{1.70 F_y t_{\text{eff}}}{F_{EXX}} \geq t_{\text{eff}} \sqrt{2}$$

41                  At the top of the right column, the minimum web doubler plate thickness,  $t_{p \min}$ , should be replaced with:

$$t_{p \min} = \frac{h \sqrt{F_y}}{418} = \frac{[13.9 \text{ in.} - 2(1.25 \text{ in.})] \sqrt{36 \text{ ksi}}}{418}$$

$$= 0.164 \text{ in.}$$

52                  In Example 6-7 at the top of the left column, lines 2 through 8 should be replaced with:

$$N = t_f + 2w + 2t_p = 0.507 \text{ in.} + 2(1/2 \text{ in.}) + 2(3/4 \text{ in.}) = 3.01 \text{ in.}$$

$$N_d = \frac{3N}{d_c} = \frac{3(3.01 \text{ in.})}{13.9 \text{ in.}} = 0.650$$

$$\begin{aligned}
\phi R_n &= 0.75 \times 135 C_t t_w^2 \times \left[ 1 + N_d \left( \frac{t_w}{t_f} \right)^{1.5} \right] \times \sqrt{\frac{F_y t_f}{t_w}} \\
&= 0.75 \times 135 (1) (0.370 \text{ in.})^2 \times \left[ 1 + (0.650) \left( \frac{0.370 \text{ in.}}{0.660 \text{ in.}} \right)^{1.5} \right] \times \sqrt{\frac{(50 \text{ ksi})(0.660 \text{ in.})}{0.370 \text{ in.}}} \\
&= 167 \text{ kips} < P_{uf} = 172 \text{ kips} \quad \mathbf{n.g.}
\end{aligned}$$